# **Transportation Impact Assessment**

Elm Place Multifamily Residential Community Pitman Road and Elm Place Swampscott, Massachusetts

Received by Town Clerk 10/01/2021 09:19AM

Prepared for:



January 2021

Prepared by:



35 New England Business Center Drive Suite 140 Andover, MA 01810



Dear Reviewer:

This letter shall certify that this *Transportation Impact Assessment* has been prepared under my direct supervision and responsible charge. I am a Registered Professional Engineer (P.E.) in the Commonwealth of Massachusetts (Massachusetts P.E. No. 38871, Civil) and hold Certification as a Professional Traffic Operations Engineer (PTOE) from the Transportation Professional Certification Board, Inc. (TPCB), an independent affiliate of the Institute of Transportation Engineers (ITE) (PTOE Certificate No. 993). I am also a Fellow of the Institute of Transportation Engineers (FITE).

Sincerely,

VANASSE & ASSOCIATES, INC.

effrey S. Dirk

effrey S. Dirk, P.E., PTOE, FITE Managing Partner

EXECUTIVE SUMMARY	. 1
Recommendations	. 2
INTRODUCTION	. 5
Project Description Study Methodology	
EXISTING CONDITIONS	.7
Traffic Volumes Pedestrian and Bicycle Facilities Public Transportation Spot Speed Measurements Motor Vehicle Crash Data	10 11 11
FUTURE CONDITIONS	14
Future Traffic Growth       I         Project-Generated Traffic       I         Trip Distribution and Assignment       I         Future Traffic Volumes - Build Condition       I	15 17
TRAFFIC OPERATIONS ANALYSIS	19
Methodology Analysis Results	
DOHERTY CIRCLE CONNECTOR	26
SIGHT DISTANCE EVALUATION	28
CONCLUSIONS AND RECOMMENDATIONS	30
Conclusions	

No.	Title
1	Site Location Map
2	Existing Intersection Lane Use, Travel Lane Width and Pedestrian Facilities
3	2020 Existing Peak Hour Traffic Volumes
4	2028 No Build Peak Hour Traffic Volumes
5	Trip Distribution Map
6	Project Generated Peak Hour Traffic Volumes
7	2028 Build Peak Hour Traffic Volumes

No.	Title
1	Study Area Intersection Description
2	2020 Existing Traffic Volumes
3	Vehicle Travel Speed Measurements
4	Motor Vehicle Crash Data Summary
5	Trip-Generation Summary
6	Peak-Hour Traffic-Volume Increases
7	Level-of-Service Criteria for Signalized Intersections
8	Level-of-Service Criteria for Unsignalized Intersections
9	Signalized Intersection Level-of-Service and Vehicle Queue Summary
10	Unsignalized Intersection Level-of-Service and Vehicle Queue Summary
11	Sight Distance Measurements

Vanasse & Associates, Inc. (VAI) has conducted a Transportation Impact Assessment (TIA) in order to determine the potential impacts on the transportation infrastructure associated with the proposed construction of a multifamily residential community to be known as Elm Place and located off Pitman Road and Elm Place in Swampscott, Massachusetts (hereafter referred to as the Project). This assessment was prepared in consultation with the Town of Swampscott and the Massachusetts Department of Transportation (MassDOT), and was performed in accordance with MassDOT's *Transportation Impact Assessment (TIA) Guidelines* and the standards of the Traffic Engineering and Transportation Planning professions for the preparation of such reports.

Based on this assessment, we have concluded the following with respect to the Project:

- Using trip-generation statistics published by the Institute of Transportation Engineers (ITE)<sup>1</sup> and with adjustment to account for the use of public transportation and pedestrian and bicycle trips, the Project is expected to generate approximately 578 automobile trips, 118 transit trips and 16 pedestrian/bicycle trips on an average weekday (two-way, 24-hour volumes), with 36 automobile trips, 8 transit trips and 1 pedestrian/bicycle trip expected during the weekday morning peak-hour, and 46 automobile trips, 10 transit trips and 1 pedestrian/bicycle trip expected during the weekday evening peak-hour;
- 2. The Project will not result in a significant impact (increase) on motorist delays or vehicle queuing over Existing or anticipated future conditions without the Project (No-Build conditions), with all movements at the study intersections shown to operate at a level-of-service (LOS) of D or better under all analysis conditions, where an LOS of "D" or better is defined as "acceptable" traffic operations;
- 3. To the extent that a vehicular connection is advanced between Pitman Road and Doherty Circle, the connection should be one-way southbound and include pedestrian and bicycle accommodations;
- 4. No apparent safety deficiencies were noted with respect to the motor vehicle crash history at the study area intersections; and

<sup>&</sup>lt;sup>1</sup>*Trip Generation*, 10<sup>th</sup> Edition; Institute of Transportation Engineers; Washington, DC; 2017.

5. Lines of sight to and from Elm Place (south) at its intersection with Essex Street and at the Project site driveway intersection with Elm Place were found to exceed the recommended minimum distance for safe operation based on the appropriate approach speed.

In consideration of the above, we have concluded that the Project can be accommodated within the confines of the existing transportation infrastructure in a safe and efficient manner with implementation of the recommendations that follow.

# **RECOMMENDATIONS**

A detailed transportation improvement program has been developed that is designed to provide safe and efficient access to the Project site and address any deficiencies identified at off-site locations evaluated in conjunction with this study. The following improvements have been recommended as a part of this evaluation and, where applicable, will be completed in conjunction with the Project subject to receipt of all necessary rights, permits, and approvals.

### **Project Access**

Access to the Project site will be provided by way of a new driveway that will intersect the west side of Elm Place approximately 190 feet south of Essex Street, the approximate location of the south access to the existing parking lot in the eastern portion of the Project site. The following recommendations are offered with respect to the design and operation of the Project site access and internal circulation, many of which are reflected on the Site Plans:

- > The Project site driveway should be a minimum of 24 feet in width and designed to accommodate the turning and maneuvering requirements of the largest anticipated responding emergency vehicle.
- ➤ Where perpendicular parking is proposed, the drive aisle behind the parking should be a minimum of 23 feet in order to facilitate parking maneuvers.
- Vehicles exiting the Project site should be placed under STOP-sign control with a marked STOP-line provided.
- All signs and pavement markings to be installed within the Project site should conform to the applicable standards of the *Manual on Uniform Traffic Control Devices* (MUTCD).<sup>2</sup>
- Pedestrian connections have been provided to the sidewalks along both Essex Street and Pitman Road.
- Americans with Disabilities Act (ADA) compliant wheelchair ramps should be provided at all pedestrian crossings that are constructed or modified as a part of the Project.
- Signs and landscaping to be installed as a part of the Project within the intersection sight triangle areas should be designed and maintained so as not to restrict lines of sight.
- Snow windrows within sight triangle areas of the Project site driveway intersection should be promptly removed where such accumulations would impede sight lines.

<sup>&</sup>lt;sup>2</sup>Manual on Uniform Traffic Control Devices (MUTCD); Federal Highway Administration; Washington, D.C.; 2009.

> Bicycle parking should be provided at appropriate locations within the Project site.

#### Off Site

#### Essex Street/Elm Place (South)

Independent of the Project, it is recommended that a marked STOP-line be provided on the Elm Place (south) approach to Essex Street in order to defined the desired stopping point for vehicles prior to entering the intersection and the marked crosswalk. In addition, it is recommended that a double-yellow centerline be installed along Elm Place for a minimum distance of 50-feet from the STOP-line in order to separate the directions of travel approaching Essex Street and to provide proper positioning for vehicles queued on the Elm Place approach.

#### **Transportation Demand Management**

Public transportation services are provided within the study area by the Massachusetts Bay Transportation Transit Authority (MBTA), including fixed-route bus services and Commuter Rail. MBTA bus Route 455, *Salem Depot – Wonderland*, provides service between Salem Depot and Wonderland Station, where connections can be made to the Blue Line subway system and other MBTA bus routes. The Route 455 bus travels along Essex Street with a stop located at the Project site (between Pitman Street and Elm Place). Commuter Rail services are provided to the Town by way of Swampscott Station on the Newburyport/Rockport Line, which provides service to North Station in Boston. Swampscott Station located off Railroad Avenue approximately 0.3 miles to the southwest of the Project site, or an approximate 4 to 5 minute walking distance. In addition to fixed-route bus services, the MBTA operates The Ride paratransit services for eligible persons who cannot use fixed-route transit all or some of the time due to a physical, cognitive or mental disability in compliance with the ADA.

In an effort to encourage the use of alternative modes of transportation to single-occupant vehicles, the following Transportation Demand Management (TDM) measures will be implemented as a part of the Project:

- A transportation coordinator will be designated for the Project to coordinate the elements of the TDM program;
- Information regarding public transportation services, maps, schedules and fare information will be posted in a central location and/or otherwise made available to residents;
- A "welcome packet" will be provided to residents detailing available public transportation services, bicycle and walking alternatives, and commuter options available;
- Work-at-home workspaces will be provided to support telecommuting by residents of the Project;
- Pedestrian accommodations will be incorporated into the Project and consist of connections to existing sidewalks and ADA compliant wheelchair ramps at all pedestrian crossings that are to be constructed or modified as a part of the Project;
- > An internal mail room will be provided within the building; and
- Secure bicycle parking will be provided within the Project site.

With implementation of the aforementioned recommendations, safe and efficient access will be provided to the Project site and the Project can be accommodated within the confines of the existing and improved transportation system.

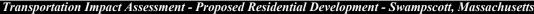
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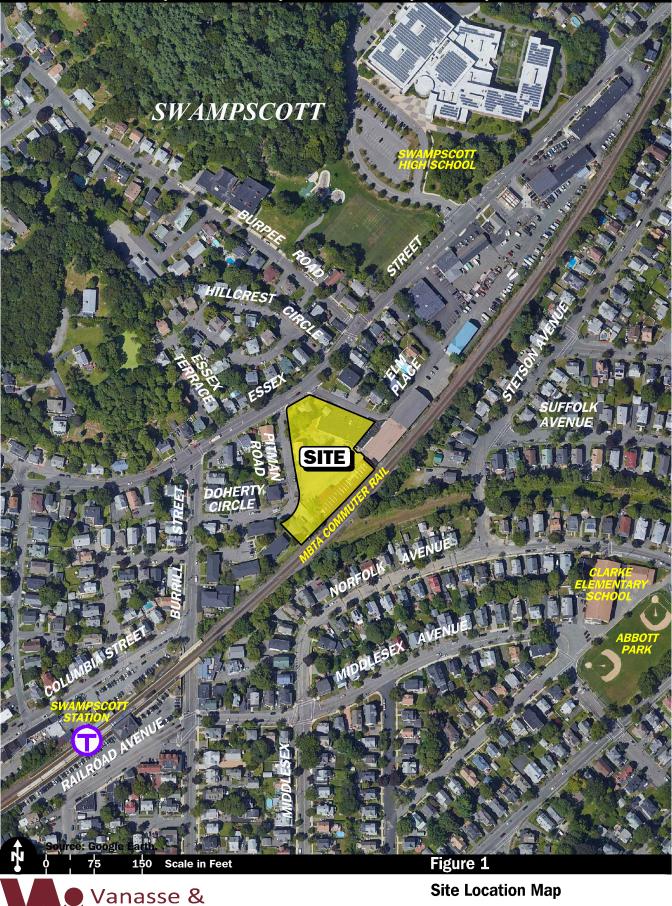
# PROJECT DESCRIPTION

The Project will entail the construction of a 128-unit, multifamily residential community to be known as Elm Place and located off Pitman Road and Elm Place in Swampscott, Massachusetts. The Project site encompasses approximately  $2.20\pm$  acres of land that is bounded by Essex Street to the north; railroad tracks associated with the Massachusetts Bay Transportation Authority (MBTA) Commuter Rail and a commercial building to the south; Elm Place to the east and Pitman Road to the west. Figure 1 depicts the Project site location in relation to the existing roadway network. The Project site is currently occupied by a multifamily residential building that fronts along Essex Street, a single-family home that fronts along Pitman Road, a commercial building and associated parking areas and appurtenances that will be removed to accommodate the Project.

Access to the Project site will be provided by way of a new driveway that will intersect the west side of Elm Place approximately 190 feet south of Essex Street, the approximate location of the south access to the existing parking lot in the eastern portion of the Project site. Off-street parking will be provided for 109 vehicles, or a parking ratio of 0.85 spaces per unit, which is below the parking requirements of Section 3.1.0.0, *Off-Street Parking and Loading, Parking*, of the Town of Swampscott Zoning By-Laws (1.5 parking spaces per unit is required); however the parking ratio is within the range of values documented by the Institute of Transportation Engineers (ITE) for similar multifamily residential communities.<sup>3</sup>

<sup>&</sup>lt;sup>3</sup>*Parking Generation Manual*, 5<sup>th</sup> Edition; Institute of Transportation Engineers; Washington D.C.; 2019. Observed parking demand ratios for a multifamily housing (mid-rise) residential community were found to range from 0.58 to 2.50 spaces per dwelling unit, with an average parking demand of 1.21 spaces per dwelling unit and an 85<sup>th</sup> percentile peak parking demand of 1.52 spaces per dwelling unit.





Associates inc

#### **STUDY METHODOLOGY**

This study was prepared in consultation with the Town of Swampscott and the Massachusetts Department of Transportation (MassDOT); was performed in accordance with MassDOT's *Transportation Impact Assessment (TIA) Guidelines* and the standards of the Traffic Engineering and Transportation Planning professions for the preparation of such reports; and was conducted in three distinct stages.

The first stage involved an assessment of existing conditions in the study area and included an inventory of roadway geometrics; pedestrian and bicycle facilities; on-street parking; public transportation services; observations of traffic flow; and collection of pedestrian, bicycle and vehicle counts.

In the second stage of the study, future traffic conditions were projected and analyzed. Specific travel demand forecasts for the Project were assessed along with future traffic demands due to expected traffic growth independent of the Project. A seven-year time horizon from the date of publication of this assessment was selected for analyses consistent with MassDOT's *Transportation Impact Assessment (TIA) Guidelines*. The traffic analysis conducted in stage two identifies existing or projected future roadway capacity, traffic safety, and site access issues.

The third stage of the study presents and evaluates measures to address traffic and safety issues, if any, identified in stage two of the study.

A comprehensive field inventory of existing conditions within the study area was conducted in December 2020, with traffic volume data adjusted following the guidance issued by MassDOT for TIAs conducted during the COVID-19 pandemic and the Governor's phased "Reopening Massachusetts" strategy.<sup>4</sup> The field investigation consisted of an inventory of existing roadway geometrics; pedestrian and bicycle facilities; public transportation services; traffic volumes; and operating characteristics; as well as posted speed limits and land use information within the study area. The study area that was assessed for the Project consisted of Essex Street, Elm Place and Pitman Road, and the following specific intersections: Essex Street at Burrill Street; Essex Street at Pitman Road; Essex Street at Elm Place; and Essex Street at Burpee Road.

The following describes the study area roadways and intersections.

#### **Roadways**

#### **Essex Street**

- > Two-lane urban minor arterial roadway under Town jurisdiction
- Traverses study area in a general northeast-southwest alignment between Paradise Road and Central Square in Lynn
- Provides two 12 to 26 foot wide travel lanes that are separated by a double-yellow centerline with no marked shoulder and additional travel lanes at major intersections
- > The posted speed limit is 30 miles per hour (mph)
- Sidewalks are provided along both sides of the roadway
- > On-street parking is prohibited within the study area
- Illumination is provided by way of street lights mounted on wood poles
- Land use within the study area consists of the Project site, and residential and commercial properties

<sup>&</sup>lt;sup>4</sup>Guidance on Traffic Count Data; MassDOT; revised April 2020.

### Elm Place

- > Two-lane local access roadway under Town jurisdiction
- Traverses study area in a circuitous alignment intersecting the south side of Essex Street southwest of Hillcrest Circle and northeast of Burpee Road
- Provides an approximate 20-foot wide traveled way (paved area) with no marked centerline or shoulders
- > A posted speed limit is not provided
- Sidewalks are provided along both sides of the Essex Place (south) for a distance of approximately 50-feet southeast of Essex Street
- > Illumination is provided by way of street lights mounted on wood poles
- Land use within the study area consists of the Project site and residential and commercial properties

### Pitman Road

- > Two-lane local access roadway under Town jurisdiction
- Traverses study area in a general north-south alignment terminating at 10 Pitman Road, approximately 350 feet south of Essex Street
- Provides an approximate 20-foot wide traveled way (paved area) with no marked centerline or shoulders
- A posted speed limit is not provided
- Sidewalks are provided along both sides of the roadway
- > Illumination is provided by way of street lights mounted on wood poles
- Land use within the study area consists of the Project site and residential and commercial properties

#### **Intersections**

Table 1 and Figure 2 summarize existing lane use, traffic control, and pedestrian and bicycle accommodations at the study area intersections as observed in December 2020.

# Table 1STUDY AREA INTERSECTION DESCRIPTION

Intersection	Traffic Control Type <sup>a</sup>	No. of Travel Lanes Provided	Shoulder Provided? (Yes/No/Width)	Pedestrian Accommodations? (Yes/No/Description)	Bicycle Accommodations? (Yes/No/Description)
Essex St./ Burrill St.	TS	1 left-turn lane and 1through lane on Essex St. southbound and 1 general purpose travel lane on Essex St. northbound and Burrill St.	No	Yes; both sides of the intersecting roadways; crosswalks provided across all legs; pedestrian traffic signal equipment and phasing (exclusive) provided	Yes; Shared traveled- way <sup>b</sup>

See notes at end of Table.

# Table 1 (Continued) STUDY AREA INTERSECTION DESCRIPTION

Intersection	Traffic Control Type <sup>a</sup>	No. of Travel Lanes Provided	Shoulder Provided? (Yes/No/Width)	Pedestrian Accommodations? (Yes/No/Description)	Bicycle Accommodations? (Yes/No/Description)
Essex St./ Pitman Rd.	S 1 general purpose travel lane on all approaches		No	Yes, both sides of the intersecting roadways; crosswalk provided across Pitman Rd.	Yes; Shared traveled- way
Essex St./ Elm Pl. (South)	S	1 general purpose travel lane on all approaches	No	Yes, both sides of the intersecting roadways; crosswalk provided across Elm Pl.	Yes; Shared traveled- way
Essex St./ Burpee Rd. TS TS TS TS TS TS TS TS TS TS		No	Yes; both sides of the intersecting roadways; crosswalks provided across all legs; pedestrian traffic signal equipment and phasing (exclusive) provided	Yes; Shared traveled- way	

<sup>a</sup>TS = Traffic Signal Control; S = STOP-sign control.

<sup>b</sup>Combined shoulder and travel lane width equal to or exceed 14 feet.

# TRAFFIC VOLUMES

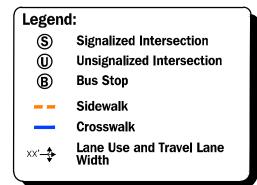
In order to determine existing traffic-volume demands and flow patterns within the study area, automatic traffic recorder (ATR) counts, manual turning movement counts (TMCs) and vehicle classification counts were completed in December 2020. The ATR counts were conducted on December 2<sup>nd</sup> and 3<sup>rd</sup>, 2020 (Wednesday through Thursday, inclusive) on Essex Street in the vicinity of the Project site in order to record weekday traffic conditions over an extended period, with weekday morning (7:00 to 9:00 AM) and evening (4:00 to 6:00 PM) peak period manual TMCs performed at the study intersections on December 2, 2020 (Wednesday). These time periods were selected for analysis purposes as they are representative of the peak-traffic-volume hours for both the Project and the adjacent roadway network.

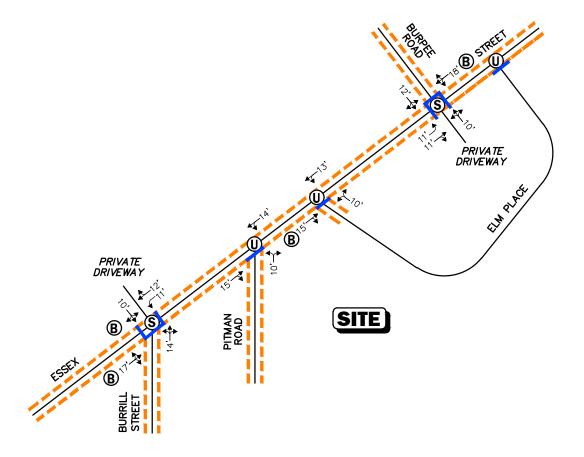
#### **Traffic-Volume Adjustments**

In order to evaluate the potential for seasonal fluctuation of traffic volumes within the study area, traffic volume data from MassDOT Continuous Count Station No. 8087 located on Lee Burbank Highway (Route 1A) in Revere were reviewed.<sup>5</sup> Based on a review of this data, it was determined that traffic volumes for the month of December are approximately 3.0 percent <u>below</u> average-month conditions. As such, the December traffic volumes were adjusted upward by 3.0 percent in order to be representative of average-month conditions.

In order to account for the impact on traffic volumes and trip patterns resulting from the "safer-at-home" order and the phased "Reopening Massachusetts" plan that was issued by the Governor on May 18, 2020, in response to the COVID-19 pandemic, the traffic-volume data collected from MassDOT Continuous Count Station No. 8087 in December 2020 was compared to

<sup>&</sup>lt;sup>5</sup>MassDOT Traffic Volumes for the Commonwealth of Massachusetts; 2020.







# Figure 2

Existing Intersection Lane Use, Travel Lane Width, and Pedestrian Facilities traffic data collected at the same station in December 2018. The 2018 traffic volumes were expanded to 2020 by applying a background traffic growth rate of 1.25 percent per year (discussion follows) in order to allow for a comparison of the data. Based on this pre and post-COVID-19 traffic count data comparison, the 2020 traffic-volume data that was collected as a part of this assessment was adjusted upward by an additional 18.4 percent in order to account for the reduced traffic volumes resulting from the phased "Reopening Massachusetts" plan.

The 2020 Existing traffic volumes are summarized in Table 2, with the weekday morning and evening peak-hour traffic volumes graphically depicted on Figure 3. Note that the peak-hour traffic volumes presented in Table 2 were obtained from the TMCs and are reflected on the aforementioned figure.

# Table 22020 EXISTING TRAFFIC VOLUMES

Location/Peak Hour	AWT <sup>a</sup>	<b>VPH</b> <sup>b</sup>	K Factor <sup>c</sup>	Directional Distribution <sup>d</sup>
<i>Essex Street, east of Elm Place</i> Weekday Morning (7:30 – 8:30 AM) Weekday Evening (4:30 – 5:30 PM)	16,155	 984 1,390	6.1 8.6	55.4% NEB 53.3% SWB

<sup>a</sup>Average weekday traffic in vehicles per day.

<sup>b</sup>Vehicles per hour.

<sup>e</sup>Percent of daily traffic occurring during the peak hour.

<sup>d</sup>Percent traveling in peak direction.

SWB=southwestbound; NEB=northeastbound.

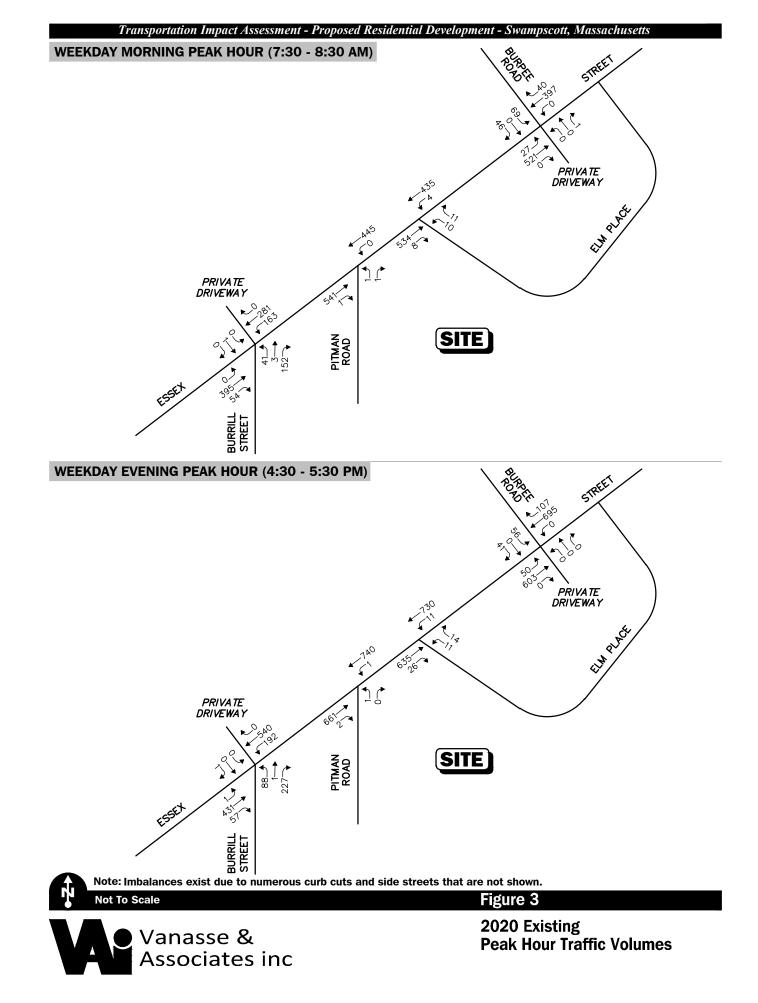
As can be seen in Table 2, Essex Street in the vicinity of the Project site was found to accommodate approximately 16,155 vehicles on an average weekday (two-way, 24-hour volume), with approximately 984 vehicles per hour (vph) during the weekday morning peak-hour and 1,390 vph during the weekday evening peak-hour.

# PEDESTRIAN AND BICYCLE FACILITIES

A comprehensive field inventory of pedestrian and bicycle facilities within the study area was undertaken in December 2020. The field inventory consisted of a review of the location of sidewalks and pedestrian crossing locations along the study roadways and at the study area intersections. As detailed on Figure 2, sidewalks are generally provided along both sides of the study area roadways, with crosswalks provided at the study area intersections and pedestrian traffic signal equipment and phasing provide at the signalized study area intersections.

Formal bicycle facilities were not identified within the study area; however, the study area roadways were identified to generally provide sufficient width to accommodate bicycle travel in a shared traveled-way configuration (i.e., bicyclists and motor vehicles sharing the traveled-way).<sup>6</sup>

<sup>&</sup>lt;sup>6</sup>A minimum combined travel lane and paved shoulder width of 14-feet is required to support bicycle travel in a shared traveled-way condition.



#### PUBLIC TRANSPORTATION

Public transportation services are provided within the study area by the Massachusetts Bay Transportation Transit Authority (MBTA), including fixed-route bus services and Commuter Rail. MBTA bus Route 455, *Salem Depot – Wonderland*, provides service between Salem Depot and Wonderland Station, where connections can be made to the Blue Line subway system and other MBTA bus routes. The Route 455 bus travels along Essex Street with a stop located at the Project site (between Pitman Street and Elm Place). Commuter Rail services are provided to the Town by way of Swampscott Station on the Newburyport/Rockport Line, which provides service to North Station in Boston. Swampscott Station located off Railroad Avenue approximately 0.3 miles to the southwest of the Project site, or an approximate 4 to 5 minute walking distance. In addition to fixed-route bus services, the MBTA operates The Ride paratransit services for eligible persons who cannot use fixed-route transit all or some of the time due to a physical, cognitive or mental disability in compliance with the Americans with Disabilities Act (ADA).

The public transportation schedules and fare information are provided in the Appendix.

### SPOT SPEED MEASUREMENTS

Vehicle travel speed measurements were performed on Essex Street in the vicinity of the Project site in conjunction with the ATR counts. Table 3 summarizes the vehicle travel speed measurements.

	Essex Street				
	Northeastbound	Southwestbound			
Mean Travel Speed (mph)	23	25			
85 <sup>th</sup> Percentile Speed (mph)	28	28			
Posted Speed Limit (mph)	30	30			

# Table 3VEHICLE TRAVEL SPEED MEASUREMENTS

mph = miles per hour.

As can be seen in Table 3, the mean vehicle travel speed along Essex Street in the vicinity of the Project site was found to be 23 mph northeastbound and 25 southwestbound. The measured 85<sup>th</sup> percentile vehicle travel speed, or the speed at which 85 percent of the observed vehicles traveled at or below, was found to be 28 mph in both directions which approximates the posted speed limit in the vicinity of the Project site (30 mph). The 85<sup>th</sup> percentile speed is used as the basis of engineering design and in the evaluation of sight distances, and is often used in establishing posted speed limits.

### MOTOR VEHICLE CRASH DATA

Motor vehicle crash information for the study area intersections was provided by the MassDOT Highway Division Safety Management/Traffic Operations Unit for the most recent fiveyear period available (2013 through 2017, inclusive) in order to examine motor vehicle crash trends occurring within the study area. The data is summarized by intersection, type, severity, roadway and weather conditions, and day of occurrence, and presented in Table 4.

As can be seen in Table 4, no (0) motor vehicle crashes were reported to have occurred at the Essex Street/Pitman Road intersection, with the remaining study area intersections found to have experienced an average of approximately one (1) reported motor vehicle crash per year over the five-year review period and were identified to have motor vehicle crash rates <u>below</u> the MassDOT statewide and District average crash rates for a signalized or unsignalized intersection, as appropriate, for the MassDOT Highway Division District in which the intersections are located (District 4). The majority of the reported crashes within the study area occurred on a weekday; during daylight; under clear weather conditions; and involved angle-type collisions that resulted in property damage only.

A review of the MassDOT statewide High Crash Location List indicated that there are no locations within the study area that are included on MassDOT's Highway Safety Improvement Program (HSIP) listing as a high crash location. In addition, no fatal motor vehicle crashes were reported to have occurred at the study area intersections over the five-year review period.

The detailed MassDOT Crash Rate Worksheets are provided in the Appendix.

## Table 4 MOTOR VEHICLE CRASH DATA SUMMARY<sup>a</sup>

	Essex St./ Burrill St.	Essex St./ Pitman Rd.	Essex St./ Elm Pl.	Essex St./ Burpee Rd.
Traffic Control Type: <sup>b</sup>	TS	U	U	TS
Year:				
2013	1	0	0	1
2014	0	0	0	1
2015	0	0	1	2
2016	2	0	1	0
2017	<u>0</u>	<u>0</u>	<u>0</u>	$\frac{3}{7}$
Total	3	0	2	7
Average	0.60	0.00	0.40	1.40
Rate <sup>c</sup>	0.10	0.00	0.07	0.22
MassDOT Crash Rate: <sup>d</sup>	0.78/0.73	0.57/0.57	0.57/0.57	0.78/0.73
Significant? <sup>e</sup>	No	No	No	No
Type:				
Angle	1	0	0	4
Rear-End	0	0	1	3
Head-On	0	0	0	0
Sideswipe	0	0	0	0
Fixed Object	0	0	0	0
Pedestrian/Bicycle	1	0	0	0
Unknown/Other	<u>1</u>	<u>0</u>	<u>1</u>	<u>0</u>
Total	3	0	2	7
Conditions:				
Clear	0	0	2	3
Cloudy	1	0	0	2
Rain	1	0	0	1
<u>Snow/Ice</u>	<u>1</u>	<u>0</u>	<u>0</u>	<u>1</u>
Total	3	0	2	7
Lighting:				_
Daylight	2	0	2	5
Dawn/Dusk	0	0	0	0
Dark (Road Lit)	1	0	0	2
Dark (Road Unlit)	$\frac{0}{2}$	$\frac{0}{0}$	$\frac{0}{2}$	$\frac{0}{7}$
Total	3	0	2	7
Day of Week:	2	<u>^</u>	2	4
Monday through Friday	2	0	2	4
Saturday	1	0	0	1
Sunday	$\frac{0}{2}$	$\frac{0}{2}$	$\frac{0}{2}$	$\frac{2}{7}$
Total	3	0	2	7
Severity:		<u>^</u>	2	-
Property Damage Only	1	0	2	5
Personal Injury	2	0	0	2
<u>Fatality</u>	$\frac{0}{2}$	$\underline{0}$	$\frac{0}{2}$	$\frac{0}{7}$
Total	3	0	2	7

<sup>a</sup>Source: MassDOT Safety Management/Traffic Operations Unit records, 2013 through 2017. <sup>b</sup>Traffic Control Type: TS = traffic signal; U = unsignalized. <sup>c</sup>Crash rate per million vehicles entering the intersection. <sup>d</sup>Statewide/District crash rate.

<sup>e</sup>The intersection crash rate is significant if it is found to exceed the MassDOT crash rate for the MassDOT Highway Division District in which the Project is located (District 4).

Traffic volumes in the study area were projected to the year 2028, which reflects a seven-year planning horizon from the date of publication of this assessment, consistent with MassDOT's *Transportation Impact Assessment (TIA) Guidelines*. Independent of the Project, traffic volumes on the roadway network in the year 2028 under No-Build conditions include all existing traffic and new traffic resulting from background traffic growth. Anticipated Project-generated traffic volumes superimposed upon the 2028 No-Build traffic volumes reflect 2028 Build traffic volume conditions with the Project.

# FUTURE TRAFFIC GROWTH

Future traffic growth is a function of the expected land development in the immediate area and the surrounding region. Several methods can be used to estimate this growth. A procedure frequently employed estimates an annual percentage increase in traffic growth and applies that percentage to all traffic volumes under study. The drawback to such a procedure is that some turning volumes may actually grow at either a higher or a lower rate at particular intersections.

An alternative procedure identifies the location and type of planned development, estimates the traffic to be generated, and assigns it to the area roadway network. This procedure produces a more realistic estimate of growth for local traffic; however, potential population growth and development external to the study area would not be accounted for in the resulting traffic projections.

To provide a conservative analysis framework, both procedures were used, the salient components of which are described below.

#### **Specific Development by Others**

The Town of Swampscott Community and Economic Development Department was consulted in order to determine if there were any projects planned within the study area that would have an impact on future traffic volumes at the study intersections. Based on this consultation, the following projects were identified for inclusion in this assessment:

Age-Qualified Residential Community, 35 Burpee Road, Swampscott, Massachusetts. This project will entail the construction of a 38-unit age-qualified residential community to be located at 35 Burpee Road. Overlook Acres Mixed-Use Development, 387 Highland Avenue, Salem Massachusetts. This project will entail the construction of a 282-unit multifamily residential community and an 8,450 square foot (sf) commercial building to be located at 387 Highland Avenue.

Traffic volumes associated with the aforementioned development projects by others were either estimated using trip-generation statistics published by the Institute of Transportation Engineers (ITE)<sup>7</sup> for the appropriate land uses or were obtained from the traffic study conducted for the specific development, and were assigned onto the study area roadway network based on existing traffic patterns where no other information was available. No other developments were identified at this time that are expected to result in an increase in traffic within the study area beyond the general background traffic growth rate.

# **General Background Traffic Growth**

Traffic-volume data compiled by MassDOT for Continuous Count Station No. 8087 in Revere was reviewed in order to determine general traffic growth trends in the area. Based on a review of this data and discussions with the Town of Swampscott Community and Economic Development Department, a 1.25 percent per year compounded annual background traffic growth rate was used in order to account for future traffic growth and presently unforeseen development within the study area.

# **Roadway Improvement Projects**

The Town of Swampscott and MassDOT were contacted in order to determine if there were any planned future roadway improvement projects expected to be complete by 2028 within the study area. Based on these discussions, no roadway improvement projects aside from routine maintenance activities were identified to be planned within the study area at this time.

# No-Build Traffic Volumes

The 2028 No-Build condition peak-hour traffic-volumes were developed by applying the 1.25 percent per year compounded annual background traffic growth rate to the 2020 Existing peak-hour traffic volumes and then adding the peak-hour traffic volumes associated with the identified specific development projects by others. The resulting 2028 No-Build weekday morning and evening peak-hour traffic volumes are shown on Figure 4.

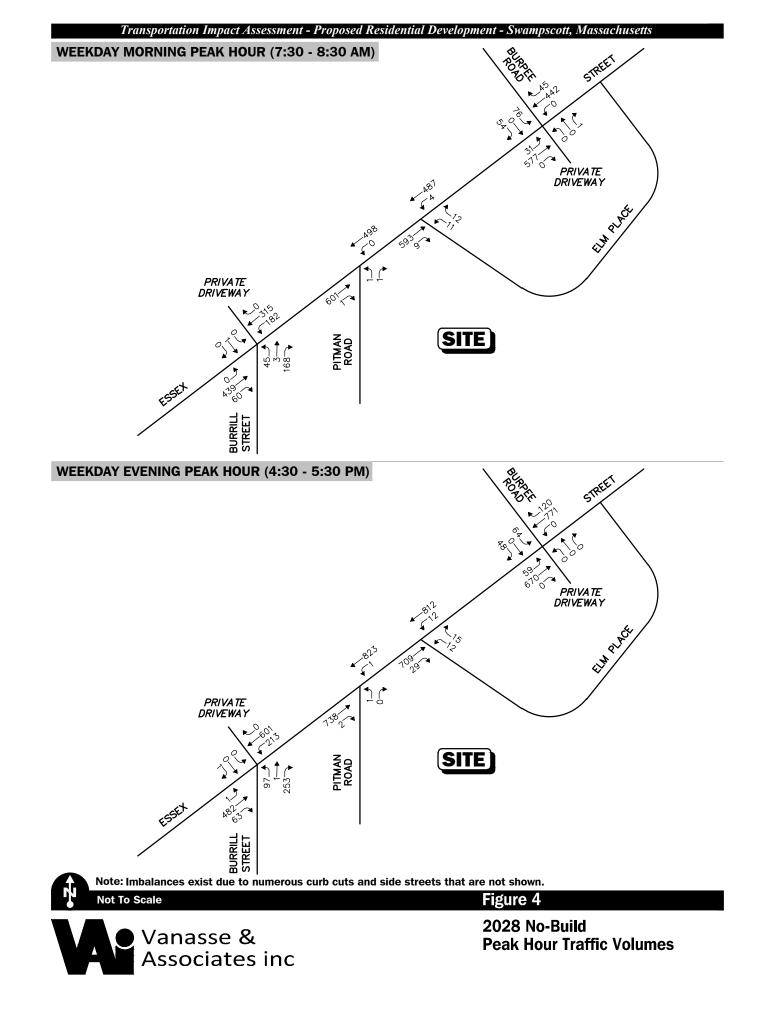
# PROJECT-GENERATED TRAFFIC

Design year (2028 Build) traffic volumes for the study area roadways were determined by estimating Project-generated traffic volumes and assigning those volumes on the study roadways. The following sections describe the methodology used to develop the anticipated traffic characteristics of the Project.

As proposed, the Project will entail the construction of a 128-unit multifamily residential community. In order to develop the traffic characteristics of the Project, trip-generation statistics published by the ITE<sup>8</sup> for a similar land use as that proposed were used. ITE Land Use Code (LUC)

<sup>7</sup>Ibid 1.

<sup>8</sup>Ibid 1.



221, *Multifamily Housing (Mid-Rise)*, was used to develop the base traffic characteristics of the Project.

## <u>Transit Use</u>

Given the availability of public transportation services to the Project site (MBTA commuter rail and fixed-route bus services), the interconnected network of sidewalks and on-road bicycle accommodations (study area roadway system generally provide sufficient width to accommodate bicycle travel in a shared traveled-way configuration), it is expected that a portion of the residents of the Project will use public transportation services, walk or bicycle, thereby reducing the volume of traffic that may be associated with the Project. In order to determine the proportion of residents of the Project that may use public transportation, walk or bicycle as their primary mode of transportation, travel mode data obtained from the 2019 American Community Survey (ACS) for the Town of Swampscott was reviewed. The ACS data identified the following commuting modes for workers age 16 or older that reside within the Town:

- Single-Occupant Vehicle: 68%
- ➤ Car/Vanpool: 7%
- > Public Transportation: 15%
- ➤ Walk: 2%
- ➢ Bicycle: 0%
- ➢ Other: 1%
- ➢ Worked at Home: 7%

According to the ACS, approximately 32 percent of workers that reside in the Town reported that they used an alternative mode of transportation to single-occupancy vehicles to travel to/from work, with 15 percent using public transportation, 7 percent participating in a car or vanpool, 2 percent walking/bicycling, 7 percent reporting that they work from home and 1 percent indicating an "other" mode.

In order to account for the use of alternative modes of transportation to single-occupancy vehicles, the base ITE trip-generation calculations were first converted to person trips using a vehicle occupancy ratio of 1.13 persons per vehicle, which was obtained from the 2009 National Household Travel Survey, and were then disseminated to the modes of transportation that are accessible to the residents of the Project: public transportation (transit), pedestrian/bicycle and automobile.

In order to provide conservative (high) traffic volume projections from which to assess the potential impact of the Project on the transportation infrastructure, it was assumed that 83 percent of the trips generated by the Project would consist of automobile trips, with 15 percent of trips assumed to be made using public transportation and 2 percent consisting of pedestrian/bicycle trips.

Table 5 summarizes the trip calculations for the Project using the above methodology.

# Table 5TRIP-GENERATION SUMMARY

		Person Trips				
			Automobile		Pedestrian/	_
	ITE	Total	Person	Transit	Bicycle	
	Vehicle	Person	Trips	Trips	Trips	Automobile
Time Period/Direction	Trips <sup>a</sup>	Trips <sup>b</sup>	(83%)	(15%)	(2%)	Trips <sup>c</sup>
Average Weekday Daily:						
Entering	348	393	326	59	8	289
Exiting	<u>348</u>	<u>393</u>	<u>326</u>	59	8	<u>289</u>
Total	696	786	652	118	$\frac{8}{16}$	578
Weekday Morning Peak Hour:						
Entering	11	13	11	2	0	10
Exiting	33	37	<u>30</u>	<u>6</u> 8	<u>1</u>	<u>26</u>
Total	<u>33</u> 44	$\frac{37}{50}$	$\frac{30}{41}$	8	1	$\frac{26}{36}$
Weekday Evening Peak Hour:						
Entering	34	38	31	6	1	27
Exiting	22	25	21	4	$\frac{0}{1}$	19
Total	<u>22</u> 56	<u>25</u> 63	$\frac{21}{52}$	10	1	<u>19</u> 46

<sup>a</sup>Based on ITE LUC 220, Multifamily Housing (Mid-Rise), 128-units.

<sup>b</sup>ITE vehicle trips x 1.13 persons per vehicle.

<sup>c</sup>Automobile person trips divided by 1.13.

#### **Project-Generated Traffic Volume Summary**

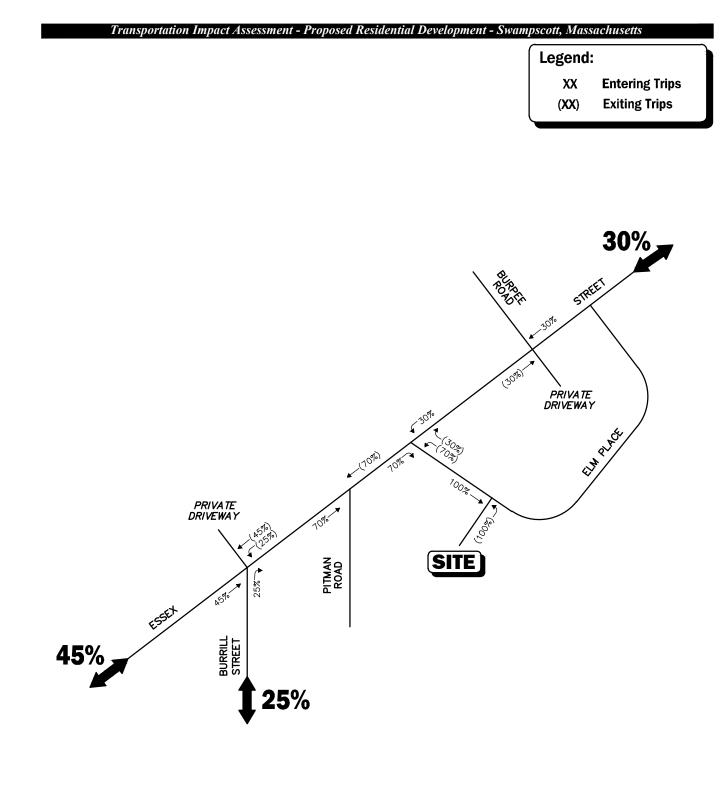
As can be seen in Table 5, using the aforementioned methodology, the Project is expected to generate approximately 578 automobile trips, 118 transit trips and 16 pedestrian/bicycle trips on an average weekday (two-way, 24-hour volumes), with 36 automobile trips (10 vehicles entering and 26 exiting), 8 transit trips and 1 pedestrian/bicycle trip expected during the weekday morning peakhour, and 46 automobile trips (27 vehicles entering and 19 exiting), 10 transit trips and 1 pedestrian/bicycle trip expected during the weekday evening peakhour.

# TRIP DISTRIBUTION AND ASSIGNMENT

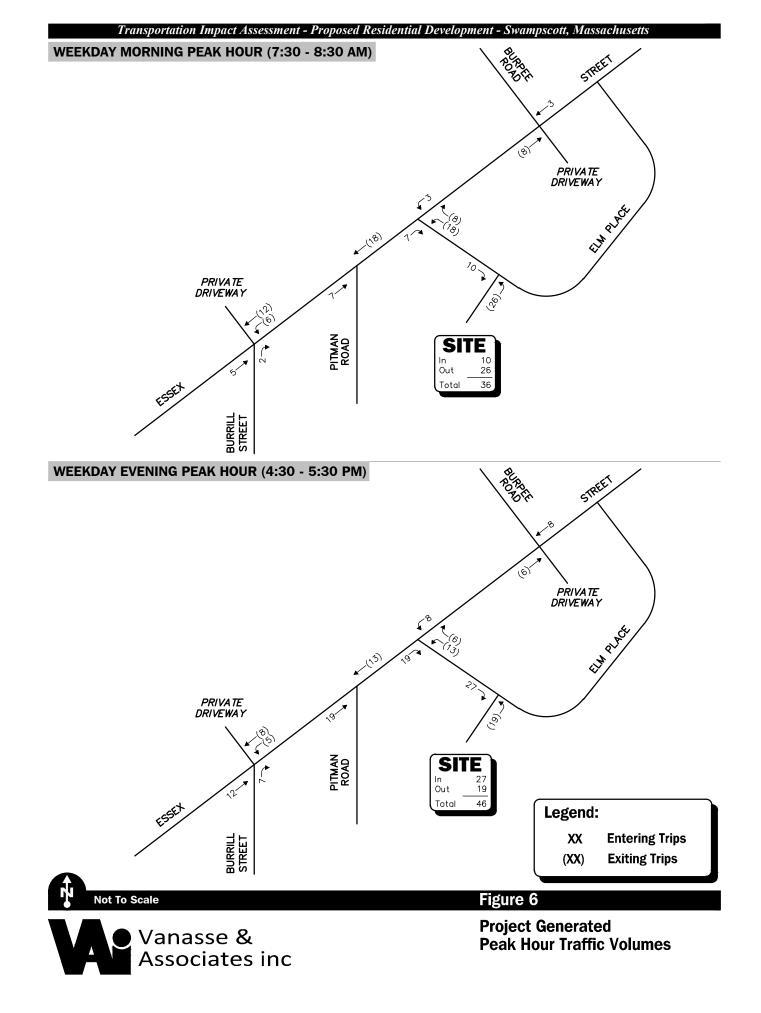
The directional distribution of generated trips to and from the Project site was determined based on a review of Journey-to-Work data obtained from the U.S. Census for persons residing in the Town of Swampscott and then refined based on existing traffic patterns within the study area. The general trip distribution for the Project is graphically depicted on Figure 5. The additional traffic expected to be generated by the Project was assigned on the study area roadway network as shown on Figure 6 for the weekday morning and evening peak hours.

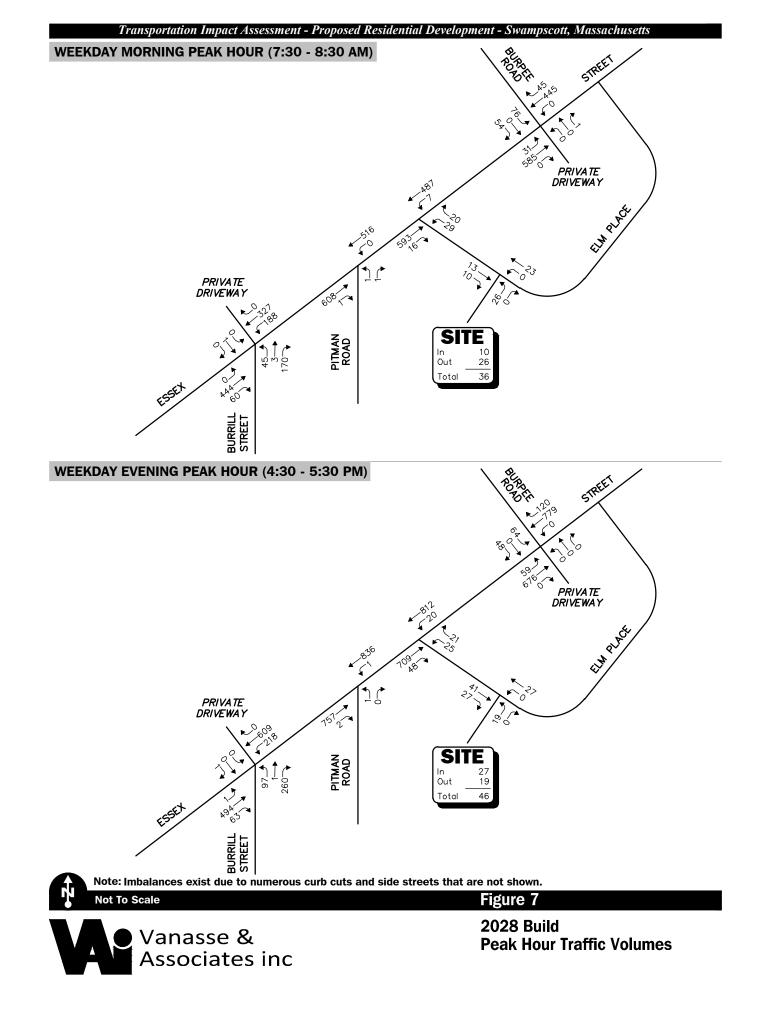
# **FUTURE TRAFFIC VOLUMES - BUILD CONDITION**

The 2028 Build condition traffic volumes consist of the 2028 No-Build traffic volumes with the additional traffic expected to be generated by the Project added to them. The 2028 Build weekday morning and evening peak-hour traffic-volumes are graphically depicted on Figure 7.









A summary of peak-hour projected traffic-volume changes outside of the study area that is the subject of this assessment is shown in Table 6. These changes are a result of the construction of the Project.

Location/Peak Hour	2020 Existing	2028 No-Build	2028 Build	Traffic Volume Increase Over No-Build	Percent Increase Over No-Build
Essex Street, northeast of Burpee Road:					
Weekday Morning	1,028	1,141	1,152	11	1.0
Weekday Evening	1,461	1,625	1,639	14	0.9
Essex Street, southwest of Burrill Street:					
Weekday Morning	771	859	876	17	2.0
Weekday Evening	1,118	1,245	1,265	20	1.6
Burrill Street, south of Essex Street:					
Weekday Morning	414	459	467	8	1.7
Weekday Evening	565	627	639	12	1.9

# Table 6PEAK-HOUR TRAFFIC-VOLUME INCREASES

As shown in Table 6, Project-related traffic-volume increases outside of the study area relative to 2028 No-Build conditions are anticipated to range from 0.9 to 2.0 percent during the peak periods, with vehicle increases shown to range from 8 to 20 vehicles. *When distributed over the peak-hour, the predicted traffic volume increases would not result in a significant impact (increase) on motorist delays or vehicle queuing outside of the immediate study area that is the subject of this assessment.* 

Measuring existing and future traffic volumes quantifies traffic flow within the study area. To assess quality of flow, roadway capacity and vehicle queue analyses were conducted under Existing, No-Build and Build traffic volume conditions. Capacity analyses provide an indication of how well the roadway facilities serve the traffic demands placed upon them, with vehicle queue analyses providing a secondary measure of the operational characteristics of an intersection or section of roadway under study.

# **METHODOLOGY**

# Levels of Service

A primary result of capacity analyses is the assignment of level of service to traffic facilities under various traffic-flow conditions.<sup>9</sup> The concept of level of service is defined as a qualitative measure describing operational conditions within a traffic stream and their perception by motorists and/or passengers. A level-of-service definition provides an index to quality of traffic flow in terms of such factors as speed, travel time, freedom to maneuver, traffic interruptions, comfort, convenience, and safety.

Six levels of service are defined for each type of facility. They are given letter designations from A to F, with level-of-service (LOS) A representing the best operating conditions and LOS F representing congested or constrained operating conditions.

Since the level of service of a traffic facility is a function of the traffic flows placed upon it, such a facility may operate at a wide range of levels of service, depending on the time of day, day of week, or period of year.

<sup>&</sup>lt;sup>9</sup>The capacity analysis methodology is based on the concepts and procedures presented in the *Highway Capacity Manual;* Transportation Research Board; Washington, DC; 2010.

#### **Signalized Intersections**

The six levels of service for signalized intersections may be described as follows:

- LOS A describes operations with very low control delay; most vehicles do not stop at all.
- *LOS B* describes operations with relatively low control delay. However, more vehicles stop than LOS A.
- *LOS C* describes operations with higher control delays. Individual cycle failures may begin to appear. The number of vehicles stopping is significant at this level, although many still pass through the intersection without stopping.
- *LOS D* describes operations with control delay in the range where the influence of congestion becomes more noticeable. Many vehicles stop and individual cycle failures are noticeable.
- *LOS E* describes operations with high control delay values. Individual cycle failures are frequent occurrences.
- LOS F describes operations with high control delay values that often occur with oversaturation. Poor progression and long cycle lengths may also be major contributing causes to such delay levels.

Levels of service for signalized intersections are calculated using the operational analysis methodology of the 2000 Highway Capacity Manual and implemented as a part of the Synchro® 10 software as recommended by MassDOT. This method assesses the effects of signal type, timing, phasing, and progression; vehicle mix; and geometrics on delay. Level-of-service designations are based on the criterion of control or signal delay per vehicle. Control or signal delay is a measure of driver discomfort, frustration, and fuel consumption, and includes initial deceleration delay approaching the traffic signal, queue move-up time, stopped delay and final acceleration delay. Table 7 summarizes the relationship between level of service and control delay. The tabulated control delay criterion may be applied in assigning level-of-service designations to individual lane groups, to individual intersection approaches, or to entire intersections.

# Table 7LEVEL-OF-SERVICE CRITERIAFOR SIGNALIZED INTERSECTIONS<sup>a</sup>

Level of Service	Control (Signal) Delay Per Vehicle (Seconds)
А	<10.0
В	10.1 to 20.0
С	20.1 to 35.0
D	35.1 to 55.0
Е	55.1 to 80.0
F	>80.0

<sup>a</sup>Source: *Highway Capacity Manual*, Transportation Research Board; Washington, DC; 2000; page 16-2.

#### **Unsignalized Intersections**

The six levels of service for unsignalized intersections may be described as follows:

- LOS A represents a condition with little or no control delay to minor street traffic.
- LOS B represents a condition with short control delays to minor street traffic.
- LOS C represents a condition with average control delays to minor street traffic.
- LOS D represents a condition with long control delays to minor street traffic.
- *LOS E* represents operating conditions at or near capacity level, with very long control delays to minor street traffic.
- LOS F represents a condition where minor street demand volume exceeds capacity of an approach lane, with extreme control delays resulting.

The levels of service of unsignalized intersections are determined by application of a procedure described in the 2010 *Highway Capacity Manual*.<sup>10</sup> Level of service is measured in terms of average control delay. Mathematically, control delay is a function of the capacity and degree of saturation of the lane group and/or approach under study and is a quantification of motorist delay associated with traffic control devices such as traffic signals and STOP signs. Control delay, queue move-up time, and final acceleration delay from a stopped condition. Definitions for level of service at unsignalized intersections are also given in the 2010 *Highway Capacity Manual*. Table 8 summarizes the relationship between level of service and average control delay for two-way stop controlled and all-way stop controlled intersections.

# Table 8LEVEL-OF-SERVICE CRITERIA FORUNSIGNALIZED INTERSECTIONS<sup>a</sup>

Level-Of-Service by V	Average Control Delay	
$v/c \le 1.0$	$v/c \le 1.0$ $v/c > 1.0$	
А	F	<10.0
В	F	10.1 to 15.0
С	F	15.1 to 25.0
D	F	25.1 to 35.0
E	F	35.1 to 50.0
F	F	>50.0

<sup>a</sup>Source: *Highway Capacity Manual*; Transportation Research Board; Washington, DC; 2010; page 19-2.

<sup>&</sup>lt;sup>10</sup>*Highway Capacity Manual*; Transportation Research Board; Washington, DC; 2010.

#### Vehicle Queue Analysis

Vehicle queue analyses are a direct measurement of an intersection's ability to process vehicles under various traffic control and volume scenarios and lane use arrangements. The vehicle queue analysis was performed using the Synchro® intersection capacity analysis software which is based upon the methodology and procedures presented in the 2010 *Highway Capacity Manual*. The Synchro® vehicle queue analysis methodology is a simulation based model which reports the number of vehicles that experience a delay of six seconds or more at an intersection. For signalized intersections, Synchro® reports both the average (50<sup>th</sup> percentile) the 95<sup>th</sup> percentile vehicle queue. For unsignalized intersections, Synchro® reports the 95<sup>th</sup> percentile vehicle queue. Vehicle queue lengths are a function of the capacity of the movement under study and the volume of traffic being processed by the intersection during the analysis period. The 95<sup>th</sup> percentile vehicle queue is the vehicle queue length that will be exceeded only 5 percent of the time, or approximately three minutes out of sixty minutes during the peak one hour of the day (during the remaining fifty-seven minutes, the vehicle queue length will be less than the 95<sup>th</sup> percentile queue length).

# ANALYSIS RESULTS

Level-of-service and vehicle queue analyses were conducted for 2020 Existing, 2028 No-Build and 2028 Build conditions for the intersections within the study area. The results of the intersection capacity and vehicle queue analyses are summarized in Tables 9 and 10, with the detailed analysis results presented in the Appendix.

The following is a summary of the level-of-service and vehicle queue analyses for the intersections within the study area. For context, we note that an LOS of "D" or better is generally defined as "acceptable" operating conditions.

As can be seen in Tables 9 and 10, *the study area intersections were shown to continue to operate under acceptable conditions (LOS "D" or better) with the addition of Project-related traffic.* Project-related impacts at the study area intersections were identified as follows:

*Essex Street at Burrill Street* – No change in LOS over No-Build conditions, with Project-related impacts defined as a predicted increase in overall average motorist delay of less than 1.0 seconds and in vehicle queuing of up to one (1) vehicle.

*Essex Street at Burpee Road* – No change in LOS over No-Build conditions, with Project-related impacts defined as a predicted increase in overall average motorist delay of less than 1.0 seconds.

*Essex Street at Pitman Road* – Operating conditions for all movements from the Pitman Road approach to Essex Street were shown to degrade from LOS B to LOS C during the weekday morning peak-hour with the addition of Project-related traffic as a result of a slight increase in average motorist delay (approximately 0.4 seconds). Vehicle queuing at the intersection was shown to be negligible.

*Essex Street at Elm Place (South)* – Operating conditions for all movements from the Elm Place approach to Essex Street were shown to degrade from LOS C to LOS D during the weekday evening peak-hour with the addition of Project-related traffic as a result of an increase in average motorist delay of 9.5 seconds. Vehicle queuing on the Elm Place approach were shown to increase by up to two (2) vehicles with the addition of Project-related traffic. All movements along Essex Street

approaching the intersection were shown to operate at LOS A during both the weekday morning and evening peak hours with negligible vehicle queuing predicted.

*Elm Place at the Project Site Driveway* – All movements at this intersection were shown to operate at LOS A under all analysis conditions with negligible vehicle queuing expected.

# Table 9 SIGNALIZED INTERSECTION LEVEL-OF-SERVICE AND VEHICLE QUEUE SUMMARY

Signalized Intersection/ Peak-Hour/Movement	2020 Existing				2028 No-Build				2028 Build			
	V/C <sup>a</sup>	Delay <sup>b</sup>	LOS <sup>c</sup>	Queue <sup>d</sup> 50 <sup>th</sup> /95 <sup>th</sup>	V/C	Delay	LOS	Queue 50 <sup>th</sup> /95 <sup>th</sup>	V/C	Delay	LOS	Queue 50 <sup>th</sup> /95 <sup>th</sup>
Essex Street at Burrill Street												
Weekday Morning:												
Essex Street NEB LT/TH/RT	0.72	23.2	С	5/18	0.76	25.3	С	6/22	0.77	25.9	С	6/23
Essex Street SWB LT	0.50	11.2	В	1/5	0.56	13.1	В	1/8	0.57	13.4	В	1/8
Essex Street SWB TH/RT	0.33	8.2	А	2/9	0.36	8.3	А	2/10	0.37	8.4	А	2/10
Burrill Street NB LT/TH/RT	0.43	26.3	С	1/5	0.52	29.5	С	2/6	0.52	29.7	С	2/6
Driveway SB LT/TH/RT	0.22	38.7	D	0/0	0.21	40.4	D	0/0	0.21	40.6	D	0/0
Overall		18.0	В			19.7	В			19.9	В	
Weekdav Evening:												
Essex Street NEB LT/TH/RT	0.81	31.7	С	4/7	0.83	34.0	С	9/25	0.84	34.0	С	9/25
Essex Street SWB LT	0.58	17.0	В	0/0	0.65	19.4	В	2/8	0.66	19.7	В	2/8
Essex Street SWB TH/RT	0.62	15.5	B	9/17	0.67	16.7	B	6/21	0.67	16.7	B	7/22
Burrill Street NB Lt/TH/RT	0.69	31.2	Ē	2/4	0.80	40.0	D	6/16	0.83	43.1	D	6/16
Driveway SB LT/TH/RT	0.00	38.6	D	0/0	0.00	41.6	D	0/0	0.00	42.1	D	0/0
Overall		24.3	B			27.7	Ċ			28.5	Ċ	
Essex Street at Burpee Road												
Weekday Morning:												
Essex Street NEB LT	0.11	6.8	А	0/1	0.14	7.5	А	0/2	0.14	7.5	А	0/2
Essex Street NEB TH/RT	0.71	12.3	В	3/18	0.76	14.8	В	3/22	0.76	14.8	В	3/22
Essex Street SWB LT/TH/RT	0.50	8.7	А	2/12	0.54	9.8	А	2/14	0.53	9.8	А	2/14
Driveway NB LT/TH/RT	0.00	23.3	С	0/0	0.00	26.6	С	0/0	0.00	27.1	С	0/0
Burpee Road SB LT/TH/RT	0.27	19.2	В	1/2	0.30	20.8	Ċ	1/3	0.31	21.5	Ċ	1/3
Overall		11.6	B			13.5	B			13.5	B	
Weekday Evening:		1110	2			1010	2			1010	2	
Essex Street NEB LT	0.28	5.1	А	0/2	0.38	5.8	А	0/3	0.39	6.0	А	0/3
Essex Street NEB TH/RT	0.63	7.2	A	3/15	0.67	7.5	A	3/21	0.67	7.6	A	3/21
Essex Street SWB LT/TH/RT	0.66	7.4	A	3/22	0.69	7.8	A	4/27	0.70	7.9	A	4/27
Driveway NB LT/TH/RT	0.00	0.0	A	0/0	0.00	0.0	A	0/0	0.00	0.0	A	0/0
Burpee Road SB LT/TH/RT	0.06	20.4	C	0/0	0.10	23.7	C	0/0	0.10	23.7	C	0/0
Overall	0.00	8.0	Ă			8.5	Ă			8.6	Ă	0.2

<sup>a</sup>Volume-to-capacity ratio. <sup>b</sup>Control (signal) delay per vehicle in seconds.

<sup>c</sup>Level-of-Service. <sup>d</sup>Queue length in vehicles.

NB = northbound SB = southbound; NEB = northeastbound; SWB = southwestbound; LT = left-turning movements; TH = through movements; RT = right-turning movements.

## Table 10 UNSIGNALIZED INTERSECTION LEVEL-OF-SERVICE AND VEHICLE QUEUE SUMMARY

		2020 Ex	isting			2028 No	-Build			2028 E	Build	
Unsignalized Intersection/Peak-hour/Movement	Demand <sup>a</sup>	Delay <sup>b</sup>	LOS <sup>c</sup>	Queue <sup>d</sup> 95 <sup>th</sup>	Demand	Delay	LOS	Queue 95 <sup>th</sup>	Demand	Delay	LOS	Queu 95 <sup>th</sup>
ssex Street at Pitman Road												
Weekday Morning:												
Essex Street NEB TH/RT	542	0.0	Α	0	602	0.0	А	0	609	0.0	А	0
Essex Street SWB LT/TH	445	0.0	Α	0	498	0.0	А	0	516	0.0	А	0
Pitman Road NB LT/RT	2	13.8	В	0	2	14.7	В	0	2	15.1	С	0
Weekday Evening:												
Essex Street NEB TH/RT	663	0.0	Α	0	740	0.0	А	0	759	0.0	А	0
Essex Street SWB LT/TH	741	0.0	А	0	824	0.0	А	0	837	0.0	А	0
Pitman Road NB LT/RT	1	20.2	С	0	1	23.6	С	0	1	24.3	С	0
ssex Street at Elm Place (South)												
Weekday Morning:												
Essex Street NEB TH/RT	542	0.0	А	0	602	0.0	А	0	609	0.0	А	0
Essex Street SWB LT/TH	439	0.1	А	0	491	0.1	А	0	494	0.3	А	0
Elm Place NB LT/RT	21	14.0	В	1	23	15.3	С	1	49	18.4	С	2
Weekday Evening:												
Essex Street NEB TH/RT	661	0.0	А	0	738	0.0	А	0	757	0.0	А	0
Essex Street SWB LT/TH	741	0.4	А	0	824	0.5	А	0	832	0.9	А	0
Elm Place NB LT/RT	25	19.2	C	1	27	23.1	C	1	46	32.6	D	3
Im Place at the Project Site Driveway												
Weekday Morning:												
Project Site Driveway EB LT/RT									26	8.8	А	0
Elm Place NB LT/TH									23	0.0	А	0
Elm Place SB TH/RT									23	0.0	А	0
Weekday Evening:												
Project Site Driveway EB LT/RT									19	9.0	А	0
Elm Place NB LT/TH									27	0.0	А	0
Elm Place SB TH/RT									68	0.0	А	0

<sup>a</sup>Demand in vehicles per hour. <sup>b</sup>Average control delay per vehicle (in seconds). <sup>c</sup>Level-of-Service.

<sup>d</sup>Queue length in vehicle.

NB = northbound; SB = southbound; NEB = northeastbound; SWB = southwestbound; LT = left-turning movements; TH = through movements; RT = right-turning movements.

An evaluation of potential opportunities to re-establish a connection between Pitman Road and Doherty Circle as either a pedestrian/bicycle connection or to serve vehicles and pedestrians/bicycles was undertaken. Providing a connection between these roadways would facilitate pedestrian and bicycle access to Burrill Street and the Swampscott Commuter Rail Station, and would afford an alternative for motorists to travel between Burrill Street and Essex Street east of Pitman Road. At a baseline level, a pedestrian/bicycle connection should be formalized irrespective of advancement of a vehicular connection.

Three (3) alternatives were evaluated for a vehicular connection between Pitman Road and Doherty Circle:

- 1. Pitman Road to Doherty Circle one-way southbound;
- 2. Doherty Circle to Pitman Road one-way northbound; and
- 3. Pitman Road/Doherty Circle two-way circulation

In order to ascertain the diversion of traffic that may occur as a result of the connection, a review of the projected future 2028 Build traffic volumes and the traffic operations analysis for the Essex Street/Burrill Street and Essex Street/Pitman Road intersections was undertaken. The traffic operations analyses were reviewed to determine the available capacity of the Essex Street/Pitman Road intersection to accommodate the additional traffic demands resulting from the connection. Once the theoretical capacity of the intersection is reached, motorists would not use the connection as the travel time benefit that would be achieved over the use of the Essex Street/Burrill Street intersection is diminished.

The following summarizes the assessment of each of the three vehicular connection alternatives.

*Pitman Road to Doherty Circle One-Way Southbound* – Based on a review of the projected vehicle queuing at the Essex Street/Burrill Street intersection, it is estimated that up to 55 vehicles from the Essex Street southwestbound approach to Burrill Street would likely divert to Pitman Road and use Doherty Circle to access to Burrill Street during the weekday peak hours with a one-way southbound connector.

**Doherty Circle to Pitman Street One-Way Northbound** – Based on a review of the predicted vehicle queuing on the Burrill Street northbound approach to Essex Street at the Essex Street/ Burrill Street intersection, it is estimated that up to 90 vehicles from the Burrill Street northbound

approach could divert to Doherty Circle to access to Essex Street during the weekday peak hours with a one-way northbound connector.

*Pitman Road/Doherty Circle Two-Way Circulation* – Based on a review of the predicted vehicle queuing at the Essex Street/Burrill Street intersection, it is estimated that up to 145 vehicles could divert from the Essex Street/Burrill Street intersection to a two-way Pitman Road/Doherty Circle Connection during the weekday peak hours.

Based on a review of the vehicular connection alternatives, the one-way southbound connection that would allow motorists from Essex Street east of Burrill Street to access Burrill Street would appear to be the least impactful to the existing uses along Pitman Road and Doherty Circle. The one-way connection can be limited to the short segment connecting the two roadways. To the extent that such a vehicular connection is advanced, it is recommended that the connection be 20-feet in width, or the minimum width required to accommodate emergency vehicle travel, and the appropriate signs ("One-Way" and "Do Not Enter") be installed to regulate the one-way traffic flow.

## SIGHT DISTANCE EVALUATION

Sight distance measurements were performed at the intersection of Essex Street at Elm Place (south) and at the Project site driveway intersection with Elm Place in accordance with MassDOT and American Association of State Highway and Transportation Officials (AASHTO)<sup>11</sup> requirements. Both stopping sight distance (SSD) and intersection sight distance (ISD) measurements were performed. In brief, SSD is the distance required by a vehicle traveling at the design speed of a roadway, on wet pavement, to stop prior to striking an object in its travel path. ISD or corner sight distance (CSD) is the sight distance required by a driver entering or crossing an intersecting roadway to perceive an on-coming vehicle and safely complete a turning or crossing maneuver with on-coming traffic. In accordance with AASHTO standards, if the measured ISD is at least equal to the required SSD value for the appropriate design speed, the intersection can operate in a safe manner. Table 11 presents the measured SSD and ISD at the subject intersections.

<sup>&</sup>lt;sup>11</sup>A Policy on Geometric Design of Highway and Streets, 7<sup>th</sup> Edition; American Association of State Highway and Transportation Officials (AASHTO); Washington D.C.; 2018.

### Table 11 SIGHT DISTANCE MEASUREMENTS<sup>a</sup>

		Feet	
Intersection/Sight Distance Measurement	Required Minimum (SSD)	Desirable (ISD) <sup>b</sup>	Measured
Essex Street at Elm Place (south)			
Stopping Sight Distance:			
Essex Street approaching from the north	200		500+
Essex Street approaching from the south	200		500+
Intersection Sight Distance:			
Looking to the north from Essex Street	200	335	236
Looking to the south from Essex Street	200	290	500+
Elm Place at the Project Site Driveway			
Stopping Sight Distance:			
Elm Place approaching from the north	80		147°
Elm Place approaching from the east	80		95
Intersection Sight Distance:			
Looking to the north from the Site Driveway	80	145	147°
Looking to the east from the Site Driveway	80	170	95

<sup>a</sup>Recommended minimum values obtained from *A Policy on Geometric Design of Highways and Streets*, 7<sup>th</sup> Edition; American Association of State Highway and Transportation Officials (AASHTO); 2018; and based on a 30 mph approach speed along Essex Street and a 15 mph approach speed along Elm Place.

<sup>b</sup>Values shown are the intersection sight distance for a vehicle turning right or left exiting a roadway under STOP control such that motorists approaching the intersection on the major street should not need to adjust their travel speed to less than 70 percent of their initial approach speed.

°Clear line of sight is provided to/from Essex Street.

As can be seen in Table 11, the available lines of sight exceed the recommended minimum sight distances to function in a safe manner (SSD) based on a 30 mph approach speed along Essex Street, which is consistent with the posted speed limit and the measured 85<sup>th</sup> percentile vehicle travel speed (28 mph) approaching Elm Place, and a 15 mph approach speed along Elm Place, which is reflective of a reasonable approach speed given the geometry of the roadway(horizontal curvature) and proximity of the Project site access to Essex Street.

## CONCLUSIONS

VAI has conducted a TIA in order to determine the potential impacts on the transportation infrastructure associated with the proposed construction of a multifamily residential community to be known as Elm Place and located off Pitman Road and Elm Place in Swampscott, Massachusetts. The following specific areas have been evaluated as they relate to the Project: i) access requirements; ii) potential off-site improvements; and iii) safety considerations; under existing and future conditions, both with and without the Project. Based on this assessment, we have concluded the following with respect to the Project:

- Using trip-generation statistics published by the ITE<sup>12</sup> and with adjustment to account for the use of public transportation and pedestrian and bicycle trips, the Project is expected to generate approximately 578 automobile trips, 118 transit trips and 16 pedestrian/bicycle trips on an average weekday (two-way, 24-hour volumes), with 36 automobile trips, 8 transit trips and 1 pedestrian/bicycle trip expected during the weekday morning peakhour, and 46 automobile trips, 10 transit trips and 1 pedestrian/bicycle trip expected during the weekday evening peak-hour;
- 2. The Project will not result in a significant impact (increase) on motorist delays or vehicle queuing over Existing or anticipated future conditions without the Project (No-Build conditions), with all movements at the study intersections shown to operate at LOS D or better under all analysis conditions, where an LOS of "D" or better is defined as "acceptable" traffic operations;
- 3. To the extent that a vehicular connection is advanced between Pitman Road and Doherty Circle, the connection should be one-way southbound and include pedestrian and bicycle accommodations;
- 4. No apparent safety deficiencies were noted with respect to the motor vehicle crash history at the study area intersections; and

<sup>&</sup>lt;sup>12</sup>Ibid 1.

5. Lines of sight to and from Elm Place (south) at its intersection with Essex Street and at the Project site driveway intersection with Elm Place were found to exceed the recommended minimum distance for safe operation based on the appropriate approach speed.

In consideration of the above, we have concluded that the Project can be accommodated within the confines of the existing transportation infrastructure in a safe and efficient manner with implementation of the recommendations that follow.

## **RECOMMENDATIONS**

A detailed transportation improvement program has been developed that is designed to provide safe and efficient access to the Project site and address any deficiencies identified at off-site locations evaluated in conjunction with this study. The following improvements have been recommended as a part of this evaluation and, where applicable, will be completed in conjunction with the Project subject to receipt of all necessary rights, permits, and approvals.

#### **Project Access**

Access to the Project site will be provided by way of a new driveway that will intersect the west side of Elm Place approximately 190 feet south of Essex Street, the approximate location of the south access to the existing parking lot in the eastern portion of the Project site. The following recommendations are offered with respect to the design and operation of the Project site access and internal circulation, many of which are reflected on the Site Plans:

- > The Project site driveway should be a minimum of 24 feet in width and designed to accommodate the turning and maneuvering requirements of the largest anticipated responding emergency vehicle.
- ➤ Where perpendicular parking is proposed, the drive aisle behind the parking should be a minimum of 23 feet in order to facilitate parking maneuvers.
- Vehicles exiting the Project site should be placed under STOP-sign control with a marked STOP-line provided.
- All signs and pavement markings to be installed within the Project site should conform to the applicable standards of the *Manual on Uniform Traffic Control Devices* (MUTCD).<sup>13</sup>
- Pedestrian connections have been provided to the sidewalks along both Essex Street and Pitman Road.
- Americans with Disabilities Act (ADA) compliant wheelchair ramps should be provided at all pedestrian crossings that are constructed or modified as a part of the Project.
- Signs and landscaping to be installed as a part of the Project within the intersection sight triangle areas should be designed and maintained so as not to restrict lines of sight.
- Snow windrows within sight triangle areas of the Project site driveway intersection should be promptly removed where such accumulations would impede sight lines.

<sup>&</sup>lt;sup>13</sup>Manual on Uniform Traffic Control Devices (MUTCD); Federal Highway Administration; Washington, D.C.; 2009.

> Bicycle parking should be provided at appropriate locations within the Project site.

#### Off Site

#### Essex Street/Elm Place (South)

Independent of the Project, it is recommended that a marked STOP-line be provided on the Elm Place (south) approach to Essex Street in order to defined the desired stopping point for vehicles prior to entering the intersection and the marked crosswalk. In addition, it is recommended that a double-yellow centerline be installed along Elm Place for a minimum distance of 50-feet from the STOP-line in order to separate the directions of travel approaching Essex Street and to provide proper positioning for vehicles queued on the Elm Place approach.

#### **Transportation Demand Management**

Public transportation services are provided within the study area by the MBTA, including fixedroute bus services and Commuter Rail. MBTA bus Route 455, *Salem Depot – Wonderland*, provides service between Salem Depot and Wonderland Station, where connections can be made to the Blue Line subway system and other MBTA bus routes. The Route 455 bus travels along Essex Street with a stop located at the Project site (between Pitman Street and Elm Place). Commuter Rail services are provided to the Town by way of Swampscott Station on the Newburyport/Rockport Line, which provides service to North Station in Boston. Swampscott Station located off Railroad Avenue approximately 0.3 miles to the southwest of the Project site, or an approximate 4 to 5 minute walking distance. In addition to fixed-route bus services, the MBTA operates The Ride paratransit services for eligible persons who cannot use fixed-route transit all or some of the time due to a physical, cognitive or mental disability in compliance with the ADA.

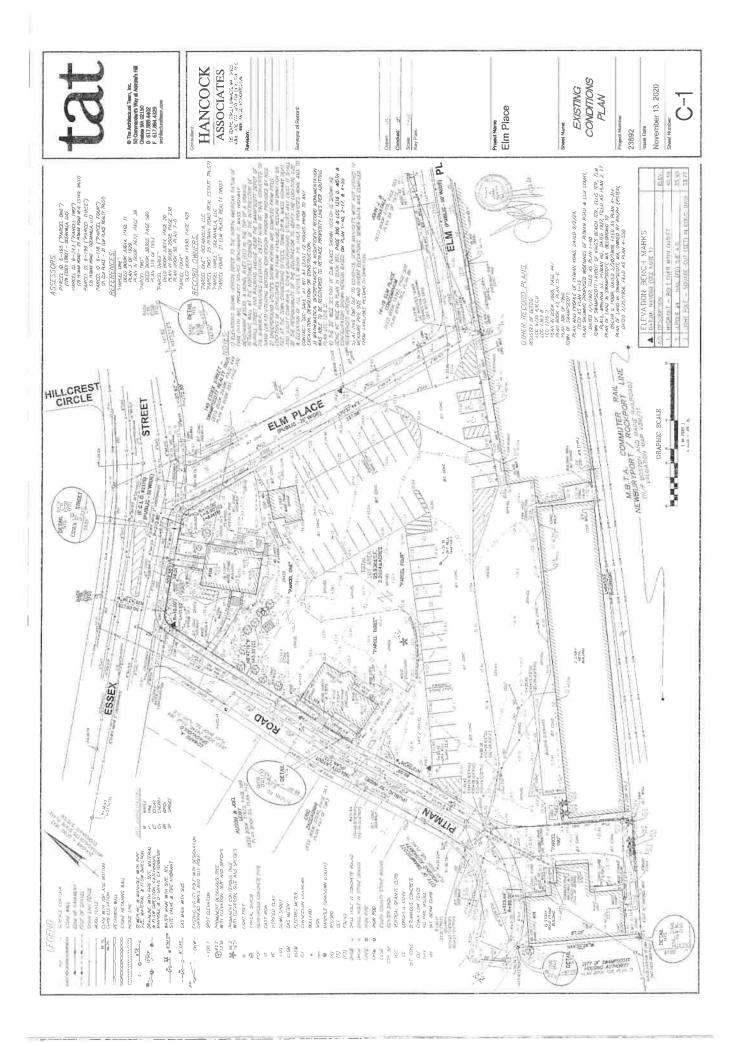
In an effort to encourage the use of alternative modes of transportation to single-occupant vehicles, the following Transportation Demand Management (TDM) measures will be implemented as a part of the Project:

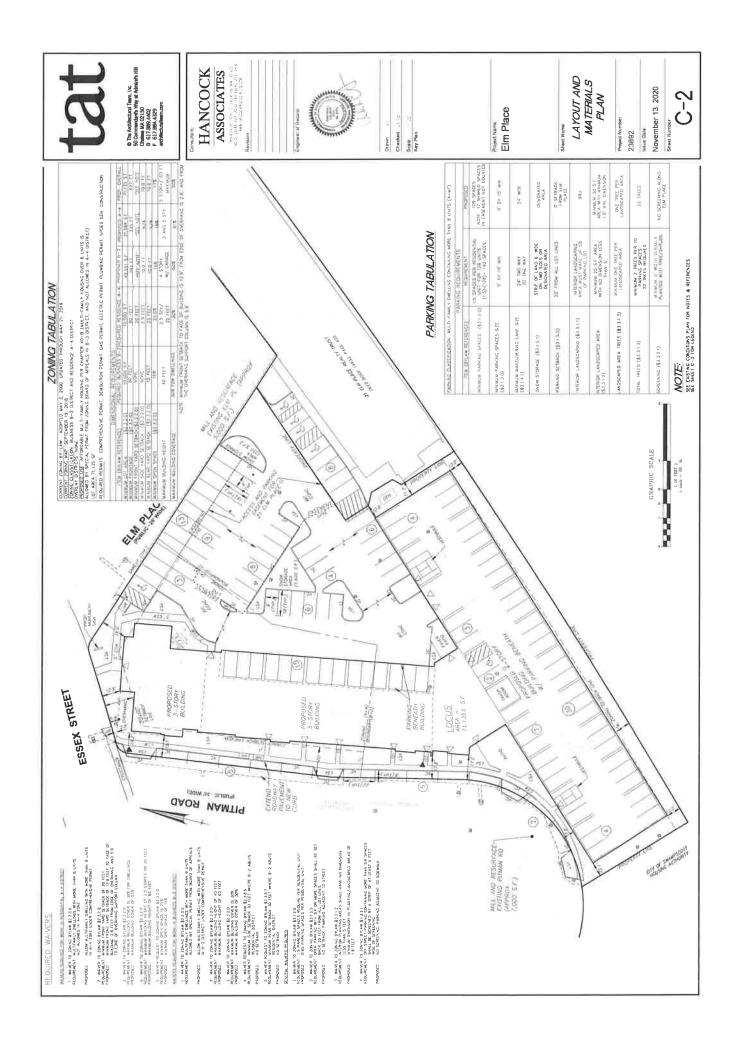
- A transportation coordinator will be designated for the Project to coordinate the elements of the TDM program;
- Information regarding public transportation services, maps, schedules and fare information will be posted in a central location and/or otherwise made available to residents;
- A "welcome packet" will be provided to residents detailing available public transportation services, bicycle and walking alternatives, and commuter options available;
- Work-at-home workspaces will be provided to support telecommuting by residents of the Project;
- Pedestrian accommodations will be incorporated into the Project and consist of connections to existing sidewalks and ADA compliant wheelchair ramps at all pedestrian crossings that are to be constructed or modified as a part of the Project;
- > An internal mail room will be provided within the building; and
- Secure bicycle parking will be provided within the Project site.

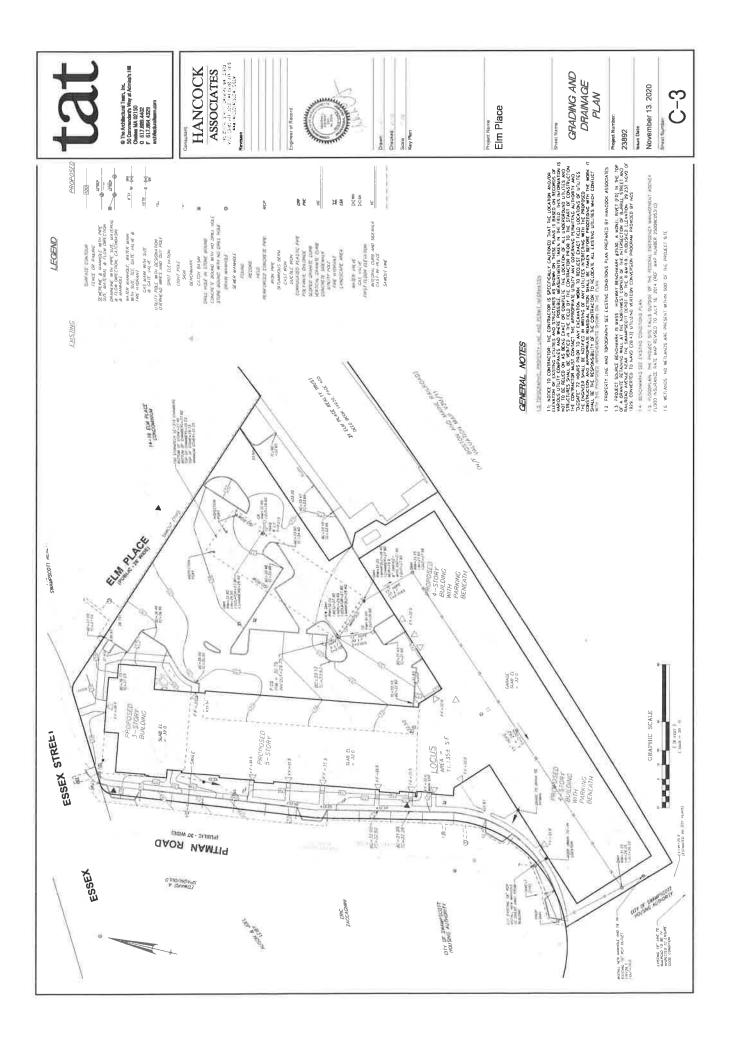
With implementation of the aforementioned recommendations, safe and efficient access will be provided to the Project site and the Project can be accommodated within the confines of the existing and improved transportation system.

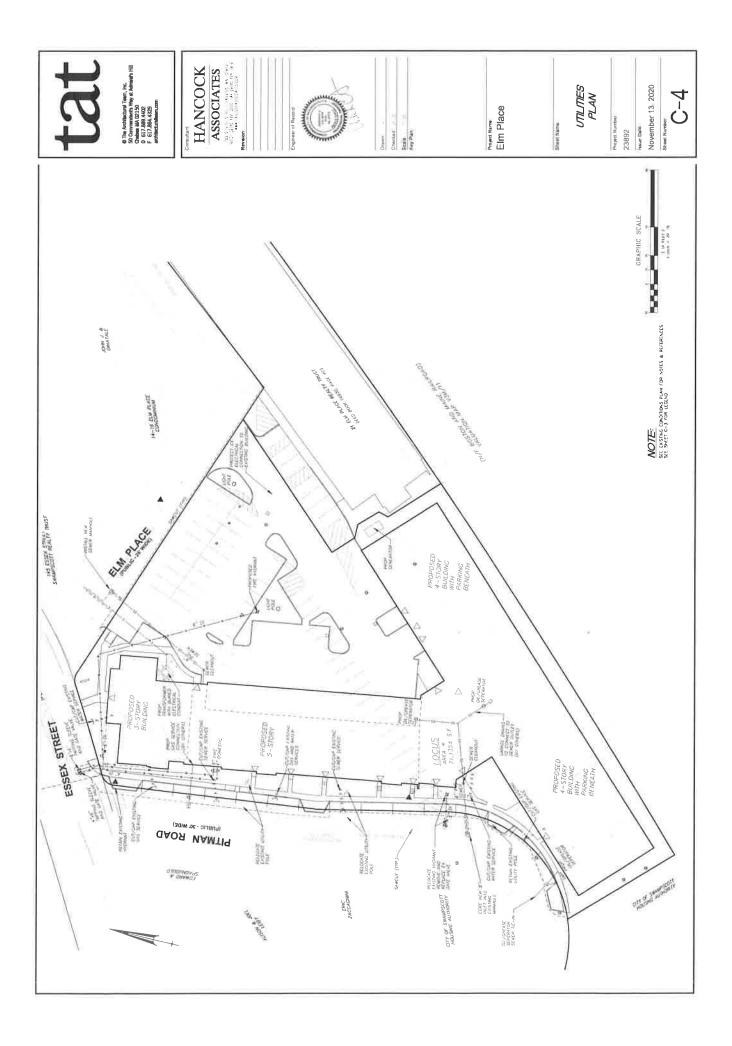
## APPENDIX

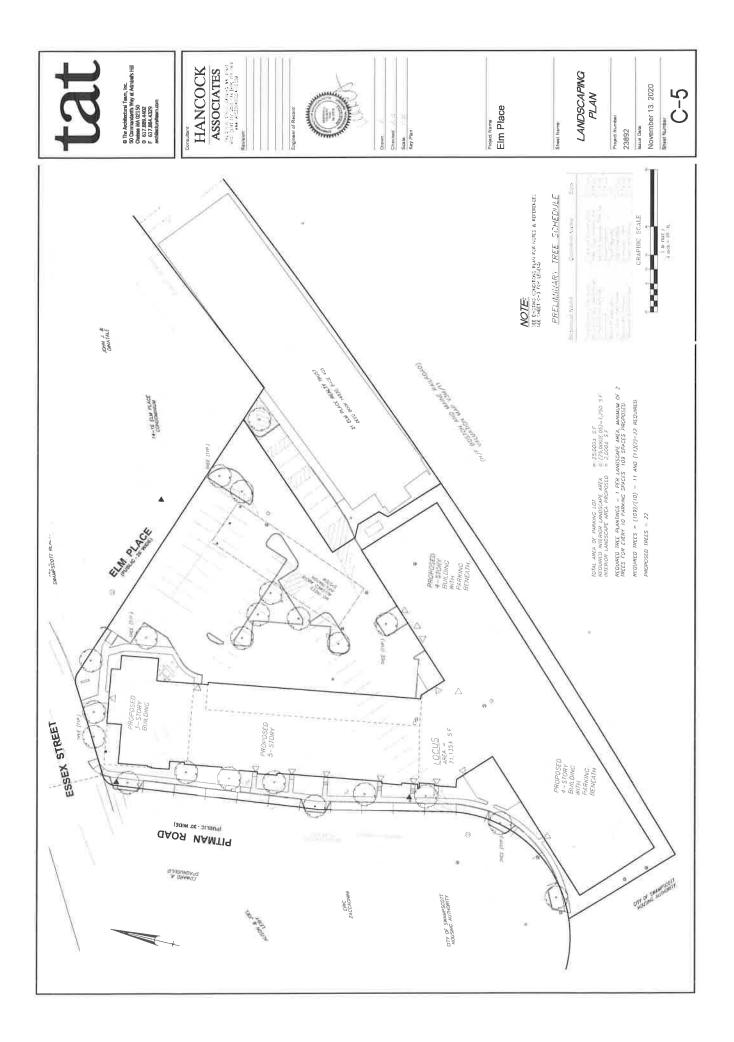
PROJECT SITE PLAN AUTOMATIC TRAFFIC RECORDER COUNT DATA MANUAL TURNING MOVEMENT COUNT DATA SEASONAL ADJUSTMENT DATA PUBLIC TRANSPORTATION INFORMATION VEHICLE TRAVEL SPEED DATA MASSDOT CRASH RATE WORKSHEETS AND HIGH CRASH LOCATION MAPPING BACKGROUND DEVELOPMENT TRAFFIC-VOLUME NETWORKS GENERAL BACKGROUND TRAFFIC GROWTH TRIP-GENERATION CALCULATIONS MODE OF TRANSPORTATION FOR THE TOWN OF SWAMPSCOTT JOURNEY TO WORK TRIP DISTRIBUTION CAPACITY ANALYSIS WORKSHEETS PROJECT SITE PLAN

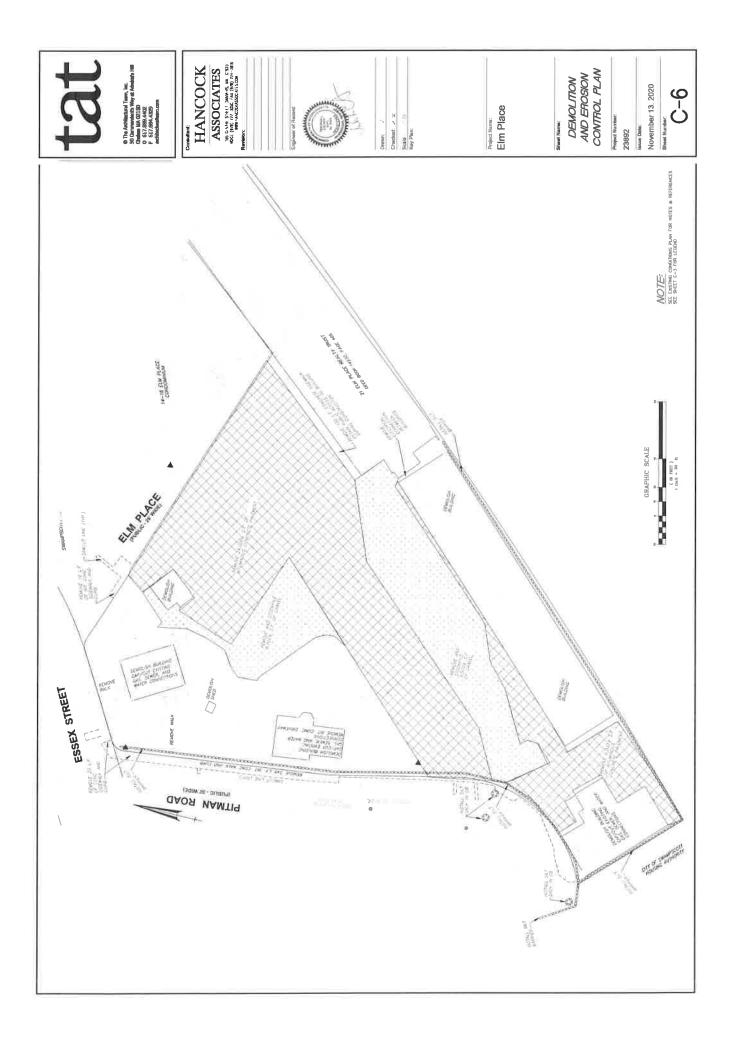












## AUTOMATIC TRAFFIC RECORDER COUNT DATA

	Combine	Totals		В		Totals	Hour	VB	M	12/2/2020	Start
Afternor	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Wed	Time
				114	11			105	10		12:00
				120	7			99	8		12:15
				128	15			107	6		12:30
8	77	473	48	111	15	414	29	103	5		12:45
				121	4			124	2		01:00
		1		112	4			130	1		01:15
				107	9	1		118	8		01:30
9	32	462	19	122	2 5	474	13	102	2		01:45
				109	5			134	5		02:00
				115	1			121	4		02:15
				86	0	1		132	1		02:30
9	19	434	7	124	1	499	12	112	2		02:45
				137	3			157	2		03:00
				116	2			160	4		03:15
				126	1			137	6		03:30
11	25	516	9	137	3	623	16	169	4		03:45
	_		-	130	7	010	10	136	5		04:00
				123	5			160	3		04:15
				110	13			152	7		04:30
11	67	518	46	155	21	608	21	160	6		04:45
	0.	0.0		119	11	000	21	163	13		04.45
				129	10			167	26		05:15
				109	23			126	12		
10	161	478	84	121	40	585	77	120	26		05:30
		110	01	93	36	505		125	40		05:45 06:00
				95	69			103	40		06:00
				92	82			87	49		00.10
7	477	371	281	91	94	403	196	88	43 64		06:30
,	477	0/1	201	87	96	403	190	80	63		06:45
				70	84			75	63		07:00
				75	114			61	65 87		07:15
5	739	294	422	62	128	288	047				07:30
5	155	294	422	51	126	200	317	72	102		07:45
				36	105			69	100		08:00
				36	89			61	81		08:15
3	736	169	399	46	09 104	040	007	41	86		08:30
5	130	109	299			210	337	39	70		08:45
				29	91			53	76		09:00
				25	91			36	77		09:15
2	COF	100	202	22	86	450		41	100		09:30
2	695	109	363	33	95	158	332	28	79		09:45
				20	96			25	74		10:00
				18	95			20	90		10:15
				10	114			18	115		10:30
1-	809	66	413	18	108	79	396	16	117		10:45
				19	82			26	85		11:00
				22	95			21	96		11:15
				12	97			10	98		11:30
1	815	67	406	14	132	66	409	9	130		11:45
83	4652			3957	2497			4407	2155		Total
64.3	35.7%			61.3%	38.7%			67.2%	32.8%		Percent

8688VL01

Start	12/3/2020		VB		Totals		B		Totals		ed Totals
Time	Thu	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoo
12:00		5	104			13	112				
12:15		9	112			9	147				
12:30		7	162			3	116				
12:45		9	128	30	506	7	103	32	478	62	98
01:00		3	106			2	116				
01:15		3	108			7	113				
01:15			100				126				
01:30		2	126		450	4		40	104	05	
01:45		1	110	9	450	3	106	16	461	25	91
02:00		4	111			2	135				
02:15		4	128			4	123				
02:30		1	146			1	133		- 1		
02:45		0	122	9	507	3	115	10	506	19	101
03:00		0	162			0	123				
03:15		1	167			0	118				
03:30		6	167			1	120				
03.30		7	195	14	691	10	133	11	494	25	118
03:45				14	091		100	11	434	20	110
04:00		3	144			7	135				
04:15		2	157			0	135				
04:30		4	161			8	133				
04:45		10	170	19	632	19	148	34	551	53	118
05:00		9	179			13	123				
05:15		29	153		1	15	140				
05:30		17	126			29	122				
05:45		29	117	84	575	44	132	101	517	185	109
06:00		35	113	04	0/0	33	107	101	0.11	100	
00.00			113			54					
06:15		44	103			04	113				
06:30		48	101			84	70				-
06:45		48	89	175	406	110	89	281	379	456	78
07:00		64	89			84	72				
07:15		66	83			93	61				
07:30		103	83			108	86				
07:45		100	66	333	321	118	46	403	265	736	58
08:00		105	57			148	62				
08:15		83	54			109	58				
08:30		75	56		1	109	43				
		109	46	372	213	124	42	490	205	862	41
08:45				372	213		42	490	205	002	-
09:00		99	48			101	43				
09:15		86	38			85	35				
09:30		90	40			103	25				
09:45		101	24	376	150	126	29	415	132	791	28
10:00		76	36			79	29				
10:15		92	26			102	37				
10:30		74	22			90	19				
10:45		103	16	345	100	103	21	374	106	719	20
11:00		80	16	0,0	,00	86	20	014			
						92	14				
11:15		88	24								
11:30		105	7			108	15			70.1	
11:45		107	8	380	55	125	8	411	57	791	11
Total		2146	4606			2578	4151			4724	875
Percent		31.8%	68.2%			38.3%	61.7%			35.0%	65.0
Grand											
Total		4301	9013			5075	8108			9376	1712
Percent		32.3%	67.7%			38.5%	61.5%			35.4%	64.6
Perceni											

Page 2

8688VL01

curate Counts	978-664-2565
PC CC	G

Location : Essex Street Location : East of Elm Place City/State: Swampscott, MA

Tue

11/30/2020 EB

MB

Start Time 12:00 AM

MB

AADT 13,248 ADT 13,248 Comb. Total ADT

13246

0

0

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13481

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Day

AM Peak Vol.

PM Peak Vol.

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# Page 1

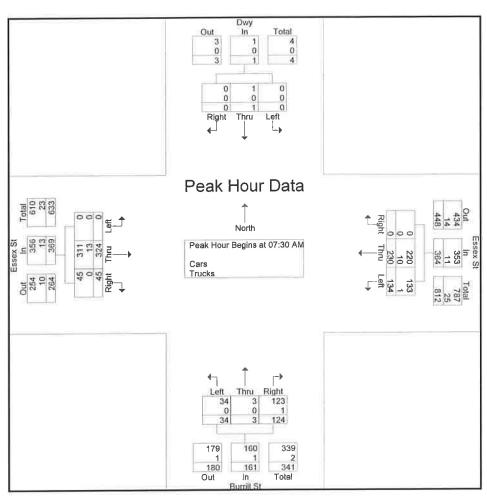
8688VL01

MANUAL TURNING MOVEMENT COUNT DATA

	Fre	Dwy om North			ssex St			Burrill St om South		Fr	ssex St om West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	0	0	0	16	48	0	4	0	22	0	73	5	168
07:15 AM	0	0	0	18	46	0	7	0	16	1	61	3	152
07:30 AM	0	0	0	30	51	0	4	1	23	0	87	12	208
07:45 AM	0	0	0	40	68	0	4	1	38	0	90	13	256
Total	0	0	0	104	213	0	21	2	99	1	311	33	784
08:00 AM	0	1	0	32	62	0	11	0	29	0	75	10	220
08:15 AM	õ	0	0	32	49	0	13	1	34	0	72	10	211
08:30 AM	Ō	0	0	36	49	0	13	0	31	0	53	8	190
08:45 AM	0	0	0	21	48	0	8	0	34	0	70	10	191
Total	0	1	0	121	208	0	45	1	128	0	270	38	812
Grand Total	0	1	0	225	421	0	66	3	227	1	581	71	1596
Apprch %	0	100	0	34.8	65.2	0	22.3	1	76.7	0.2	89	10.9	
Total %	0	0.1	0	14.1	26.4	0	4.1	0.2	14.2	0.1	36.4	4.4	
Cars	0	1	0	223	403	0	66	3	226	1	557	69	1549
% Cars	0	100	0	99.1	95.7	0	100	100	99.6	100	95.9	97.2	97.1
Trucks	0	0	0	2	18	0	0	0	1	0	24	2	47
% Trucks	0	0	0	0.9	4.3	0	0	0	0.4	0	4.1	2.8	2.9

		_	wy North				ex St 1 East				rill St South				ex St West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis Fron	n 07:00	AM to C	8:45 AM -	Peak 1	of 1											
Peak Hour for Er																	
07:30 AM	0	0	0	0	30	51	0	81	4	1	23	28	0	87	12	99	208
07:45 AM	0	0	0	0	40	68	0	108	6	1	38	45	0	90	13	103	256
08:00 AM	0	1	0	1	32	62	0	94	11	0	29	40	0	75	10	85	220
08:15 AM	0	0	0	0	32	49	0	81	13	1	34	48	0	72	10	82	211
Total Volume	0	1	0	1	134	230	0	364	34	3	124	161	0	324	45	369	895
% App. Total	0	100	0		36.8	63.2	0		21.1	1.9	77		0	87.8	12.2		
PHF	.000	.250	.000	.250	.838	.846	.000	.843	.654	.750	.816	.839	.000	.900	.865	.896	.874
Cars	0	1	0	1	133	220	0	353	34	3	123	160	0	311	45	356	870
% Cars	0	100	0	100	99.3	95.7	0	97_0	100	100	99.2	99.4	0	96.0	100	96.5	97.2
Trucks	õ	0	Ō	0	1	10	0	11	0	0	1	1	0	13	0	13	25
% Trucks	Ō	Ō	0	0	0.7	4.3	0	3_0	0	0	0.8	0,6	0	4.0	0	3.5	2.8

File Name	86880001
Site Code	86880001
Start Date	12/2/2020
Page No	2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

(	07 15 AM				07:45 AM				07:45 AM				07:30 AM			
+0 mins.	0	0	0	0	40	68	0	108	6	1	38	45	0	87	12	99
+15 mins.	0	0	0	0	32	62	0	94	11	0	29	40	0	90	13	103
+30 mins.	0	0	0	0	32	49	0	81	13	1	34	48	0	75	10	85
+45 mins.	0	1	0	1	36	49	0	85	13	0	31	44	0	72	10	82
otal Volume	0	1	0	1	140	228	0	368	43	2	132	177	0	324	45	369
6 App. Total	0	100	0		38	62	0		24.3	1.1	74.6		0	87.8	12.2	
PHF	.000	.250	.000	.250	.875	.838	.000	.852	.827	.500	.868	.922	.000	.900	.865	.896
Cars	0	1	0	1	139	217	0	356	43	2	131	176	0	311	45	356
% Cars	õ	100	0	100	99.3	95.2	0	96.7	100	100	99.2	99.4	0	96	100	96.5
Trucks	õ	0	Õ	0	1	11	0	12	0	0	1	1	0	13	0	13
% Trucks	õ	ñ	õ	õ	0.7	4.8	0	3.3	0	0	0.8	0.6	0	4	0	3.5

	Fn	Dwy om North			ssex St om East			urrill St om South			ssex St om West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Tota
07:00 AM	0	0	0	16	47	0	4	0	22	0	72	4	16
07:15 AM	Ō	Ō	0	18	44	0	7	0	16	1	58	3	14
07:30 AM	Õ	Ó	0	30	49	0	4	1	23	0	85	12	20
07:45 AM	Ō	Ō	0	40	68	0	6	1	38	0	84	13	25
Total	0	0	0	104	208	0	21	2	99	1	299	32	76
08:00 AM	0	1	0	31	59	0	11	0	28	0	74	10	21
08:15 AM	Õ	Ó	0	32	44	0	13	1	34	0	68	10	20
08:30 AM	Ō	0	0	36	46	0	13	0	31	0	50	8	18
08:45 AM	0	0	0	20	46	0	8	0	34	0	66	9	18
Total	0	1	0	119	195	0	45	1	127	0	258	37	7
Grand Total	0	1	0	223	403	0	66	3	226	1	557	69	154
Apprch %	õ	100	0	35.6	64.4	0	22.4	1	76.6	0.2	88.8	11	
Total %	õ	0.1	0	14.4	26	0	4.3	0.2	14.6	0.1	36	4.5	

		-	wy North				ex St 1 East				rill St South		Essex St From West				
Start Time	Left	Thru		App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis Fron	n 07:00	AM to C	8:45 AM	- Peak 1	of 1	77:										
Peak Hour for Er												2					
07:30 AM	0	0	Ō	0	30	49	0	79	4	1	23	28	0	85	12	97	204
07:45 AM	Ő	Ō	Ō	0	40	68	0	108	6	1	38	45	0	84	13	97	250
08:00 AM	Ō	1	Ō	1	31	59	0	90	11	0	28	39	0	74	10	84	214
08:15 AM	0	0	0	0	32	44	0	76	13	1	34	48	0	68	10	78	202
Total Volume	Ő	1	0	1	133	220	0	353	34	3	123	160	0	311	45	356	870
% App. Total	ő	100	ŏ		37.7	62.3	0		21.2	1.9	76.9		0	87.4	12.6		
PHF	.000	.250	.000	.250	.831	.809	.000	.817	.654	.750	.809	.833	.000	.915	.865	.918	.870

						rucks	s Printed- T	Group					
		ssex St om West			urrill St m South	В		ssex St om East			Dwy om North	Fro	
Int. Tota	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Start Time
	1	1	0	0	0	0	0	1	0	0	0	0	07:00 AM
1	0	3	0	0	0	0	0	2	0	0	0	0	07:15 AM
	0	2	0	0	0	0	0	2	0	0	0	0	07:30 AM
	0	6	0	0	0	0	0	0	0	0	0	0	07:45 AM
1	1	12	0	0	0	0	0	5	0	0	0	0	Total
	0	1	0	1	0	0	0	3	1	0	0	0	08:00 AM
	0	4	0	0	0	0	0	5	0	0	0	0	08:15 AM
1	0	3	0	0	0	0	0	3	0	0	0	0	08:30 AM
	1	4	0	0	0	0	0	2	1	0	0	0	08:45 AM
2	1	12	0	1	0	0	0	13	2	0	0	0	Total
4	2	24	0	1	0	0	0	18	2	0	0	0	Grand Total
	7.7	92.3	0	100	0	0	0	90	10	0	0	0	Apprch %
	4.3	51. <b>1</b>	0	2.1	0	0	0	38.3	4.3	0	0	0	Total %

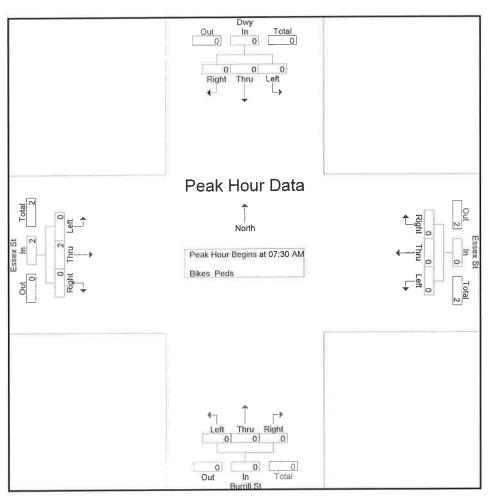
		_	wy North				ex St n East				rill St South				ex St West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int, Total
Peak Hour Analy	sis Fror	n 07:00	AM to C	8:45 AM	Peak 1	of 1											
Peak Hour for Er	ntire Inte	rsection	Begins	at 08:00	AM												7
08:00 AM	0	0	0	0	1	3	0	4	0	0	1	1	0	1	0	1	6
08:15 AM	0	0	0	0	0	5	0	5	0	0	0	0	0	4	0	4	9
08:30 AM	0	0	0	0	0	3	0	3	0	0	0	0	0	3	0	3	6
08:45 AM	0	0	0	0	1	2	0	3	0	0	0	0	0	4	1	5	8
Total Volume	0	0	0	0	2	13	0	15	0	0	1	1	0	12	1	13	29
% App. Total	0	0	0		13.3	86.7	0		0	0	100		0	92.3	7.7		
PHF	.000	.000	.000	.000	.500	.650	.000	750	.000	.000	.250	.250	.000	.750	.250	.650	.806

								Groups	Printed	- Bikes	Peds								
		D				Esse					ill St			Esse					
		From				From				From				From					I West az stratif
Start Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu Total	Inclu Total	Int. Total
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	1
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	1
08:30 AM	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2
08:45 AM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	11
Total	0	0	0	3	0	0	0	0	0	0	0	0	0	2	0	0	3	2	5
One of Total	0	0	0	2	0	0	0	0	0	0	0	0	0	2	Ο	0	3	2	5
Grand Total	0	0	0	3	0	0	0	0	0	0	0	U	0	100	0	0	5	2	0
Apprch %	0	0	0		0	0	0		0	0	0		0	100	0		60	40	
Total %	0	0	0		U	0	0		U	0	0		0	100	0		00	40	

			wy North			Part of the second	ex St 1 East				rill St South				ex St i West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis Fron	n 07:00	AM to C	08:45 AM -	Peak 1	of 1											
Peak Hour for Er	tire Inte	rsectior	Begins	s at 07:30	AM												
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	2
% App. Total	Ő	Ő	Ő		0	0	0		0	0	0		0	100	0		
PHF	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.500	.000	.500	.500

N/S Street : Driveway / Burrill Street E/W Street : Essex Street City/State : Swampscott, MA Weather : Clear

File Name	: 86880001
Site Code	:86880001
Start Date	: 12/2/2020
Page No	::11



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

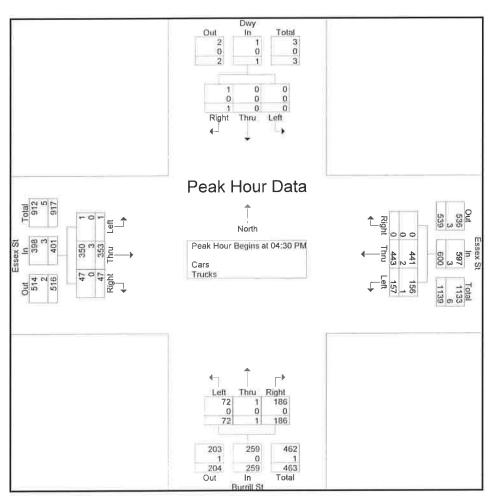
	07:00 AM				07:00 AM				07:00 AM				07:30 AM			
+0 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+15 mins.	Ő	ō	õ	Õ	0	0	0	0	0	0	0	0	0	0	0	0
+30 mins.	Ő	Ō	Ō	0	0	0	0	0	0	0	0	0	0	1	0	1
+45 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1
otal Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2
% App. Total	Ő	0	0		0	0	0		0	0	0		0	100	0	
PHF	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.500	.000	500

		ssex St			unit CA		rinted- Cars						
		om West			urrill St m South		_	ssex St om East			Dwy om North	Fro	
Int. Tota	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Start Time
30	9	98	0	46	0	13	0	96	40	0	0	0	04:00 PM
31	11	71	0	55	1	22	0	120	37	0	0	0	04:15 PM
29	19	82	1	33	0	16	0	104	43	0	0	0	04:30 PM
33	9	101	0	64	0	16	0	114	27	0	0	0	04:45 PM
124	48	352	1	198	1	67	0	434	147	0	0	0	Total
30	8	75	0	50	0	20	0	110	39	0	0	0	05:00 PM
33	11	95	0	39	1	20	0	115	48	1	0	0	05:15 PM
25	10	77	0	33	0	13	0	86	36	0	0	0	05:30 PM
28	13	79	0	58	0	17	0	76	38	0	0	0	05:45 PM
116	42	326	0	180	1	70	0	387	161	1	0	0	Total
241	90	678	1	378	2	137	0	821	308	1	0	0	Grand Total
	11.7	88.2	0.1	73.1	0.4	26.5	0	72.7	27.3	100	0	0	Apprch %
	3.7	28.1	0	15.6	0.1	5.7	0	34	12.7	0	0	0	Total %
240	89	670	1	378	2	136	0	817	307	1	0	0	Cars
99.	98.9	98.8	100	100	100	99.3	0	99.5	99.7	100	0	0	% Cars
1	1	8	0	0	0	1	0	4	1	0	0	0	Trucks
0.	1.1	1.2	0	0	0	0.7	0	0.5	0.3	0	0	0	% Trucks

			wy North				ex St i East				rill St South				ex St West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis From	1 04:00	PM to 0	5:45 PM -	Peak 1	of 1											
Peak Hour for Er	itire Inte	rsectior	n Begins	at 04:30	PM							7					
04:30 PM	0	0	0	0	43	104	0	147	16	0	33	49	1	82	19	102	298
04:45 PM	Ő	0	0	0	27	114	0	141	16	0	64	80	0	101	9	110	331
05:00 PM	0	0	0	0	39	110	0	149	20	0	50	70	0	75	8	83	302
05:15 PM	Õ	Õ	1	1	48	115	0	163	20	1	39	60	0	95	11	106	330
Total Volume	0	0	1	1	157	443	0	600	72	1	186	259	1	353	47	401	1261
% App. Total	0	0	100		26.2	73.8	0		27.8	0.4	71.8		0.2	88	11.7		
PHF	.000	.000	.250	.250	.818	.963	.000	.920	.900	.250	.727	.809	.250	.874	618	.911	.952
Cars	0	0	1	1	156	441	0	597	72	1	186	259	1	350	47	398	1255
% Cars	Ő	õ	100	100	99.4	99.5	0	99.5	100	100	100	100	100	99.2	100	99.3	99.5
Trucks	õ	ñ	0	0	1	2	0	3	0	0	0	0	0	3	0	3	6
% Trucks	0	Ő	Ő	0	0.6	0.5	0	0.5	0	0	0	0	0	0.8	0	0.7	0.5

N/S Street : Driveway / Burrill Street E/W Street : Essex Street City/State : Swampscott, MA Weather : Clear

File Name	86880001
Site Code	86880001
Start Date	:12/2/2020
Page No	: 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

	04:30 PM				04:30 PM				04:15 PM				04:00 PM			
+0 mins.	0	0	0	0	43	104	0	147	22	1	55	78	0	98	9	107
+15 mins.	0	0	0	0	27	114	0	141	16	0	33	49	0	71	11	82
+30 mins.	0	0	0	0	39	110	0	149	16	0	64	80	1	82	19	102
+45 mins.	0	0	1	1	48	115	0	163	20	0	50	70	0	101	9	110
otal Volume	0	0	1	1	157	443	0	600	74	1	202	277	1	352	48	401
6 App. Total	0	0	100		26.2	73.8	0		26.7	0.4	72.9		0.2	87.8	12	
PHF	.000	.000	.250	.250	.818	.963	.000	.920	.841	.250	.789	.866	.250	.871	.632	.911
Cars	0	0	1	1	156	441	0	597	74	. 1	202	277	1	347	48	396
% Cars	0	0	100	100	99.4	99.5	0	99.5	100	100	100	100	100	98.6	100	98.8
Trucks	0	0	0	0	1	2	0	3	0	0	0	0	0	5	0	5
% Trucks	0	0	0	0	0.6	0.5	0	0.5	0	0	0	0	0	1.4	0	1.2

		ssex St	E		urrill St	B		ssex St	F		Dwy		
		om West			m South	_		om East			om North	Fro	
Int. Tot	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Start Time
30	9	97	0	46	0	13	0	96	40	0	0	0	04:00 PM
31	11	69	0	55	1	22	0	119	37	0	0	0	04:15 PM
29	19	81	1	33	0	16	0	103	43	0	0	0	04:30 PM
33	9	100	0	64	0	16	0	114	27	0	0	0	04:45 PM
124	48	347	1	198	1	67	0	432	147	0	0	0	Total
30	8	75	0	50	0	20	0	109	39	0	0	0	05:00 PM
32	11	94	0	39	1	20	0	115	47	1	0	0	05:15 PM
25	9	77	0	33	0	12	0	85	36	0	0	0	05:30 PM
27	13	77	0	58	0	17	0	76	38	0	0	0	05:45 PM
116	41	323	0	180	1	69	0	385	160	1	0	0	Total
240	89	670	1	378	2	136	0	817	307	1	0	0	Grand Total
	11.7	88.2	0.1	73.3	0.4	26.4	0	72.7	27.3	100	0	0	Apprch %
	3.7	27.9	0	15.7	0.1	5.7	0	34	12.8	0	0	0	Total %

		11111 1101 - 111-11	wy North				ex St n East				rill St South				ex St n West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App: Total	Int. Total
Peak Hour Analy	sis Fror	n 04:00	PM to 0	)5:45 PM	- Peak 1	of 1											
Peak Hour for Er	tire Inte	ersectior	Begins	s at 04:30	PM												
04:30 PM	0	0	0	0	43	103	0	146	16	0	33	49	1	81	19	101	296
04:45 PM	0	0	0	0	27	114	0	141	16	0	64	80	0	100	9	109	330
05:00 PM	0	0	0	0	39	109	0	148	20	0	50	70	0	75	8	83	301
05:15 PM	0	0	1	1	47	115	0	162	20	1	39	60	0	94	11	105	328
Total Volume	0	0	1	1	156	441	0	597	72	1	186	259	1	350	47	398	1255
% App. Total	0	0	100		26.1	73.9	0		27.8	0.4	71.8		0.3	87.9	11.8		
PHF	.000	.000	.250	.250	.830	.959	.000	.921	.900	.250	.727	.809	.250	.875	.618	.913	.951

					Group	s Printed- 1	rucks						
	Fr	Dwy om North			ssex St om East		E	Burrill St om South			ssex St om West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	0	0	0	0	0	0	0	0	0	0	1	0	1
04:15 PM	0	0	0	0	1	0	0	0	0	0	2	0	3
04:30 PM	0	0	0	0	1	0	0	0	0	0	1	0	2
04:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	1
Total	0	0	0	0	2	0	0	0	0	0	5	0	7
05:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	1
05:15 PM	0	0	0	1	0	0	0	0	0	0	1	0	2
05:30 PM	0	0	0	0	1	0	1	0	0	0	0	1	3
05:45 PM	0	0	0	0	0	0	0	0	0	0	2	0	2
Total	0	0	0	1	2	0	1	0	0	0	3	1	8
Grand Total	0	0	0	1	4	0	1	0	0	0	8	1	15
Apprch %	0	0	0	20	80	0	100	0	0	0	88.9	11.1	
Total %	0	0	0	6.7	26.7	0	6.7	0	0	0	53.3	6.7	

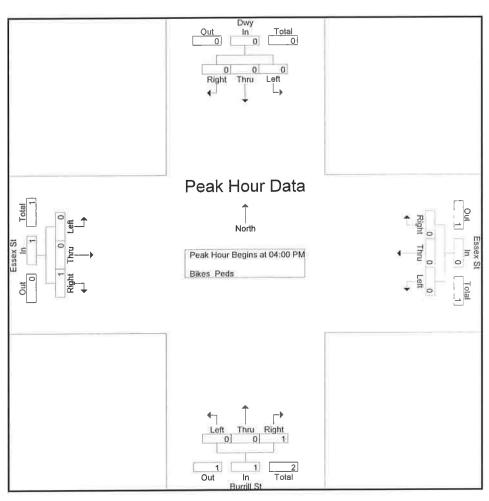
		_	wy North				ex St 1 East				rill St South				ex St n West		
Start Time	Left			App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis Fror	n 04:00	PM to C	)5:45 PM -	Peak 1	of 1											
Peak Hour for Er	itire Inte	rsection	Begins	at 05:00	PM							1.0					
05:00 PM	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	1
05:15 PM	0	0	0	0	1	0	0	1	0	0	0	0	0	1	0	1	2
05:30 PM	0	0	0	0	0	1	0	1	1	0	0	1	0	0	1	1	3
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	2
Total Volume	0	0	0	0	1	2	0	3	1	0	0	1	0	3	1	4	8
% App. Total	Ő	Ō	Ō		33.3	66.7	0		100	0	0		0	75	25		
PHF	.000	.000	.000	.000	.250	.500	.000	.750	.250	.000	.000	.250	.000	.375	.250	.500	.667

								Groups	Printed	- Bikes	Peds						v.		
			wy North		Essex St From East			Burrill St From South			Essex St From West						1		
Start Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu Total	inclu. Total	Int. Total
04:00 PM	0	0	0	0	0	0	0	1	0	0	1	1	0	0	1	0	2	2	4
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	1	0	0	0	1	0	0	1	1	0	0	1	0	3	2	5
05:00 PM	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	2	0	2
05:15 PM	ō	Ō	Ō	1	0	0	0	0	0	0	0	0	0	0	0	1	2	0	2
05:30 PM	õ	Ō	Ō	1	0	0	0	1	0	0	0	0	0	0	0	1	3	0	3
05:45 PM	ō	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	1
Total	0	0	0	2	0	0	0	3	1	0	0	0	0	0	0	2	7	1	8
Grand Total	0	0	0	3	0	0	0	4	1	0	1	1	0	0	1	2	10	3	13
Apprch % Total %	0 0	0 0	0 0		0	0	0 0	),	50 33,3	0 0	50 33.3		0	0	100 33.3		76.9	23.1	

			wy North				ex St n East				rill St South				ex St n West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App Total	Int. Total
Peak Hour Analy	sis Fron	n 04:00	PM to C	5:45 PM	Peak 1	of 1											
Peak Hour for Er																	
04:00 PM	0	0	0	0	0	0	0	0	0	0	1	1	0	0	1	1	2
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:45 PM	Ő	0	Ō	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0	0	1	1	0	0	1	1	2
% App. Total	ő	Ő	ō		Õ	0	0		0	0	100		0	0	100		
PHF	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.250	.250	.000	.000	.250	.250	.250

N/S Street : Driveway / Burrill Street E/W Street : Essex Street City/State : Swampscott, MA Weather : Clear

File Name	:86880001
Site Code	86880001
Start Date	: 12/2/2020
Page No	11



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

	04 00 PM				04:00 PM				04:00 PM				04:00 PM			
+0 mins.	0	0	0	0	0	0	0	0	0	0	1	1	0	0	1	1
+15 mins.	Ō	Ō	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+30 mins.	0	Ō	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+45 mins.	0	õ	Ō	0	0	0	0	0	0	0	0	0	0	0	0	0
otal Volume	0	0	0	0	0	0	0	0	0	0	1	1	0	0	1	1
& App. Total	0	0	0		0	0	0		0	0	100		0	0	100	
PHF	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.250	.250	.000	.000	.250	.250

N/S Street : Pitman Road E/W Street : Essex Street City/State : Swampscott, MA Weather : Clear

		Essex St From West		Pitman Rd From South		Essex St From East	
Int. Total	Right	Thru	Right	Left	Thru	Left	Start Time
156	0	94	0	0	62	0	07:00 AM
145	2	76	2	1	64	0	07:15 AM
188	0	107	0	0	81	Ő	07:30 AM
236	0	129	0	1	106	Ő	07:45 AM
725	2	406	2	2	313	0	Total
200	1	104	0	0	95	0	08:00 AM
184	0	104	0	0	80	0	08:15 AM
172	1	84	1	1	85	0	08:30 AM
172	0	103	0	0	68	4	08:45 AM
728	2	395	1	1	328	1	Total
1453	4	801	3	3	641	1	Grand Total
	0.5	99.5	50	50	99.8	0.2	Apprch %
	0.3	55.1	0.2	0.2	44.1	0.1	Total %
1411	4	779	3	3	621	1	Cars
97.1	100	97.3	100	100	96.9	100	% Cars
42	0	22	0	0	20	0	Trucks
2.9	0	2.7	0	0	3.1	Ō	% Trucks

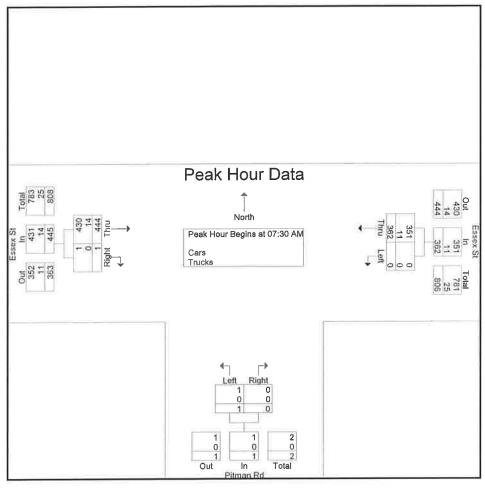
		Essex St From East			Pitman Rd From South			Essex St From West		
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07										
Peak Hour for Entire Interse	ction Begins	at 07:30 Ai							10-	400
07:30 AM	0	81	81	0	0	0	107	0	107	188
07:45 AM	0	106	106	1	0	1	129	0	129	236
08:00 AM	0	95	95	0	0	0	104	1	105	200
08:15 AM	0	95 80	80	0	0	0	104	0	104	184
Total Volume	0	362	362	1	0	1	444	1	445	808
% App. Total	0	100		100	0		99.8	0.2		
PHF	.000	.854	.854	.250	.000	.250	.860	.250	.862	.856
Cars	0	351	351	1	0	1	430	1	431	783
% Cars	õ	97.0	97.0	100	0	100	96.8	100	96.9	96.9
Trucks	Õ	11	11	0	0	0	14	0	14	25
% Trucks	Ō	3.0	3.0	0	0	0	3.2	0	3.1	3.1

N/S Street : Pitman Road E/W Street : Essex Street City/State : Swampscott, MA Weather : Clear 
 File Name
 86880002

 Site Code
 86880002

 Start Date
 12/2/2020

 Page No
 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

еактной тог сасл Арр	07:45 AM			07:00 AM			07:30 AM		
+0 mins.	0	106	106	0	0	0	107	0	107
+15 mins.	0	95	95	1	2	3	129	0	129
+30 mins.	0	80	80	0	0	0	104	1	105
+45 mins.	0	85	85	1	0	1	104	0	104
Total Volume	0	366	366	2	2	4	444	1	445
% App. Total	0	100		50	50		99.8	0.2	
PHF	.000	.863	.863	.500	.250	.333	.860	.250	.862
Cars	0	353	353	2	2	4	430	1	431
% Cars	0	96.4	96.4	100	100	100	96.8	100	96.9
Trucks	0	13	13	0	0	0	14	0	14
% Trucks	0	3.6	3.6	0	0	0	3.2	0	3.1

		Essex St		Pitman Rd		Essen Ct	
						Essex St	
		From West		From South		From East	
Int. Total	Right	Thru	Right	Left	Thru	Left	Start Time
155	0	93	0	0	62	0	07:00 AM
141	2	74	2	1	62	0	07:15 AM
183	0	104	0	0	79	0	07:30 AM
230	0	123	0	1	106	0	07:45 AM
709	2	394	2	2	309	0	Total
194	1	102	0	0	91	0	08:00 AM
176	0	101	0	0	75	0	08:15 AM
166	1	82	1	1	81	0	08:30 AM
166	0	100	0	0	65	1	08:45 AM
702	2	385	1	1	312	1	Total
1411	4	779	3	3	621	1	Grand Total
	0.5	99.5	50	50	99.8	0.2	Apprch %
	0.3	55.2	0.2	0.2	44	0.1	Total %

		Essex St From East			Pitman Rd From South			Essex St From West		
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Tota
Peak Hour Analysis From 07	:00 AM to 0	8:45 AM -	Peak 1 of 1							
Peak Hour for Entire Interse	ction Begins	at 07:30 A	M						10	
07:30 AM	0	79	79	0	0	0	104	0	104	183
07:45 AM	0	106	106	1	0	1	123	0	123	230
08:00 AM	0	91	91	0	0	0	102	1	103	194
08:15 AM	0	75	75	0	0	0	101	0	101	176
Total Volume	0	351	351	1	0	1	430	1	431	783
% App. Total	0	100		100	0		99.8	0,2		
PHF	000	.828	.828	.250	.000	.250	.874	.250	.876	.851

	Essex St From West		Pitman Rd From South		Essex St From East	
Right Int.	Thru	Right	Left	Thru	Left	Start Time
0	1	0	0	0	0	07:00 AM
0	2	0	0	2	0	07:15 AM
0	3	0	0	2	0	07:30 AM
0	6	0	0	0	0	07:45 AM
0	12	0	0	4	0	Total
0	2	0	0	4	0	08:00 AM
0	3	0	0	5	0	08:15 AM
0	2	0	0	4	0	08:30 AM
0	3	0	0	3	0	08:45 AM
0	10	0	0	16	0	Total
0	22	0	0	20	0	Grand Total
0	100	0	0	100	Ō	Apprch %
0	52.4	0	0	47.6	0	Total %

		Essex St From East			Pitman Rd from South			Essex St From West		
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07	:00 AM to 0	8:45 AM - F	Peak 1 of 1							
Peak Hour for Entire Interse	ction Begins	at 07:45 A	M							
07:45 AM	o	0	0	0	0	0	6	0	6	6
08:00 AM	0	4	4	0	0	0	2	0	2	6
08:15 AM	0	5	5	0	0	0	3	0	3	8
08:30 AM	0	4	4	0	0	0	2	0	2	6
Total Volume	0	13	13	0	0	0	13	0	13	26
% App. Total	0 0	100		0	0		100	0		
PHF	.000	.650	.650	.000	.000	.000	.542	.000	.542	.813

					Groups Prir	nted- Bikes	Peds			27		
		ssex St rom East		Р	Pitman Rd From South		E	Essex St rom West				
Start Time	Left	Thru	Peds	Left	Right	Peds	Thru	Right	Peds	Exclu. Total	Inciu. Total	Int. Tota
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	C
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	C
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	C
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	C
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	C
08:15 AM	0	0	0	0	0	1	0	0	0	1	0	1
08:30 AM	Ō	0	0	0	0	0	0	0	0	0	0	C
08:45 AM	0	0	0	0	0	1	0	0	0	1	0	
Total	0	0	0	0	0	2	0	0	0	2	0	2
Grand Total	0	0	0	0	0	2	0	0	0	2	0	2
Apprch % Total %	0	0		0	0		0	0		100	0	

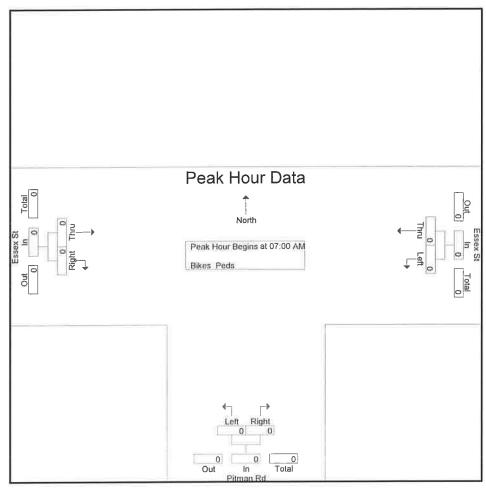
		Essex St From East			Pitman Rd From South			Essex St From West		
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Tota
Peak Hour Analysis From 07	7:00 AM to 0	8:45 AM - I	Peak 1 of 1							
Peak Hour for Entire Interse									- 6	
07:00 AM	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0	0
% App. Total	0	õ	-	0	0		0	0		
PHF	.000	,000	.000	.000	.000	.000	.000	.000	.000	.000

 File Name
 86880002

 Site Code
 86880002

 Start Date
 12/2/2020

 Page No
 11



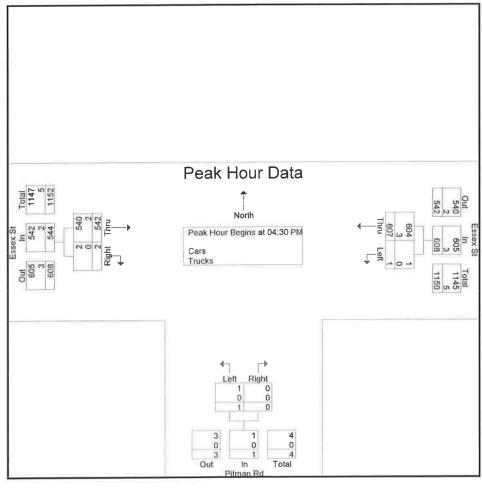
### Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

	07 00 AM		07	':00 AM			07:00 AM		
+0 mins.	0	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	0	0	0	0
+45 mins.	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0
% App. Total	0	0		0	0		0	0	
PHF	.000	.000	.000	.000	.000	.000	.000	.000	.000

	lest	Essex St From West		Pitman Rd From South		Essex St From East	
nt Int. To	Right	Thru	Right	Left	Thru	Left	Start Time
0 2	0	145	1	0	143	0	04:00 PM
0 2	0	128	0	0	155	0	04:15 PM
1 2	1	117	0	0	144	0	04:30 PM
0 3	0	162	0	0	145	1	04:45 PM
1 11	1	552	1	0	587	1	Total
0 2	0	131	0	0	154	0	05:00 PM
1 2	1	132	0	1	164	0	05:15 PM
0 2	0	112	0	0	123	0	05:30 PM
1 2	1	132	0	0	117	0	05:45 PM
2 10	2	507	0	1	558	0	Total
3 22	3	1059	1 📋	1	1145	1	Grand Total
3	0.3	99.7	50	50	99.9	0.1	Apprch %
	0.1	47.9	0	0	51.8	0	Total %
-	3	1054	1	1	1139	1	Cars
0 9	100	99.5	100	100	99.5	100	% Cars
0	0	5	0	0	6	0	Trucks
0	0	0.5	0	0	0.5	0	% Trucks

		Essex St From East			Pitman Rd From South					
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04	4:00 PM to 0	5:45 PM - P	eak 1 of 1							
Peak Hour for Entire Interse										
04:30 PM	o	144	144	0	0	0	117	1	118	262
04:45 PM	1	145	146	0	0	0	162	0	162	308
05:00 PM	0	154	154	0	0	0	131	0	131	285
05:15 PM	0	164	164	_1	0	1	132	1	133	298
Total Volume	1	607	608	1	0	1	542	2	544	1153
% App. Total	0.2	99.8		100	0		99.6	0.4		
PHF	.250	.925	.927	.250	.000	.250	.836	.500	.840	.936
Cars	1	604	605	1	0	1	540	2	542	1148
% Cars	100	99.5	99.5	100	0	100	99.6	100	99.6	99.6
Trucks	0	3	3	0	0	0	2	0	2	5
% Trucks	0	0.5	0.5	0	0	0	0.4	0	0.4	0.4

File Name : 86880002 Site Code : 86880002 Start Date : 12/2/2020 Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

eak Hour for Each App	04:30 PM	<i>.</i>		04:00 PM			04:00 PM		
	04.30 PIVI			04.001 14					
+0 mins.	0	144	144	0	1	1	145	0	145
+15 mins.	1	145	146	0	0	0	128	0	128
+30 mins.	0	154	154	0	0	0	117	1	118
+45 mins.	0	164	164	0	0	0	162	0	162
Total Volume	1	607	608	0	1	1	552	1	553
% App. Total	0.2	99.8		0	100		99.8	0.2	
PHF	.250	.925	.927	.000	.250	.250	.852	.250	.853
Cars	1	604	605	0	1	1	550	1	551
% Cars	100	99.5	99.5	0	100	100	99.6	100	99.6
Trucks	0	3	3	0	0	0	2	0	2
% Trucks	Õ	0.5	0.5	0	0	0	0.4	0	0.4

		Easay St		os Printed- Cars			
		Essex St		Pitman Rd		Essex St	
		From West		From South		From East	
Int. Total	Right	Thru	Right	Left	Thru	Left	Start Time
289	0	145	1	0	143	0	04:00 PM
281	0	127	0	0	154	0	04:15 PM
261	1	117	0	0	143	0	04:30 PM
307	0	161	0	0	145	1	04:45 PM
1138	1	550	1	0	585	1	Total
284	0	131	0	0	153	0	05:00 PM
296	1	131	0	1	163	0	05:15 PM
234	0	112	0	0	122	Õ	05:30 PM
247	ୀ	130	0	0	116	0	05:45 PM
1061	2	504	0	1	554	0	Total
2199	3	1054	1	1	1139	1	Grand Total
	0.3	99.7	50	50	99.9	0.1	Apprch %
	0.1	47.9	0	0	51.8	0	Total %

		Essex St From East			Pitman Rd From South		1	Essex St From West		
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
eak Hour Analysis From 04	:00 PM to 0	5:45 PM - I	Peak 1 of 1							
eak Hour for Entire Intersed									0	
04:30 PM	o	143	143	0	0	0	117	1	118	261
04:45 PM	1	145	146	0	0	0	161	0	161	307
05:00 PM	0	153	153	0	0	0	131	0	131	284
05:15 PM	0	163	163	1	0	1	131	1	132	296
Total Volume	1	604	605	1	0	1	540	2	542	1148
% App. Total	0.2	99.8		100	0		99.6	0.4		
PHF	.250	.926	.928	.250	.000	.250	.839	.500	.842	.935

	Essex St		Pitman Rd		Essex St		
	From East		From Sout		From Wes		
Start Time	Left	Thru	Left	Right	Thru	Right	Int. Total
04:00 PM	0	0	0	0	0	0	0
04:15 PM	0	1	0	0	1	0	2
04:30 PM	0	1	0	0	0	0	1
04:45 PM	0	0	0	0	1	0	1
Total	0	2	0	0	2	0	4
05:00 PM	0	3	0	0	0	0	1
05:15 PM	0	1	0	0	1	0	2
05:30 PM	0	1	0	0	0	0	1
05:45 PM	0	1	0	0	2	0	3
Total	0	4	0	0	3	0	7
Grand Total	0	6	0	0	5	0	11
Apprch %	0	100	0	0	100	0	
Total %	Ō	54.5	0	0	45.5	0	

		Essex St From East			Pitman Rd From South					
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
eak Hour Analysis From 04	:00 PM to 0	5:45 PM - F	Peak 1 of 1							
Peak Hour for Entire Interse									10	
05:00 PM	0	1	1	0	0	0	0	0	0	1
05:15 PM	0	1	1	0	0	0	1	0	1	2
05:30 PM	0	1	1	0	0	0	0	0	0	1
05:45 PM	0	1	1	0	0	0	2	0	2	3
Total Volume	0	4	4	0	0	0	3	0	3	7
% App. Total	0	100		0	0		100	0		
PHF	.000	1.00	1.00	.000	.000	.000	.375	.000	.375	.583

					Groups Prir	nted- Bikes	Peds					
		ssex St om East		P	itman Rd om South		8	Essex St rom West				
Start Time	Left	Thru	Peds	Left	Right	Peds	Thru	Right	Peds	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	0	0	0	0	1	0	0	0	1	0	1
04:15 PM	0	0	0	0	0	1	0	0	0	1	0	1
04:30 PM	0	0	0	0	0	2	1	0	0	2	1	3
04:45 PM	0	0	0	0	0	1	0	0	0	1	0	1
Total	0	0	0	0	0	5	1	0	0	5	1	6
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	Ō	0	0	0	0	1	0	0	0	1	0	1
05:30 PM	Ō	0	0	0	0	0	0	0	0	0	0	0
05:45 PM	0	0	0	0	0	1	0	0	0	1	0	1
Total	0	0	0	0	0	2	0	0	0	2	0	2
Grand Total	0	0	0	0	0	7	1	0	0	7	1	8
Apprch %	0	0		0	0		100	0				
Total %	0	0	1	0	0		100	0		87.5	12.5	

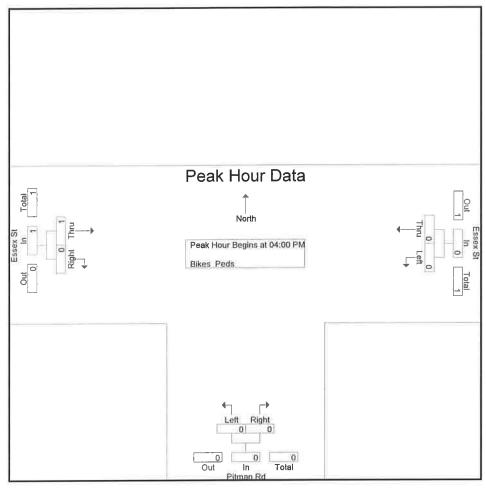
		Essex St From East			Pitman Rd From South					
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04	:00 PM to 0	5:45 PM - F	Peak 1 of 1							
Peak Hour for Entire Interse	ction Begins	at 04:00 P	M							
04:00 PM	o	0	0	0	0	0	0	0	0	0
04:15 PM	0	0	0	0	0	0	0	0	0	0
04:30 PM	0	0	0	0	0	0	1	0	1	1
04:45 PM	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	1	0	1	1
% App. Total	0	0		0	0		100	0		
PHF	.000	.000	.000	.000	.000	.000	.250	.000	.250	.250

 File Name
 86880002

 Site Code
 86880002

 Start Date
 12/2/2020

 Page No
 11



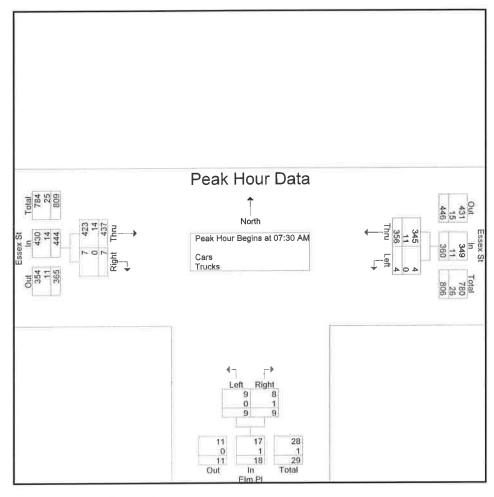
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

eak Hour for Each Approac	00 PM			04:00 PM			04:00 PM		
+0 mins.	0	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	0	1	0	1
+45 mins.	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	1	0	1
% App. Total	0	0		0	0		100	0	
PHF	.000	.000	.000	.000	.000	.000	.250	.000	.250

		Essex St From West		Elm Pi From South		Essex St From East	
Int. Tota	Right	Thru	Right	Left	Thru	Left	Start Time
16	2	94	1	1	60	2	07:00 AM
14:	1	78	0	1	62	0	07:15 AM
19	3	105	1	0	85	1	07:30 AM
23	2	124	5	6	97	1	07:45 AM
73	8	401	7	8	304	4	Total
204	2	102	3	2	93	2	08:00 AM
18	0	106	0	1	81	0	08:15 AM
17:	0	86	0	1	85	0	08:30 AM
17	1	100	1	0	69	0	08:45 AM
73:	3	394	4	4	328	2	Total
146	11	795	11	12	632	6	Grand Total
	1.4	98.6	47.8	52.2	99.1	0.9	Apprch %
	0.7	54.2	0.7	0.8	43.1	0.4	Total %
142	11	773	9	12	614	5	Cars
97.	100	97.2	81.8	100	97.2	83.3	% Cars
4:	0	22	2	0	18	1	Trucks
2.9	0	2.8	18.2	0	2.8	16.7	% Trucks

		Essex St From East		F	Elm Pl From South					
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07	7:00 AM to 0	8:45 AM - F	eak 1 of 1							
Peak Hour for Entire Interse	ction Begins	at 07:30 AM	N. I						47	
07:30 AM	1	85	86	0	1	1	105	3	108	195
07:45 AM	1	97	98	6	5	11	124	2	126	235
08:00 AM	2	93	95	2	3	5	102	2	104	204
08:15 AM	0	81	81	1	0	1	106	0	106	188
Total Volume	4	356	360	9	9	18	437	7	444	822
% App. Total	1.1	98.9		50	50		98.4	1.6		
PHF	.500	.918	.918	.375	.450	.409	.881	.583	.881	.874
Cars	4	345	349	9	8	17	423	7	430	796
% Cars	100	96.9	96.9	100	88.9	94.4	96.8	100	96.8	96.8
Trucks	0	11	11	0	1	1	14	0	14	26
% Trucks	0	3.1	3.1	0	11.1	5.6	3.2	0	3.2	3.2

File Name : 86880003 Site Code : 86880003 Start Date : 12/2/2020 Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

	07:30 AM			07:15 AM			07:30 AM		
+0 mins.	1	85	86	1	0	1	105	3	108
+15 mins.	1	97	98	0	1	1	124	2	126
+30 mins.	2	93	95	6	5	11	102	2	104
+45 mins.	0	81	81	2	3	5	106	0	106
Total Volume	4	356	360	9	9	18	437	7	444
% App. Total	1.1	98.9		50	50		98.4	1.6	
PHF	.500	.918	.918	.375	.450	.409	.881	.583	.881
Cars	4	345	349	9	8	17	423	7	430
% Cars	100	96.9	96.9	100	88.9	94.4	96.8	100	96.8
Trucks	0	11	11	0	1	1	14	0	14
% Trucks	0	3.1	3.1	0	11.1	5.6	3.2	0	3.2

		Essex St From West		Elm Pl From South		Essex St From East	
Int. Tota	Right	Thru	Right	Left	Thru	Left	Start Time
157	2	93	0	1	60	1	07:00 AM
138	1	76	0	1	60	0	07:15 AM
190	3	102	1	0	83	1	07:30 AM
228	2	118	4	6	97	1	07:45 AM
713	8	389	5	8	300	3	Total
198	2	100	3	2	89	2	08:00 AM
180	0	103	0	1	76	0	08:15 AM
167	0	84	0	1	82	0	08:30 AM
166	1	97	1	0	67	0	08:45 AM
711	3	384	4	4	314	2	Total
1424	11	773	9	12	614	5	Grand Total
	1.4	98.6	42.9	57.1	99.2	0.8	Apprch %
	0.8	54.3	0.6	0.8	43.1	0.4	Total %

		Essex St From East		F	Elm PI From South			Essex St From West		
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Tota
eak Hour Analysis From 07	:00 AM to 0	8:45 AM - I	Peak 1 of 1							
Peak Hour for Entire Intersed						52				
07:30 AM	1	83	84	0	1	1	102	3	105	190
07:45 AM	1	97	98	6	4	10	118	2	120	228
08:00 AM	2	89	91	2	3	5	100	2	102	198
08:15 AM	0	89 76	76	1	0	1	103	0	103	180
Total Volume	4	345	349	9	8	17	423	7	430	796
% App. Total	1.1	98.9		52.9	47.1		98.4	1.6		
PHF	.500	.889	.890	.375	.500	.425	.896	.583	.896	.873

		Essex St		Elm Pl		Essev Ct	
						Essex St	
		From West		From South		From East	
Int. Tota	Right	Thru	Right	Left	Thru	Left	Start Time
3	0	1	1	0	0	1	07:00 AM
4	0	2	0	0	2	0	07:15 AM
5	0	3	0	0	2	0	07:30 AM
7	0	6	1	0	0	0	07:45 AM
19	0	12	2	0	4	1	Total
e	0	2	0	0	4	0	08:00 AM
8	0	3	0	0	5	0	08:15 AM
5	0	2	0	0	3	0	08:30 AM
5	0	3	0	0	2	0	08:45 AM
24	0	10	0	0	14	0	Total
43	0	22	2	0	18	1	Grand Total
	0	100	100	0	94.7	5.3	Apprch %
	0	51.2	4.7	0	41.9	2.3	Total %

		Essex St From East		F	Elm PI From South			Essex St From West		
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07	:00 AM to 0	8:45 AM - F	Peak 1 of 1							
Peak Hour for Entire Intersed									-74-	
07:30 AM	0	2	2	0	0	0	3	0	3	5
07:45 AM	0	0	0	0	1	1	6	0	6	7
08:00 AM	0	4	4	0	0	0	2	0	2	6
08:15 AM	0	5	5	0	0	0	3	0	3	8
Total Volume	0	11	11	0	1	1	14	0	14	26
% App. Total	Ő	100		0	100		100	0		
PHF	.000	.550	.550	.000	.250	.250	.583	.000	.583	.813

					Groups Prir	ted- Bikes	Peds					
		ssex St om East		Elm Pl From South			Essex St From West					
Start Time	Left	Thru	Peds	Left	Right	Peds	Thru	Right	Peds	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0
08:15 AM	Ō	0	0	0	0	1	0	0	0	1	0	1
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	0	1	0	0	0	1	0	1
Total	0	0	0	0	0	2	0	0	0	2	0	2
Grand Total	0	0	0	0	0	2	0	0	0	2	0	2
Apprch % Total %	0	0		0	0		0	0		100	0	

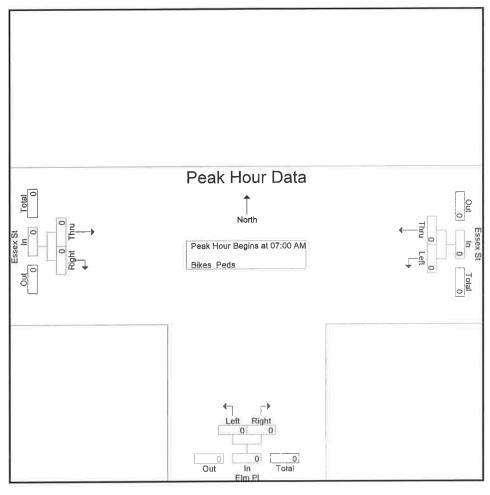
	Essex St From East			F	Elm Pl From South					
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07	:00 AM to 0	8:45 AM - F	Peak 1 of 1							
Peak Hour for Entire Interse										
07:00 AM	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0	0
% App. Total	ō	0		0	0		0	0		
PHF	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000

N/S Street : Elm Place E/W Street : Essex Street City/State : Swampscott, MA Weather : Clear 
 File Name
 86880003

 Site Code
 86880003

 Start Date
 12/2/2020

 Page No
 11



### Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

	07:00 AM			07:00 AM		(	07:00 AM		
+0 mins.	0	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	0	0	0	0
+45 mins.	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0
% App. Total	0	0		0	0		0	0	
PHF	.000	.000	.000	.000	.000	.000	.000	.000	.000

		Essex St From West		Elm PI From South		Essex St From East	
Int. Tota	Right	Thru	Right	Left	Thru	Left	Start Time
29	3	140	6	4	140	2	04:00 PM
283	2	125	0	1	153	2	04:15 PM
262	2	114	2	2	141	1	04:30 PM
323	12	149	10	3	143	6	04:45 PM
116	19	528	18	10	577	11	Total
28	7	124	0	3	151	2	05:00 PM
29	0	132	0	1	162	0	05:15 PM
23	1	111	0	0	123	2	05:30 PM
260	12	122	6	6	112	8	05:45 PM
108	20	489	6	10	548	12	Total
2248	39	1017	24	20	1125	23	Grand Total
	3.7	96.3	54.5	45.5	98	2	Apprch %
	1.7	45.2	1.1	0.9	50	1	Total %
2236	39	1011	24	20	1119	23	Cars
99.	100	99.4	100	100	99.5	100	% Cars
12	0	6	0	0	6	0	Trucks
0.5	0	0.6	0	0	0.5	0	% Trucks

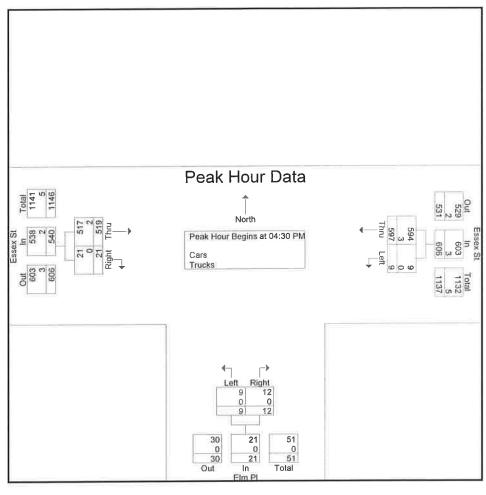
		Essex St From East		F	Elm Pl From South			Essex St From West		
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04	4:00 PM to 0	5:45 PM - P	eak 1 of 1							
Peak Hour for Entire Interse	ection Begins	at 04:30 PM	1							
04:30 PM	1	141	142	2	2	4	114	2	116	262
04:45 PM	6	143	149	3	10	13	149	12	161	323
05:00 PM	2	151	153	3	0	3	124	7	131	287
05:15 PM	0	162	162	1	0	1	132	0	132	295
Total Volume	9	597	606	9	12	21	519	21	540	1167
% App. Total	1.5	98.5		42.9	57.1		96.1	3.9		
PHF	.375	.921	.935	.750	.300	.404	.871	.438	.839	.903
Cars	9	594	603	9	12	21	517	21	538	1162
% Cars	100	99.5	99.5	100	100	100	99.6	100	99.6	99.6
Trucks	0	3	3	0	0	0	2	0	2	5
% Trucks	0	0.5	0.5	0	0	0	0.4	0	0.4	0.4

 File Name
 86880003

 Site Code
 86880003

 Start Date
 12/2/2020

 Page No
 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

ak nour for caun Appr	04:30 PM			04:00 PM			04:00 PM		
+0 mins.	1	141	142	4	6	10	140	3	143
+15 mins.	6	143	149	1	0	1	125	2	127
+30 mins. :	2	151	153	2	2	4	114	2	116
+45 mins.	0	162	162	3	10	13	149	12	161
Total Volume	9	597	606	10	18	28	528	19	547
% App. Total	1.5	98.5		35.7	64.3		96.5	3.5	
PHF	.375	.921	.935	.625	.450	.538	.886	.396	.849
Cars	9	594	603	10	18	28	525	19	544
% Cars	100	99.5	99.5	100	100	100	99.4	100	99.5
Trucks	0	3	3	0	0	0	3	0	3
% Trucks	0	0.5	0.5	0	0	0	0.6	0	0.5

				ps Printed- Cars	Grou		
		Essex St From West		Elm Pl From South		Essex St From East	
Int. Total	Right	Thru	Right	Left	Thru	Left	Start Time
295	3	140	6	4	140	2	04:00 PM
280	2	123	0	1	152	2	04:15 PM
261	2	114	2	2	140	1	04:30 PM
322	12	148	10	3	143	6	04:45 PM
1158	19	525	18	10	575	11	Total
286	7	124	0	3	150	2	05:00 PM
293	0	131	0	1	161	0	05:15 PM
236	1	111	0	0	122	2	05:30 PM
263	12	120	6	6	111	8	05:45 PM
1078	20	486	6	10	544	12	Total
2236	39	1011	24	20	1119	23	Grand Total
	3.7	96.3	54.5	45.5	98	2	Apprch %
	1.7	45.2	1.1	0.9	50	-	Total %

	Essex St From East			F	Elm Pl From South					
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Tota
Peak Hour Analysis From 04	1:00 PM to 0	5:45 PM - I	Peak 1 of 1							
Peak Hour for Entire Interse									i i	
04:30 PM	1	140	141	2	2	4	114	2	116	261
04:45 PM	6	143	149	3	10	13	148	12	160	322
05:00 PM	2	150	152	3	0	3	124	7	131	286
05:15 PM	0	161	161	1	0	1	131	0	131	293
Total Volume	9	594	603	9	12	21	517	21	538	1162
% App. Total	1.5	98.5		42.9	57.1		96.1	3.9		
PHF	.375	.922	.936	.750	.300	.404	.873	.438	.841	.902

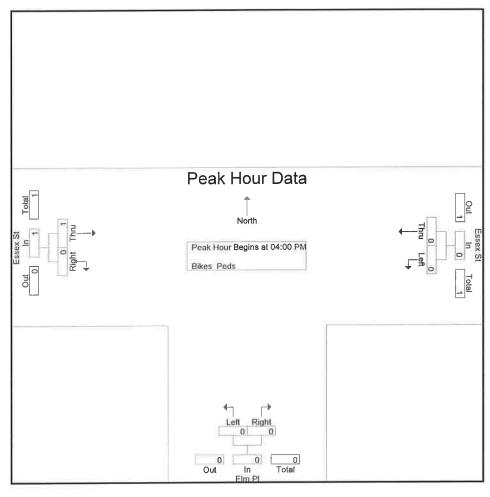
		Grou	os Printed- Trucks				
	Essex St From East		Elm Pl From South		Essex St From Wes		
Start Time	Left	Thru	Left	Right	Thru	Right	Int. Total
04:00 PM	0	0	0	0	0	0	0
04:15 PM	0	1	0	0	2	0	3
04:30 PM	0	1	0	0	0	0	1
04:45 PM	0	0	0	0	1	0	1
Total	0	2	0	0	3	0	5
05:00 PM	0	1	0	0	0	0	1
05:15 PM	0	1	0	0	1	0	2
05:30 PM	0	1	0	0	0	0	1
05:45 PM	0	1	0	0	2	0	3
Total	0	4	0	0	3	0	7
Grand Total	0	6	0	0	6	0	12
Apprch %	0	100	0	0	100	0	
Total %	0	50	0	0	50	0	

		Essex St From East		F	Elm Pl From South					
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04	:00 PM to 0	5:45 PM - F	Peak 1 of 1							
Peak Hour for Entire Intersed										
05:00 PM	0	1	1	0	0	0	0	0	0	1
05:15 PM	0	1	1	0	0	0	1	0	1	2
05:30 PM	0	1	1	0	0	0	0	0	0	1
05:45 PM	0	1	1	0	0	0	2	0	2	3
Total Volume	0	4	4	0	0	0	3	0	3	7
% App. Total	0	100		0	0		100	0		
PHF	.000	1.00	1.00	.000	.000	.000	.375	.000	.375	.583

				(	Groups Prir	ted-Bikes	Peds					
		ssex St om East			Elm Pl om South		Essex St From West			1		
Start Time	Left	Thru	Peds	Left	Right	Peds	Thru	Right	Peds	Exclu. Total	Inclu, Total	Int. Total
04:00 PM	0	0	0	0	0	1	0	0	0	1	0	1
04:15 PM	0	0	0	0	0	1	0	0	0	1	0	1
04:30 PM	0	0	0	0	0	2	1	0	0	2	1	3
04:45 PM	0	0	0	0	0	1	0	0	0	1	0	1
Total	0	0	0	0	0	5	1	0	0	5	1	6
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	0	0	0	0	0	1	0	0	0	1	0	1
05:30 PM	0	0	0	0	0	0	1	0	0	0	1	1
05:45 PM	0	0	0	0	0	1	0	0	0	1	0	1
Total	0	0	0	0	0	2	1	0	0	2	1	3
Grand Total	0	0	0	0	0	7	2	0	0	7	2	9
Apprch %	Ō	0		0	0		100	0				
Total %	Ō	0		0	0		100	0		77.8	22.2	

	Essex St From East			F	Elm Pl From South					
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Tota
Peak Hour Analysis From 04	1:00 PM to 0	5:45 PM - I	Peak 1 of 1							
Peak Hour for Entire Interse	ction Begins	at 04:00 P	Μ						i.	
04:00 PM	o	0	0	0	0	0	0	0	0	0
04:15 PM	0	0	0	0	0	0	0	0	0	0
04:30 PM	0	0	0	0	0	0	1	0	1	1
04:45 PM	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	1	0	1	1
% App. Total	Ō	ō		0	0		100	0		
PHF	.000	.000	.000	.000	.000	.000	.250	.000	.250	.250

File Name : 86880003 Site Code : 86880003 Start Date : 12/2/2020 Page No : 11



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

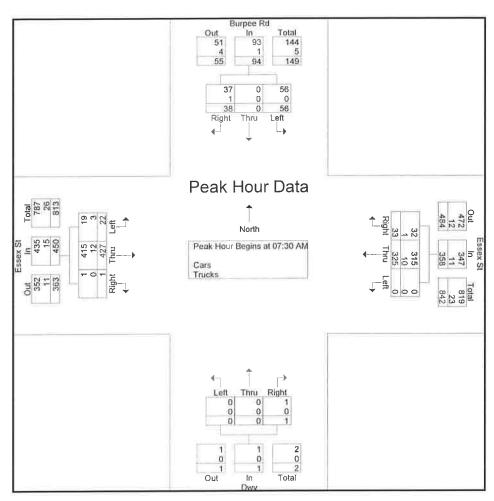
	04:00 PM			04:00 PM			04:00 PM		
+0 mins.	0	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	0	1	0	1
+45 mins.	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	1	0	1
% App. Total		0		0	0		100	0	
PHF	.000	.000	.000	.000	.000	.000	.250	.000	.250

					Groups P	rinted- Car	s - Trucks						
		urpee Rd om North			Essex St rom East			Dwy om South		Fi	Essex St rom West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	13	0	4	0	57	4	0	0	0	5	92	0	175
07:15 AM	11	0	3	0	60	4	0	0	0	5	75	0	158
07:30 AM	12	0	6	0	78	3	0	0	1	4	103	0	207
07:45 AM	15	0	17	0	84	8	0	0	0	2	129	1	256
Total	51	0	30	0	279	19	0	0	1	16	399	1	796
08:00 AM	17	0	7	0	89	14	0	0	0	9	99	0	235
08:15 AM	12	0	8	0	74	8	0	0	0	7	96	0	205
08:30 AM	20	Ō	11	0	74	8	0	0	0	7	77	0	197
08:45 AM	18	0	6	0	61	10	0	0	0	6	98	0	199
Total	67	0	32	0	298	40	0	0	0	29	370	0	836
Grand Total	118	0	62	0	577	59	0	0	1	45	769	1	1632
Apprch %	65.6	0	34.4	0	90.7	9.3	0	0	100	5.5	94.4	0.1	
Total %	7.2	0	3.8	0	35.4	3.6	0	0	0.1	2.8	47.1	0.1	
Cars	118	0	58	0	561	58	0	0	1	41	749	1	1587
% Cars	100	0	93.5	0	97.2	98.3	0	0	100	91.1	97.4	100	97.2
Trucks	0	Ó	4	0	16	1	0	0	0	4	20	0	45
% Trucks	0	0	6.5	0	2.8	1.7	0	0	0	8.9	2.6	0	2.8

			ee Rd North				ex St 1 East				wy South				ex St West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App, Total	Left	Thru	Right	App. Total	Left	Thru	Right	App, Total	Int. Total
Peak Hour Analy	sis Fron	n 07:00	AM to C	8:45 AM -	Peak 1	of 1											
Peak Hour for Er	ntire Inte	rsectior	Begins	at 07:30	٩M							2					
07:30 AM	12	0	6	18	0	78	3	81	0	0	1	1	4	103	0	107	207
07:45 AM	15	0	17	32	0	84	8	92	0	0	0	0	2	129	1	132	256
08:00 AM	17	0	7	24	0	89	14	103	0	0	0	0	9	99	0	108	235
08:15 AM	12	0	8	20	0	74	8	82	0	0	0	0	7	96	0	103	205
Total Volume	56	0	38	94	0	325	33	358	0	0	1	1	22	427	1	450	903
% App. Total	59.6	0	40.4		0	90,8	9.2		0	0	100		4.9	94.9	0.2		
PHF	.824	.000	.559	.734	.000	.913	.589	.869	.000	.000	.250	.250	.611	.828	.250	.852	.882
Cars	56	0	37	93	0	315	32	347	0	0	1	1	19	415	1	435	876
% Cars	100	0	97.4	98.9	0	96.9	97.0	96.9	0	0	100	100	86.4	97.2	100	96.7	97.0
Trucks	0	0	1	1	0	10	1	11	0	0	0	0	3	12	0	15	27
% Trucks	0	0	2.6	1.1	0	3.1	3.0	3.1	0	0	0	0	13.6	2.8	0	3.3	3.0

N/S Street : Burpee Road / Driveway E/W Street : Essex Street City/State : Swampscott, MA Weather : Clear

File Name	86880004
Site Code	86880004
Start Date	::12/2/2020
Page No	: 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

	07:45 AM				07:45 AM				07:00 AM				07:30 AM			
+0 mins	15	0	17	32	0	84	8	92	0	0	0	0	4	103	0	107
+15 mins.	17	0	7	24	0	89	14	103	0	0	0	0	2	129	1	132
+30 mins.	12	0	8	20	0	74	8	82	0	0	1	1	9	99	0	108
+45 mins.	20	0	11	31	0	74	8	82	0	0	0	0	7	96	0	103
otal Volume	64	0	43	107	0	321	38	359	0	0	1	1	22	427	1	450
6 App. Total	59.8	0	40.2		0	89.4	10.6		0	0	100		4.9	94.9	0.2	
PHF	.800	.000	.632	.836	.000	.902	.679	.871	.000	.000	.250	,250	.611	.828	.250	.852
Cars	64	0	41	105	0	311	37	348	0	0	1	1	19	415	1	435
% Cars	100	0	95.3	98.1	0	96.9	97.4	96.9	0	0	100	100	86.4	97.2	100	96.7
Trucks	0	0	2	2	0	10	1	11	0	0	0	0	3	12	0	15
% Trucks	0	0	4.7	1.9	0	3.1	2.6	3.1	0	0	0	0	13.6	2.8	0	3.3

		ssex St om West	_		Dwy om South	Fro		ssex St om East			arpee Rd om North		
Int. Tot	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Start Time
17	0	91	4	0	0	0	4	55	0	4	0	13	07:00 AM
15	0	73	5	0	0	0	4	59	0	2	0	11	07:15 AM
20	0	100	4	1	0	0	3	77	0	5	0	12	07:30 AM
25	1	123	2	0	0	0	8	84	0	17	Ō	15	07:45 AM
77	1	387	15	1	0	0	19	275	0	28	0	51	Total
22	0	97	9	0	0	0	13	85	0	7	0	17	08:00 AM
19	0	95	4	0	0	0	8	69	0	8	Ō	12	08:15 AM
19	0	75	7	0	0	0	8	73	0	9	Ō	20	08:30 AM
19	0	95	6	0	0	0	10	59	0	6	0	18	08:45 AM
81	0	362	26	0	0	0	39	286	0	30	0	67	Total
158	1	749	41	1	0	0	58	561	0	58	0	118	Grand Total
	0.1	94.7	5.2	100	0	0	9.4	90.6	0	33	Ő	67	Apprch %
	0.1	47.2	2.6	0.1	0	0	3.7	35.3	0	3.7	Ō	7.4	Total %

			ee Rd North				ex St 1 East				wy South				ex St n West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int, Total
Peak Hour Analy	sis Fror	n 07:00	AM to C	8:45 AM -	Peak 1	of 1											
Peak Hour for Er								57									
07:30 AM	12	0	5	17	0	77	3	80	0	0	1	1	4	100	0	104	202
07:45 AM	15	0	17	32	0	84	8	92	0	0	0	0	2	123	1	126	250
08:00 AM	17	Ō	7	24	0	85	13	98	0	0	0	0	9	97	0	106	228
08:15 AM	12	0	8	20	0	69	8	77	0	0	0	0	4	95	0	99	196
Total Volume	56	0	37	93	0	315	32	347	0	0	1	1	19	415	1	435	876
% App. Total	60.2	ŏ	39.8		0	90.8	9.2		0	0	100		4.4	95.4	0.2		
PHF	.824	.000	.544	.727	.000	.926	.615	.885	.000	.000	.250	.250	.528	.843	.250	.863	.876

					Group	s Printed- 1	rucks						
		urpee Rd om North			ssex St om East		Fro	Dwy om South			ssex St om West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	0	0	0	0	2	0	0	0	0	1	1	0	4
07:15 AM	0	0	1	0	1	0	0	0	0	0	2	0	4
07:30 AM	0	0	1	0	1	0	0	0	0	0	3	0	5
07:45 AM	0	0	0	0	0	0	0	0	0	0	6	0	6
Total	0	0	2	0	4	0	0	0	0	1	12	0	19
08:00 AM	0	0	0	0	4	1	0	0	0	0	2	0	7
08:15 AM	0	0	0	0	5	0	0	0	0	3	1	0	9
08:30 AM	0	0	2	0	1	0	0	0	0	0	2	0	5
08:45 AM	0	0	0	0	2	0	0	0	0	0	3	0	5
Total	0	0	2	0	12	1	0	0	0	3	8	0	26
Grand Total	0	0	4	0	16	1	0	0	0	4	20	0	45
Apprch %	Ō	0	100	0	94.1	5.9	0	0	0	16.7	83.3	0	
Total %	0	0	8.9	0	35.6	2.2	0	0	0	8.9	44.4	0	

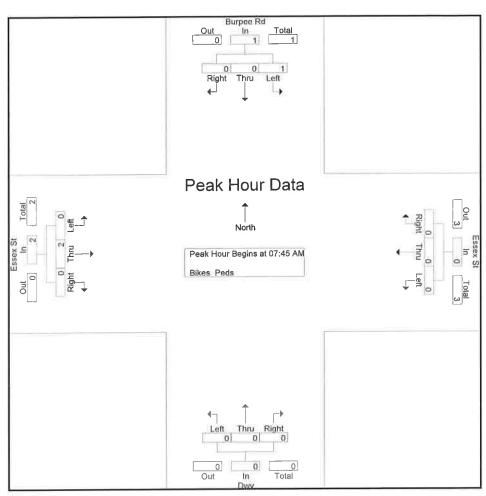
			ee Rd North				ex St 1 East				wy South				ex St i West		
Start Time	Left	Thru	Right	App, Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis Fror	n 07:00	AM to C	8:45 AM	- Peak 1	of 1											
Peak Hour for Er	ntire Inte	rsection	Begins	at 07:30	AM												
07:30 AM	0	0	1	1	0	1	0	1	0	0	0	0	0	3	0	3	5
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	6	0	6	6
08:00 AM	0	0	0	0	0	4	1	5	0	0	0	0	0	2	0	2	7
08:15 AM	0	0	0	0	0	5	0	5	0	0	0	0	3	1	0	4	9
Total Volume	0	0	1	1	0	10	1	11	0	0	0	0	3	12	0	15	27
% App. Total	0	0	100		0	90.9	9.1		0	0	0		20	80	0		
PHF	.000	.000	.250	.250	.000	.500	.250	.550	.000	.000	.000	.000	.250	.500	.000	.625	.750

								Groups	Printed	- Bikes	Peds								
		Burpe From				Esse From	ex St East			From	wy South			From					
Start Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu Total	Inclu Total	Int. Total
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	1
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	1
07:30 AM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1
07:45 AM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	2	0	2
Total	0	0	0	2	0	0	0	0	0	0	0	1	0	0	0	2	5	0	5
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	1
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	1	2
08:30 AM	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	1	2
08:45 AM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1
Total	1	0	0	2	0	0	0	0	0	0	0	0	0	2	0	1	3	3	6
Grand Total	1	0	0	4	0	0	0	0	0	0	0	1	0	2	0	3	8	3	11
Apprch %	100	0	0		0	0	0		0	0	0		0	100	0		70.7	07.0	
Total %	33.3	0	0		0	0	0		0	0	0		0	66.7	0		72.7	27.3	

			ee Rd North				ex St n East			-	wy South				ex St 1 West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analys	sis Fron	n 07:00	AM to C	08:45 AM	- Peak 1	of 1											
Peak Hour for En	tire Inte	rsectior	Begins	at 07:45	AM												
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1
08:30 AM	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
Total Volume	1	0	0	1	0	0	0	0	0	0	0	0	0	2	0	2	3
% App. Total	100	0	0		0	0	0		0	0	0		0	100	0		
PHF	.250	.000	.000	.250	.000	.000	.000	.000	.000	.000	.000	.000	.000	.500	.000	.500	.750

N/S Street : Burpee Road / Driveway E/W Street : Essex Street City/State : Swampscott, MA Weather : Clear

File Name	: 86880004
Site Code	86880004
Start Date	12/2/2020
Page No	: 11



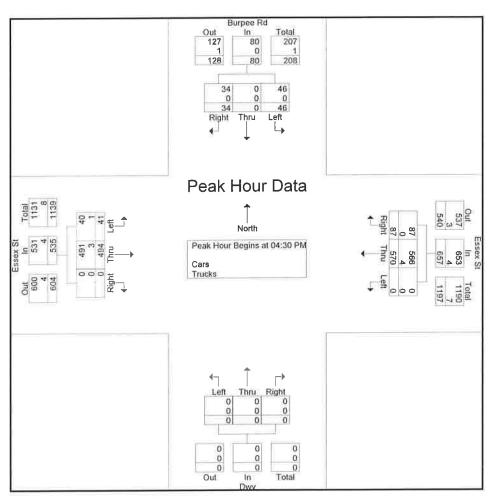
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

eak nour for La			Sauna au						07-00 414				07:30 AM			
	07 45 AM				07:00 AM				07:00 AM				07:30 AIVI			
+0 mins. *	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1
+45 mins.	1	0	0	1	0	0	0	0	0	0	0	0	0	1	0	1
Total Volume	1	0	0	1	0	0	0	0	0	0	0	0	0	2	0	2
% App. Total	100	0	0		0	0	0		0	0	0		0	100	0	
PHF	.250	.000	.000	.250	.000	.000	.000	.000	.000	.000	.000	.000	.000	500	.000	.500

- 0 - 10		ssex St om West			Dwy om South	Fro		ssex St om East			rpee Rd		
Int. Tota	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Start Time
315	0	137	10	0	0	0	16	139	0	4	0	9	04:00 PM
310	0	118	7	0	0	1	14	147	0	6	1	16	04:15 PM
286	0	111	6	0	0	0	19	129	0	8	0	13	04:30 PM
342	0	141	17	0	0	0	20	144	0	8	0	12	04:45 PM
1253	0	507	40	0	0	1	69	559	0	26	1	50	Total
311	0	116	8	0	0	0	25	144	0	8	0	10	05:00 PM
333	0	126	10	0	0	0	23	153	0	10	0	11	05:15 PM
271	0	104	8	1	0	0	23	118	0	7	0	10	05:30 PM
276	0	114	15	0	0	0	13	118	0	6	1	9	05:45 PM
1191	0	460	41	1	0	0	84	533	0	31	1	40	Total
2444	0	967	81	1	0	1	153	1092	0	57	2	90	Grand Total
	0	92.3	7.7	50	0	50	12.3	87.7	0	38.3	1.3	60.4	Apprch %
	0	39.6	3.3	0	0	0	6.3	44.7	0	2.3	0.1	3.7	Total %
2427	0	958	80	1	0	1	153	1085	0	57	2	90	Cars
99.3	0	99.1	98.8	100	0	100	100	99.4	0	100	100	100	% Cars
17	0	9	1	0	0	0	0	7	0	0	0	0	Trucks
0.7	0	0.9	1.2	0	0	0	0	0.6	0	0	0	Ó	% Trucks

			ee Rd North				ex St i East				wy South				ex St i West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis Fron	n 04:00	PM to C	5:45 PM -	Peak 1	of 1											
Peak Hour for Er	ntire Inte	rsection	Begins	at 04:30	PM												
04:30 PM	13	0	8	21	0	129	19	148	0	0	0	0	6	111	0	117	286
04:45 PM	12	0	8	20	0	144	20	164	0	0	0	0	17	141	0	158	342
05:00 PM	10	0	8	18	0	144	25	169	0	0	0	0	8	116	0	124	311
05:15 PM	11	0	10	21	0	153	23	176	0	0	0	0	10	126	0	136	333
Total Volume	46	Ő	34	80	0	570	87	657	0	0	0	0	41	494	0	535	1272
% App. Total	57.5	0	42.5		0	86.8	13.2		0	0	0	-	7.7	92.3	0		
PHF	.885	.000	.850	.952	.000	.931	.870	.933	.000	.000	.000	.000	.603	.876	.000	.847	.930
Cars	46	0	34	80	0	566	87	653	0	0	0	0	40	491	0	531	1264
% Cars	100	0	100	100	0	99.3	100	99.4	0	0	0	0	97.6	99.4	0	99.3	99.4
Trucks	0	Ō	0	0	0	4	0	4	0	0	0	0	1	3	0	4	8
% Trucks	Ő	Ō	0	0	0	0.7	0	0.6	0	0	0	0	2.4	0.6	0	0.7	0.6

File Name	86880004
Site Code	: 86880004
Start Date	12/2/2020
Page No	:2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

eak mour for E	acii Appi	Uacii Di	egins at.													
	04:15 PM				04:30 PM				04:00 PM				04:00 PM			
+0 mins	16	1	6	23	0	129	19	148	0	0	0	0	10	137	0	147
+15 mins.	13	0	8	21	0	144	20	164	1	0	0	1	7	118	0	125
+30 mins.	12	0	8	20	0	144	25	169	0	0	0	0	6	111	0	117
+45 mins.	10	0	8	18	0	153	23	176	0	0	0	0	17	141	0	158
otal Volume	51	1	30	82	0	570	87	657	1	0	0	1	40	507	0	547
% App. Total	62.2	1.2	36.6		0	86.8	13.2		100	0	0		7.3	92.7	0	
PHF	797	.250	.938	.891	.000	.931	.870	.933	.250	.000	.000	.250	.588	.899	.000	.866
Cars	51	1	30	82	0	566	87	653	1	0	0	1	39	502	0	541
% Cars	100	100	100	100	0	99.3	100	99.4	100	0	0	100	97.5	99	0	98.9
Trucks	0	0	0	0	0	4	0	4	0	0	0	0	1	5	0	6
% Trucks	õ	Ō	Ō	0	0	0.7	0	0.6	0	0	0	0	2.5	1	0	1.1

						Cars	s Printed-	Group					
		ssex St om West			Dwy om South	Fre		ssex St om East			rpee Rd		
Int. Tota	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Start Time
314	0	136	10	0	0	0	16	139	0	4	0	9	04:00 PM
30	0	116	7	0	0	1	14	146	0	6	1	16	04:15 PM
283	0	110	6	0	0	0	19	127	0	8	0	13	04:30 PM
340	0	140	16	0	0	0	20	144	0	8	0	12	04:45 PM
1244	0	502	39	0	0	1	69	556	0	26	1	50	Total
31(	0	116	8	0	0	0	25	143	0	8	0	10	05:00 PM
331	0	125	10	0	0	0	23	152	0	10	0	11	05:15 PM
270	0	104	8	1	0	0	23	117	0	7	0	10	05:30 PM
273	0	111	15	0	0	0	13	117	0	6	1	9	05:45 PM
118:	0	456	41	1	0	0	84	529	0	31	1	40	Total
242	0	958	80	1	0	1	153	1085	0	57	2	90	Grand Total
	0	92.3	7.7	50	0	50	12.4	87.6	0	38.3	1.3	60.4	Apprch %
1	0	39.5	3.3	0	0	0	6.3	44.7	0	2.3	0.1	3.7	Total %

			ee Rd North				ex St n East				wy South				sex St 1 West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis Fron	n 04:00	PM to C	5:45 PM -	Peak 1	of 1											
Peak Hour for Er																	16
04:30 PM	13	0	8	21	0	127	19	146	0	0	0	0	6	110	0	116	283
04:45 PM	12	0	8	20	0	144	20	164	0	0	0	0	16	140	0	156	340
05:00 PM	10	Ō	8	18	0	143	25	168	0	0	0	0	8	116	0	124	310
05:15 PM	11	0	10	21	0	152	23	175	0	0	0	0	10	125	0	135	331
Total Volume	46	0	34	80	0	566	87	653	0	0	0	0	40	491	0	531	1264
% App. Total	57.5	Ō	42.5		0	86.7	13.3		0	0	0		7.5	92.5	0		
PHF	.885	.000	.850	.952	.000	.931	.870	.933	.000	.000	.000	.000	.625	.877	.000	.851	.929

					Group	s Printed- 1	Trucks						
		urpee Rd om North			ssex St om East			Dwy om South			ssex St om West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	0	0	0	0	0	0	0	0	0	0	1	0	1
04:15 PM	0	0	0	0	1	0	0	0	0	0	2	0	3
04:30 PM	0	0	0	0	2	0	0	0	0	0	1	0	3
04:45 PM	0	0	0	0	0	0	0	0	0	1	1	0	2
Total	0	0	0	0	3	0	0	0	0	1	5	0	9
05:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	1
05:15 PM	0	0	0	0	1	0	0	0	0	0	1	0	2
05:30 PM	0	0	0	0	1	0	0	0	0	0	0	0	1
05:45 PM	0	0	0	0	1	0	0	0	0	0	3	0	4
Total	0	0	0	0	4	0	0	0	0	0	4	0	8
Grand Total	0	0	0	0	7	0	0	0	0	1	9	0	17
Apprch %	0	0	0	0	100	0	0	0	0	10	90	0	
Total %	0	0	0	0	41.2	0	0	0	0	5,9	52.9	0	

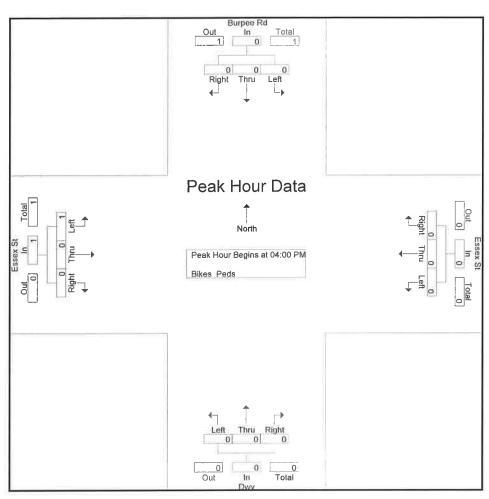
			ee Rd North				ex St i East			_	wy South				ex St West		
Start Time	Left			App. Total	Left	Thru		App. Total	Left	Thru		App, Total	Left	Thru		App. Total	Int. Total
Peak Hour Analy	sis Fror				Peak 1	of 1					10						
Peak Hour for Er	ntire Inte	rsection	n Begins	at 04:00	PM			10				1					
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1
04:15 PM	0	0	0	0	0	1	0	1	0	0	0	0	0	2	0	2	3
04:30 PM	0	0	0	0	0	2	0	2	0	0	0	0	0	1	0	1	3
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	2	2
Total Volume	0	0	0	0	0	3	0	3	0	0	0	0	1	5	0	6	9
% App. Total	0	0	0		0	100	0		0	0	0		16.7	83.3	0		
PHF	.000	.000	.000	.000	.000	.375	.000	.375	.000	.000	.000	.000	.250	.625	.000	.750	.750

								Groups	Printed	1- Bikes	Peds								
		Burpe					ex St			D					ex St				
		From	North			From	East				South			From					1.000 1.00000000
Start Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu, Total	Inclu Total	Int. Total
04:00 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	2	0	2
04:15 PM	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	1
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	3	0	0	0	0	0	0	0	0	1	0	0	1	4	1	5
												7							
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:30 PM	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	1
Total	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1	2
Grand Total	1	0	0	3	0	0	0	0	0	0	0	0	1	0	0	2	5	2	7
Apprch %	100	0	0		0	0	0		0	0	0		100	0	0				
Total %	50	0	0		0	0	0		0	0	0		50	0	0		71.4	28.6	

			ee Rd North				ex St 1 East			-	wy South				ex St 1 West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App, Total	Left	Thru	Right	App. Total	Int, Total
Peak Hour Analy	sis Fron	n 04:00	PM to C	5:45 PM -	Peak 1	of 1											
Peak Hour for Er	ntire Inte	rsection	Begins	at 04:00	PM												
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	1
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	1
% App. Total	0	0	0		0	0	0		0	0	0		100	0	0		
PHF	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.250	.000	.000	.250	.250

N/S Street : Burpee Road / Driveway E/W Street : Essex Street City/State : Swampscott, MA Weather : Clear

File Name	:86880004
Site Code	: 86880004
Start Date	12/2/2020
Page No	11



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

	04:45 PM				04:00 PM				04:00 PM				04.00 PM						
+0 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
+15 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
+30 mins.	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1			
+45 mins.	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0			
Total Volume	1	0	0	1	0	0	0	0	0	0	0	0	1	0	0	1			
% App. Total	100	0	-0		0	0	0		0	0	0		100	0	0				
PHF	.250	.000	.000	.250	.000	.000	.000	.000	.000	.000	.000	.000	.250	.000	.000	.250			

SEASONAL ADJUSTMENT DATA

# 8087: Monthly Hourly Volume for December 2018 Massachusetts Highway Department

		QC Status	Accepted		Accepted																												
		TOTAL	58874	51925	60268	60496	60830	62528	64409	58895	54344	59408		61980	63749	65511	60265	53319	61102	62655	64548	65451	63923	61638	56170	58031	43128	56879	59047	59322	56606	51041	57301
		L 00:EZ	1958	1372	1207	1261	1425	1599	1885	1792	1216	1249		1492	1743	1983	2102	1336	1249	1408	1743	1945	2136	2114	1586	1784	1189	1333	1357	1554	1681	1329	1599
		22:00	2221	1786	1826	1790	1980	2039	2362	2132	1705	1822		1965	2253	2377	2385	1688	1904	2006	2117	2356	2454	2323	1963	2284	1565	1681	1863	1991	2004	1762	2020
		21:00	2286	1943	2139	2279	2450	2235	2523	2310	2068	2085		2324	2351	2528	2501	2052	2181	2401	2426	2514	2501	2561	2313	2538	2021	1997	2200	2156	2120	1937	2154
		20:00	2470	2577	2292	2389	2359	2655	2587	2490	2613	2364		2565	2843	2752	2630	2542	2389	2546	2639	2854	2654	2725	2556	2534	2441	2223	2259	2353	2521	2239	2369
		19:00	2760	2679	2695	3008	2983	3027	3384	2825	3083	2835		3057	3088	3241	2959	2706	2809	3062	3192	3175	3011	3123	2715	2578	2567	2730	2713	2735	2741	2140	2631
		18:00	3510	2783	3181	3469	3392	3638	3649	3327	3394	3221		3406	3446	3604	3548	2800	3483	3499	3545	3464	3303	3307	3085	2719	2772	2927	3006	3181	3169	2911	2839
		17:00	3448	3041	3546	3683	3668	3742	3773	3443	3604	3542		3805	3803	3768	3492	3046	3591	3728	3793	3747	3601	3485	3365	3197	2772	3437	3766	3413	3335	3304	3175
		16:00	3699	3360	3746	3774	3716	3786	3929	3696	3406	3570		3829	3822	3888	3731	3440	3749	3922	3919	3652	3709	3559	3353	3298	2702	3662	3780	3634	3434	3262	3367
U3 U3		15:00	3835	3382	3706	3722	3831	3904	3742	3752	3451	3829		4009	3869	3949	3867	3622	3807	3784	3975	3845	3615	3612	3402	3422	2721	3840	3922	3775	3665	3257	3764
Seasonal Factor Group: U Daily Factor Group: Axle Factor Group: Growth Factor Group:		14:00	3781	3505	3717	3613	3811	3717	3927	3738	3619	3649		3895	3893	4001	3876	3791	3785	3779	3903	3926	3802	3785	3619	3721	3048	3790	3676	3880	3735	3448	3859
	-	13:00	3628	3592	3551	3277	3565	3528	3624	3799	3759	3263		3548	3712	3602	3672	3698	3577	3644	3659	3881	3669	3818	3778	3681	2890	3527	3500	3598	3682	3378	3745
		12:00	3442	3425	3410	3248	3230	3520	3566	3775	3671	3362		3395	3487	3556	3622	3493	3544	3479	3727	3782	3607	3663	3689	3779	2551	3588	3448	3642	3521	3379	3772
Seasona Daily Fa Axle Fau		11:00	3377	3168	3218	3195	3022	3300	3357	3417	3195	3149		3063	3327	3455	3372	3148	3354	3322	3501	3519	3448	3620	3513	3313	2231	3221	3190	3263	3292	3203	3296
		10:00	3088	2734	3242	3431	3187	3477	3205	3155	2906	3172		3235	3541	3439	3076	2823	3289	3449	3482	3329	3358	3394	3245	3215	1724	3105	3075	3088	2972	2755	2882
		9:00	2914	2328	3412	3450	3237	3344	3369	2980	2368	3239		3396	3485	3302	2830	2379	3314	3449	3320	3391	3194	2988	2658	2919	1293	2751	2776	2847	2575	2259	2708
		8:00	2579	1731	3153	3173	3174	3225	3268	2517	1874	3249		3169	3158	3348	2405	1841	3103	3237	3174	3282	3200	2386	1998	2672	961	2584	2795	2691	2230	1761	2512
		7:00	2117	1377	2987	3046	3122	3029	3168	2133	1470	3085		3078	3188	3195	2003	1411	3038	3082	3155	3135	2972	2062	1526	2229	766	2447	2761	2606	1810	1396	2316
	/AY	6:00	1908	1182	2894	2839	2846	2885	2919	1767	1218	2871		2824	2913	2975	1754	1322	2786	2918	2910	2966	2795	1746	1422	2121	929	2550	2779	2636	1836	1308	2227
8087 Suffolk 3 I FF RI IRRANK HIGHMAAV	,	5:00	1434	950	2479	2571	2389	2412	2353	1387	982	2437		2480	2353	2363	1378	1006	2373	2274	2459	2503	2240	1444	1157	1740	1022	2081	2261	2161	1506	1288	1963
RANK		4:00	913	850	1131	1079	1112	1125	1098	878	724	1154		1099	1070	1099	950	773	1152	1166	1195	1253	1264	1049	927	1005	836	1079	1195	1187	1102	955	1055
8087 Suffolk 3 I EF RI IR		3:00	727	774	622	568	600	598	647	682	669	575		577	611	684	793	753	651	655	658	3 725	9 762	883	854	t 694	689	t 671	2 725	5 759	849	1 809	2 722
		2:00	752	893	478	410	471	464	509	757	878	381		493	450	1 578	5 781	969	3 517	491	538	569	679	L 1029	3 946	9 624	5 847	0 414	1 552	9 655	9 766	5 821	4 602
lass		1:00	827	1052	708	510	528	508	635	688 1	1111 (	554		556	l 562	0 754	2 1066	1090	1 623	7 567	9 649	5 642	0 839	5 1361	2 1098	5 799	5 1085	1 520	7 661	8 649	1 909	5 1005	D 754
Location ID: County: Funcation Class		0:00	1200	1441	928	711	732	771	930	1254	1330	751		720	781	1070	1472	1590	834	787	869	966	1110	1595	1402	1165	1275	721	787	868	1151	1135	970
Location County: Funcation	2		1	2	£	4	S	9	7	83	6h	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31

1.03 Adjustment Factor =

60868 Average Yearly =

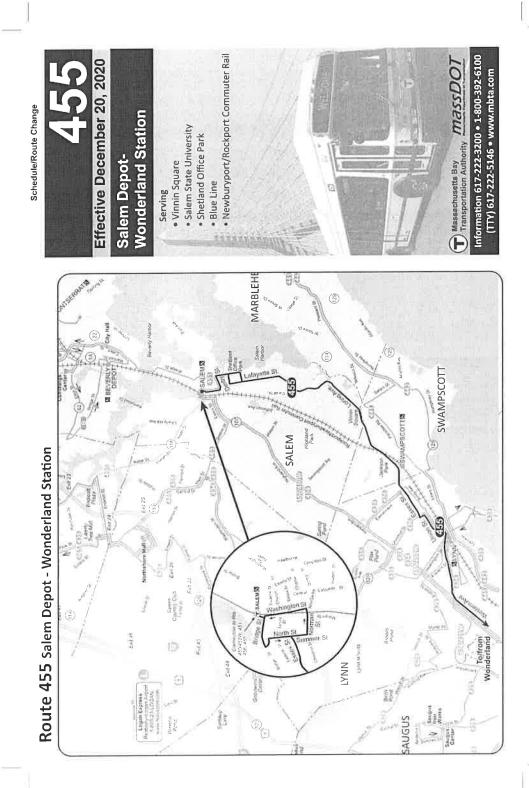
59121.43 Average Monthly =

	QC Status Accepted	Accepted																													
	TOTAL 52625	51852	51880	54411	39701	40909	50085	49930	50648	51602	54894	47118	43876	49428	52476	54314	51880	49616	49837	42647	53862	55762	57788	53287	28548	44120	42731	51114	52245	54766	52121
	23:00 1 817	864	848	1196	849	804	608	752	789	892	1211	1139	864	860	885	851	069	1180	1166	972	926	986	1199	1163	852	1069	904	886	906	1039	1059
	22:00 1394	1251	1283	1638	1233	1165	1167	1178	1201	1320	1610	1427	1271	1155	1280	1197	979	1711	1585	1339	1298	1434	1673	1459	1098	1400	1288	1228	1265	1456	1480
	21:00 1716	1634	1637	1843	1352	1624	1566	1536	1703	1540	1911	1667	1567	1522	1656	1612	1055	1855	1865	1627	1733	1883	2012	1837	1292	1710	1505	1572	1632	1729	1660
	20:00 1891	1932	1906	2127	1300	1795	1811	1716	1791	1907	2088	1823	1850	1729	1893	1944	1145	2010	2064	1781	1809	2035	2245	1965	1431	1855	1693	1755	1699	2004	1980
	<b>19:00</b> 2246	2238	2240	2520	1493	2108	2073	2150	2203	2181	2515	2121	2146	2075	2319	2417	1274	2519	2309	2056	2395	2596	2701	2164	1651	2077	1947	2085	2235	2465	2253
	<b>18:00</b> 2903	2841	2934	2997	1845	2331	2613	2703	2687	2883	2983	2508	2468	2574	2791	2967	1517	2928	2653	2270	2831	3190	3177	2701	1685	2516	2285	2682	2929	2846	2780
	<b>17:00</b> 3444	3396	3332	3557	2171	2525	3177	3246	3333	3476	3599	2710	2935	3217	3469	3671	1559	2969	3027	2600	3461	3564	3576	2970	1746	2812	2702	3317	3215	3337	2990
	<b>16:00</b> 3556	3451	3552	3667	2436	2946	3516	3324	3439	3614	3746	3078	3111	3384	3770	3670	1579	1895	3264	2872	3557	3653	3605	3206	1829	3065	3051	3389	3460	3613	3344
ოო	15:00 3651	3687	3649	3755	2681	3246	3646	3582	3513	3659	3809	3220	3340	3465	3606	3821	1574	2313	3448	2963	3630	3802	3978	3457	1931	3164	3219	3588	3670	3815	3588
n3 N	<b>14:00</b> 3570	3500	3488	3693	3066	3256	3447	3460	3496	3591	3591	3458	3527	3328	3542	3734	1539	3504	3497	3070	3690	3667	3990	3788	2048	3210	3335	3634	3590	3787	3654
Group: up: iroup:	<b>13:00</b> 3064	3122	3211	3249	2945	3332	3094	3053	3142	3087	3388	3347	3490	3027	3170	3332	1276	3326	3541	3254	3447	3613	3677	3722	1948	3302	3414	3415	3324	3585	3702
Seasonal Factor Group: Daily Factor Group: Axle Factor Group: Growth Factor Group:	<b>12:00</b> 3145	3070	2952	3137	3013	2989	2958	2959	2979	2916	3323	3393	3242	2809	3077	3286	1122	3047	3346	3032	3303	3348	3473	3876	1837	3108	3211	3278	3365	3422	3501
easonal aily Fac xle Fact rowth I	11:00 2861	2864	2852	2940	2757	2764	2713	2821	2937	2759	2941	3100	3044	2725	2942	3175	932	3074	3177	2967	3324	3186	3469	3549	1482	2972	2865	3151	3135	3426	3187
N D A D	10:00 2771	2594	2740	2763	2449	2282	2627	2580	2649	2595	2828	2889	2361	2630	2719	2926	795	2708	2909	2508	2966	3045	3168	3255	1142	2520	2344	2936	3023	3098	2992
	9:00 2754	2515	2614	2670	2195	1767	2532	2643	2460	2554	2642	2437	1969	2517	2629	2692	679	2611	2438	2108	2816	2847	2879	2875	912	2024	1880	2625	2636	2842	2547
	8:00 2849	2826	2788	2832	1736	1260	2817	2716	2811	2787	2791	1992	1428	2731	2781	2970	765	2649	1951	1413	2851	2630	2948	2358	684	1626	1391	2384	2558	2678	2395
	7:00 3158	3056	2987	2936	1528	991	2958	2892	2927	2990	2994	1702	1182	2921	2928	3029	803	2689	1712	1224	3019	2982	2915	2321	577	1399	1269	2543	2643	2652	2354
>	<b>6:00</b> 2707	2827	2769	2733	1297	955	2651	2669	2624	2789	2743	1411	971	2709	2802	2783	812	2486	1644	1066	2548	2660	2585	2153	661	1246	1092	2491	2552	2552	2352
8087 Suffolk 3 LEE BURBANK HIGHWAY	5:00 1772	1800	1803	1815	863	624	1669	1722	1752	1785	1747	1058	694	1733	1856	1859	625	1685	1158	756	1808	1896	1885	1586	616	819	754	1756	1877	1815	1649
BANK F	4:00 754	829	760	787	606	421	733	705	705	746	762	595	526	702	754	739	380	798	717	590	754	839	862	784	534	675	601	811	823	898	808
8087 Suffolk 3 LEE BUR	3:00 402	449	424	451	446	336	392	428	398	443	444	436	375	438	442	455	300	508	532	477	474	530	502	520	521	452	405	406	485	484	537
~~~~	2:00 309	301	268	284	324	313	276	308	287	279	300	384	371	321	319	317	245	341	446	469	335	377	349	419	555	298	384	293	332	311	335
ISS	1:00 353	334	319	343	449	408	305	286	342	332	371	511	467	341	343	359	274	404	558	496	374	413	374	484	713	301	454	332	381	351	392
Location ID: County: Funcationl Class Location:	0:00 538	471	524	478	667	667	535	501	480	477	557	712	677	515	503	508	404	406	830	737	513	586	546	675	803	500	738	557	510	561	581
Location ID: County: Funcationl C Location:	1	2	£	4	5	9	7	00	6	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31

51202.43

10 New England Business Center Drive Vanasse & Associates, Inc. Suite 314 **Transportation Engineers & Planners** Andover, MA 01810-1066 Office 978-474-8800 **Calculations** Fax 978-688-6508 8630 Non. lasing the Development Job Number: Job: 1/6/2021 Swanpscon Date: Location: istpent Fretor of 4 Sheet Title: BC Checked by:\_ Calculated by: Dec: 2018 Average x 53,122 Dec. 2020 Average -2 51,203 59,122 × 1,01252 2 60,609 60,609 /51,203 = 1. 184 = Adjustment Factor for COVID

PUBLIC TRANSPORTATION INFORMATION



		a 5 🖶	A SA		0.10	15 or	0	5 -		. ന	5	ν.4	6.0					1.000													
		e Arrive n Salem The Depot	3A 5:55A			9 10:56 4 <b>12:11P</b>	001-1 DV		6 3:41 6 4:41			4 /:43 9 8:54		2 11:47				Bus +	Subway	\$2.40	\$4.10	54.10	\$1.10 \$1.10		FREE FARES: Children 11 and under ride free when accompanied by a paying customer; Blind Access CharlieCard holders ride free and if using a guide, the guide rides free.	rds available	lieCards	*****			
	Outbound	re Arrive ral Vinnin tre Square	13A 5:43A			24 10:39 39 11:54	07-1 001		11 3:26 11 4:26					18 11:32		Stand	cation.	10	subway		1		51.10 S1.10		ed by a paying the guide ri	nt CharlieCa	ton.	ג יכופטומנים וו		/21: Sat	
		/e ler- Arrive d Central on Square	. a 5:33A			0 10:24 5 11:39	00V-01 05		IS 3:11 IS 4:11					11:18 11:18		Empire	chool va								accompanie, ing a guide,	52.4U Card. Stude	high schools Greater Bosi	שובמורקוב בק		ays & 2/15	
Sunday		ler- Vonder- d Iand on Station	3A 6.15A	_	-	4 10:00 4P 11:15	19-950	_	9 2:45 9 3:45	-	-	8 8:05		10:58	8A	a - From/to West Lynn Garage e - From Brookline SI. & Emoire St and	does not run during school vacation.	d	Bus + Bus	\$1.70	\$1.70	\$3.40	\$0.85		le free when ree and if up	auth Charlie و م	middle and		Holial F	12/25/20 & 1/1/21 Sun; 1/18/21 & 2/15/21: Sat	
Ñ		ive Wonder- al land re Stalion	0A 6:03A			8 11:04 6 12:14P	anc-1 ac		1 3:39			5 8:48	,	8	8 12:18A	/to West Brooklii	not run	4	Local Bus	S1.70	\$1.70	50.05	50.85		nd under rid afders ride f	rlieCard or Y	articipating	bilines	000 101	21 Sun;	
	Inbound	e Lv'Arrive in Central ra Square	5:40A		b 8:33		5D 19-52D		4 3:11 4 4:21					5 11:28	5 11:58	a - From e - From	does					Board	Deve	2	hildren 11 a harlieCard h	student Chai	through corr	uns with disa	14/5-	0 & 1/1/	
455	-	Arrive m Vinnin of Square	6-164			5 11:29	19-350		0 2:54 0 4:04						5 11:45				rare	CharlieCard	Charlie Ticket	Cash-on-Board	Senior/TAP*** S0.85		LEE FARES: C nd Access C	Requires	available 1	and perso		12/25/2(	
4		Leave Salern Depot	A 6-050	_		10:10	10.010		3:50	_	_	_	_	a 11:05	11:35			-	-"		_	_		)	9 <u>.</u> 20		:		-	_	
		Arrive Salem	A 5:49A						11:07			P 1:23P		3:06		4:57				8:12	,	11:28					bilities				
	Outbound	Arrive 1 Vinnin 5quare	A 5:36A			8:10	9:11 9:44		10:45			P 1:01P				4:35				7:54		11:12	·		serve		ith dísa				
		r- Arrive Central Square	a 5:26A	2					10:31			P 12:47P				3:45		10:9		7:41			11:58		loes not		w suos			ř-	ation
Saturday	-	r- Wonder- land Station	×		_	8:05	8:35	_	10:07	-		7P 12:22P		2:05	-	3:56		_	_	7:20	-	4 10:42		No.	s route o	2	le to pe		Route 455	Salem Depot-	and St
Sat		Arrive Vonder- I land E Station	6-19 6-19				9:20			12:14		DP 1:27P	1:53				2 5:29				3 8:36	-	0 11:28		Weekend Note: This route does not serve	Shelland Office Park	All buses are accessible to persons with disabilities		Roll	Salen	Wonderland Station
	Inbound	Lv/Arrive Central Square		A 6:32					7 11:03		7P 12:24P	3P 1:00P	1:26				7 5:02			3 a7:46		-	8 11:10		ekend N	illand Ol	e are a				Ň
455	Ē	Arrve Arrve Arrinin Square		5A 6:16A			8:43		3 10:20 5 10:47		1 12:07P	6P 12:43P					7 4:47		5 6:31			÷			Ne	She	All buse				
4		Leave Salem Depot		6:05A	7:05	8:05	8:32	9:35	10:08	11:14	_	12:26P	12:55	2:15	2:47	3:55	4:31		6:15	69	7:47	9:47	10:45	-			_			E	
		Arnve Salem Depot	V 5:52A 6:34				9:18						C + - +		2:12		3:51		5:21		6:45					9:39		-	01:1	ips to/from	eliand
	Outbound	Arrive Minnin Square		6:53	24-2	8:36	8:56	9:36	9:56 10:26	10:56				1:21	1:47	2:56	3:26	4:26	4:56	5:58	6:22	7:18	7:34	8:24	8:49	9:19 10-16		12:07A		e: All trip	IIC avia
		- Amve Central Square	a 5:28A a 6:04	6:37	7:29	8:00 8:20	8:40 9:05	9:20	9:40 10:10	10:40	11:10	12:05P	10.05	1:05	1:30	2:39	3:09	4:09	4:39	5:41	6:07	7:05	7:21	8:11	8:36	9:06	11:07	11:58	12:496	Weekday Note: All tr	Salem Depot serve Shetland Office Park
Weekday		Leave Wonder- bind Station	ħ	6:15A	7:04	7:55	8:15	8:55	9:15	10:15	10:45	11:40	101-01	12:40	_	2:05	2:35	3:35	4:05	5:05	5:35	6:35	7:00	7:50	8:15	8:45	10:48	11:40	HRZ:ZL		
Wee		Arrive Ly/Arrive Wonder- Central land Square Slation	5.08A	5:43	6.02	6:50	7:23	8:02	8:42	9:07	9:24	10:27		11:54	12:17P	04.7	1:17	2:19	2:49	2	3:49	4:49	5:19	6:13	6:42	7:06	8:02	8:28	9:29 10:36		1 07
	Inbound	Lv/Arrive Central Square	5:00	5:20	5:37	6:22	6:51 7:05	7:30	7:55 8:10	8:35	8:52	9:55	10:28	11:22	11:45		12:46		2:18				4:48			5:38 7-08		8:05			12:05A 12:50
	tnb	Arrive Vicinin	Ø.	Ē		9C:C			7:39 7:54						11:29		P 12:29P	1:31	2:01		3:01		4:31					7:54			11:56 A 12:41A
455		Leave Salem Derbl	ħy		5 10A	5:55	6:20	6:55	7:35	8:00	8:17 8:47	9:20	9:53	10:47	11 10	D t	12:10F	1:05	1:35	C0.7	2:35	3:35	4:05	5:05	5:35	6:05	7:05	7:35	9:47	10:44	11:45 12:30A

	SERVICE SCHEDU	020/20 ERVICE	CE CE	SC SC	REDUCI	Sē	U U U	Ош	Ric	Ride Safer.		Wear Care a face covering	Maintain a healthy distance	Maintain a healthy distance	Practi good hygie	(Lan)	Reduced service schedule in effect when declared in advance by the MBTA. In mos announcement made late in the afternoor prior day. Stay connected to MBTA.com fo-the minute information.	schedule in nce by the M ade late in t nnected to l rrmation.	Reduced service schedule in effect when declared in advance by the MBTA. In most cases, announcement made late in the afternoon on the prior day. Stay connected to MBTA.com for up-to-the minute information.
NEV	NEWBURYPORT/ROCKPORT LINE	ROCKF	TAOC	LINE													「「「「「」」		
Mon	Monday to Friday Inbound to Boston				AM						M	5							
ZONE	STATION	TRAIN # 7150	0 1100	1150	1102	1152	1104	1154	1106	1156	1108	1158	1110 1	1160 1	1112	B: Due t	o construction activi	ties for the Glo	B: Due to construction activities for the Gloucester Drawbridge
8	Rockport	ا م–							B 1:50	-					B 9:50	עבהופרפווופ	Rockport, dus situities will replace train se Rockport, Gloucester, West Gloucester	cester, West Gli	Replacement project, due smuthes will replace non service detween Rockport, Gloucester, West Gloucester
	Gloucester	رم۔ ۱	B 6:57	1	B 9:57		B 11:57		B 1:57	i	B 5:07		B 7:27		B 9:57	(and	(and Manchester on outbound trips) on the Rockport Line,	ound trips) on	the Rockport Line.
	West Gloucester	ب م	7:13		10:13		12:13	00	2:13	•	5:23	1	7:43		10:13	ä	Bicycles cannot be taken on substitute bus service.	en on substitu	te bus service.
ם ע ס ע	Manchester Beverly Farms	ч ф-я	7-26		10:20		12:20	r 3	2:20	ř 4	5.36		7-56	; ;	10:26				
	Montserrat	ا م- د	7:32		10:32		12:32	,	2:32	¥	5:42		8:02		10:32		in purple with "f" ind ctor that they wish to le	i <b>cate a flag stop</b> eave. Passengers	Times in purple with "f" indicate a flag stop: Passengers must tell the conductor that they wish to leave. Passengers waiting to board must be
	Newburyport	δ:35						12:52	ı	2:52	1			9:00		visible	on the platform for the	train to stop.	
	Rowley		-	8:58	1	10:58	•	12:58	¢	2:58		5:58	,	9:06	12				
	lpswich		-	9:05	1	11:05	ľ	1:05	ì	3:05	•	6:05		9:13	3				
م	Hamilton/Wenham	6 5.53 5.53		9:12	•	ZL:LL	·	21:1	i, i	3:12		0:12		9.20	9) (				
	North Beverly Reveriv	6 0.00	2 7.38		10-28	11-20	17-38	_	2.38	3-20	5.48		α	-	10-38				
	Salem							_	2-42	3-24	2:22				10-42				
	Swampscott							-	2:49	3:30	5:59				10:49				
	Lvnn							_	2:54	3:34	6:04	6:34			10:54				
	River Works	f 6:22	-	-	-	-			f 2:57						f 10:57				
1A (	Chelsea	6:29						1:44	3:03	3:44	6:13				11:03				
1A 1	North Station	b 6:41	1 8:15	9:56	11:15	11:56	1:15	1:56	3:15	3:56	6:25	6:56	8:45 1	10:04 1	11:15				
Моп	Monday to Friday										3								
Dutbor	Outbound from Baston			AM		3					Md								
						_			1157	1109	7151				1113				
	North Station	\$ 8:30				_			4:30	5:30	6:00				11:30				
_	Chelsea	8:41			1 11:41									10:31 1	11:41				
	River Works	-	-	-	-			-	-		~	1/:33 1	~	-0	t 11:48				
	Lynn	\$ Q.D	10.6	10:41	LC:LL L	12:41	19:1	1.4.7	LC:4	10:0	17:0		8:51 10:0	10:41	10:11				
	Salom					-		0.514	4 10 10 10	40.0 10.0	0.44				10.01				
	Beverly					_			5:05	6:05	6:35				12:05				
	North Beverly					_			5:09	ı	6:39				Si				
2 L	Hamilton/Wenham	-0	10:13	د ع	12:13	e	2:13	ı	5:13	,	6:43	7:58	1	11:03	ı				
	lpswich	а 6,0	10:20	0	12:20		2:20	8	5:20	•	6:50	8:05	ŝ	11:10	ı				
	Rowley	۰ م-	10:27		12:27	PAR .	2:27	ŝ.	5:27	1	6:57	8:12		11:17	ı				
	Newburyport	р. ССС ССС ССС ССС ССС ССС ССС ССС ССС С	CC:01		CC:71	_	V		0:30		CO:/	Ω7:2		ŝ					
4 U	Montserrat Devery Earme	0.03 0.11	יי	11-04	, ,	PO-1	. )	80.7 80.7		0.03 6.14			9.03 0-14		12:03				
	Manchester		20	B 11:10	- 9	B 1:10		B 3:10		B 6:20	i ca	2 22	B 9:20		B 12:20				
	West Gloucester	m		B 11:30	30	B 1:30		B 3:30		B 6:40	e		B 9:40	-	B 12:40				
~	Gloucester	m	48	B 11:38		<b>B</b> 1:38		B 3:38		B 6:48	a		B 9:48		B 12:48				
	Rockport	8	9:48	B 11:		B 1:38	•	B 3:38		B 6:48			3 9:48	•	12:48				
Rec	Reduced Schedule advisory system The MBTA and Keolis closely monitor events to determine if changes to the Commuter Rail schedule are needed. During this time, the	adviso s to the Cor	ry syst	em The il schedule	MBTA and 3 are need:	Keolis clo ed: During	isely monit this time,	tor the	0		REGULAR SCHEDULE	SCHEDU	Ë	4	REDUCE	REDUCED SCHEDULE	ц. - -	8	NO SERVICE
symb	symbols to the right will communicate service level and impact on passengers.	unicate ser	rvice level	and impac	t on passe	ngers				= č	Irains viii operate on a normal schedule.	dule.			also availa.	nges to the reg able on MBTA <sub>s</sub> c	major changes to the regular schedule, schedules also available on MBTA,com and in Boston stations.		vo passenger service on Commuter Rail,
	Mass	sachuset	ts Bay		1245				iv V	sit			istomer S6	srvice	ſ	Download the	a	Follow	
	Transnortation Authority	enortatio	n Autho	ritu				_		MBTA.com	E		617-222-3200	3200			Commuter Rail App	@MBTA CR	A CR

Truck hour louve aver, wear outdouch trips) on the Rockport Line           Outdound trips) on the Rockport Line           R-32         124         465         126         468         128         470           R-32         8-33         9-3         3-3         5-3         3-3         5-3           R-32         8-33         8-33         8-10-10         6-3         5-3         3-3         5-3           R-32         8-33         8-33         9-10-10         3-5         6-3         3-3         5-3           R-32         8-33         8-30         8-33         9-10-10         3-5         5-3         5-3         5-3         5-3         5-3         5-3         5-3         5-3         5-3         7-10-3         5-3         7-10-3         5-3         7-10-3         5-3         7-10-3         5-3         7-10-3         5-3         7-10-3         5-3         7-10-3         5-3         7-10-3         5-3         7-10-3         5-3         7-10-3         5-3         7-10-3         5-3         7-10-3         5-3         7-10-3         7-3         7-3         7-3         7-3         7-3         7-3         7-3         7-3         7-3         7-3         7-3         7-3	165         127         167         129         169         131         171           200         84.5         9.30         10.15         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.6         54.7         50.2         51.7         52.6         54.6         54.6         54.7         50.6         54.6         54.7         50.6         56.7         54.7         52.6         55.8         56.7         50.6         51.7         51.7         52.6         55.8         56.7         50.6         51.7         51.7         52.6         55.8         56.7         50.6         51.7         51.7         51.7         52.6         55.8         56.6         51.7         51.7         52.6         55.8         56.6         51.6         51.6         51.6         52.6         55.8         52.6         55.8         56.6         51.6         56.6         52.6	Schedules may change in the event of severe weather During weather events, the symbol: below will communicate service level and impact on passengers. These modified schedules are available on MBTA.com.	SCHEDULE Trains will operate on a normal schedule. STORM SCHEDULE	Major changes to Relevant schedule. Schedules will be available on mbtacorr and in Boston stations NO SERVICE No passenger service
Iso anumus annuta nun anunta nunta	M         15         121         197         161         123         163         125           5.5         5.6         5.4         5.4         5.4         5.4         5.4           5.26         5.0         5.1         5.0         5.1         5.5         7.20           5.26         5.0         5.1         6.25         5.5         7.20           5.33         6.01         6.34         7.44         7.46         7.46           5.34         5.05         5.12         7.20         7.21         7.36           5.41         6.01         6.01         6.47         7.17         7.36           5.41         6.06         6.23         6.23         7.33         8.07           5.53         6.00         6.26         6.27         7.33         8.07           5.58         6.00         6.26         6.27         7.33         8.07           5.58         6.00         6.26         6.27         7.33         8.07           5.58         6.00         6.27         7.33         7.34         7.47           6.61         6.57         7.30         7.33         8.07         8.64 <t< th=""><th>PM         1157         1109         1159         1111         1161           2157         2109         1159         1111         1161           2157         2109         2159         2111         2161           230         530         530         530         730         00.20           4:36         5:31         7:36         8:30         102.01           4:51         5:51         7:36         8:51         10.41           4:54         5:51         7:36         8:54         10.41           4:54         5:54         17:26         9:01         10:51           5:05         6:05         7:56         9:05         10:55           5:13         2:46         9:01         10:55         10:55           5:13         2:46         9:01         10:55         10:55           5:13         2:46         9:01         10:55         10:55           5:13         2:82         8:82         10:10:55         11:10           5:21         8:82         8:82         11:10         11:10           5:22         8:82         9:01         10:10:55         11:10           5:23         8:8</th><th>3:04         -         15:14         -         12:14           3:10         -         16:20         -         9:20         -         8:12:02           3:30         -         8:640         -         9:40         -         8:12:02           3:33         -         8:640         -         8:940         -         8:12:02           3:33         -         8:648         -         8:948         -         8:12:48           3:33         -         8:648         -         8:948         -         8:12:48           3:33         -         8:648         -         8:948         -         8:12:48           3:33         -         8:648         -         8:948         -         8:12:48</th><th>he train to stop. If eace ahead of schedule at these stops with below the train number. om/accessibility for more information. Follow</th></t<>	PM         1157         1109         1159         1111         1161           2157         2109         1159         1111         1161           2157         2109         2159         2111         2161           230         530         530         530         730         00.20           4:36         5:31         7:36         8:30         102.01           4:51         5:51         7:36         8:51         10.41           4:54         5:51         7:36         8:54         10.41           4:54         5:54         17:26         9:01         10:51           5:05         6:05         7:56         9:05         10:55           5:13         2:46         9:01         10:55         10:55           5:13         2:46         9:01         10:55         10:55           5:13         2:46         9:01         10:55         10:55           5:13         2:82         8:82         10:10:55         11:10           5:21         8:82         8:82         11:10         11:10           5:22         8:82         9:01         10:10:55         11:10           5:23         8:8	3:04         -         15:14         -         12:14           3:10         -         16:20         -         9:20         -         8:12:02           3:30         -         8:640         -         9:40         -         8:12:02           3:33         -         8:640         -         8:940         -         8:12:02           3:33         -         8:648         -         8:948         -         8:12:48           3:33         -         8:648         -         8:948         -         8:12:48           3:33         -         8:648         -         8:948         -         8:12:48           3:33         -         8:648         -         8:948         -         8:12:48	he train to stop. If eace ahead of schedule at these stops with below the train number. om/accessibility for more information. Follow
Instancy (insertion for the section for	193         153         115         155         117         157         195         119           7.1         4.3         4.5         4.1         5.7         195         4.9           2.03         2.03         3.03         4.05         4.95         4.95         4.95         4.95           2.05         2.20         3.03         3.41         4.05         4.95         4.95         5.0           2.255         2.51         3.31         4.16         4.23         4.55         5.0           2.258         2.51         3.51         4.34         4.55         5.00         5.00           2.258         3.53         4.31         4.55         5.00         5.00         5.00           2.258         3.53         3.51         4.07         4.51         5.00         5.00           2.258         3.03         3.33         4.07         4.55         5.00         5.00           16.341         7         4.07         4.50         5.00         5.00           17.341         7         4.07         4.50         5.00         5.00           17.341         7         4.07         4.51         5.00         5.0	AM         AM           1101         1151         1103         1153         1105         1155           2101         2151         2103         2153         2105         2155           2300         2300         0320         1120         1120         1131           8330         9330         0320         1120         1120         1301           8551         9531         1041         11231         11441         1531         1541           8551         9541         1041         1151         12241         1541         1541           8551         9554         10431         11561         12241         1544         154           901         1001         10551         1201         12241         154         154           910         1001         10551         1201         12241         154         154           910         1001         12241         12341         154         154         154           910         1010         1220         1220         1220         205         1523         205           910         1020         1223         1223         1223         223 <td< th=""><th>werky Farms         6         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         18:04         18:05         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04          &lt;</th><th>Passengers waiting to brand must be visible on the platform for the train to stop. Times in blue indicate an early departure (L stop): The train may leave ahead of schedule at these stops Bikes: Bicycles are allowed on trains with the bicycle symbol shown below the train number. High level platform and bridge plate available. Visit mbta.com/accessibility for more information. tomer Service Follow</th></td<>	werky Farms         6         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         17:04         18:04         18:05         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04         18:04          <	Passengers waiting to brand must be visible on the platform for the train to stop. Times in blue indicate an early departure (L stop): The train may leave ahead of schedule at these stops Bikes: Bicycles are allowed on trains with the bicycle symbol shown below the train number. High level platform and bridge plate available. Visit mbta.com/accessibility for more information. tomer Service Follow
<b>2, 2020</b> <b>1</b> 10 <b>152 188 112 154</b> <b>3</b> 205 <b>3 3</b> 8 <b>112 154</b> <b>3</b> 205 <b>3 3</b> 8 <b>112 154</b> <b>3</b> 205 <b>3 3 3</b> 5 <b>4</b> 5 <b>4</b> 5 <b>3</b> 205 <b>1 3 3</b> 10540 <b>1</b> 10540 <b>1</b> 10540 <b>1</b> 1056 <b>1</b> 11112 <b>1</b> 11121 <b>1</b> 111121 <b>1</b> 11121 <b>1</b> 111121	(47)         (09)         (69)         (49)         (41)         (61)         (51)         (13)           1         4         5         4         4         5         4         5         4         5         4         5         4         5         4         5         4         5         4         4         5         135         105         135         105         135         105         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         135         1	Title         Title         Title         Saturday & Sunday           Title         Title         Title         Title         Saturday & Sunday           Zitle         Zitle         Zitle         Zitle         Saturday & Sunday           Zitle         Zitle         Zitle         Zitle         Staturday & Sunday           Zitle         Zitle         Zitle         Zitle         Staturday           Zitle         Zitle         Zitle         Staturday         Staturday           Zitle         Zitle         Zitle         Staturday         Staturday         Staturday           Zitle         Zitle         Zitle         Zitle         Staturday         St	9:32 10:42 5 80 9:32 10:42 5 M 9:42 10:54 7 W 10:54 11:15 8 R 10:04 11:15 8 R	] () 18 - <b>1</b> () ]
Am         Am           46         106         148           92         33         44           92         33         44           92         33         34           733         733         733           733         733         753           733         744         755           733         744         755           733         744         755           733         8:0         8:0           744         755         3:5           741         8:0         8:0           744         755         3:5           741         8:0         8:0           741         8:0         8:0           741         8:0         8:0           741         8:0         8:0           743         8:0         8:0           744         755         8:0           744         755         8:0           744         755         8:0           744         8:0         8:0           744         8:0         8:0           744         8:0         8:0           744 <td>AM         AM           183         143         105         145         107         117           6155         7.05         145         105         145         107         117           6155         7.05         7.45         5.10         5.33         900         9.45           6155         7.05         7.35         6.10         5.33         900         9.45           7735         8.05         8.20         8.20         8.20         9.20         9.55           721         723         8.06         8.24         8.23         9.23         9.56           721         723         8.07         8.24         8.23         9.00         9.24         0.00           721         723         8.03         8.20         8.24         8.20         9.00         9.24         0.00           721         723         8.20         8.20         9.00         9.24         0.00         9.24         0.00           721         723         8.00         9.00         9.24         9.00         9.24         0.00         9.24         0.00         9.24         0.00         9.24         0.00         9.24         0.00</td> <td>Ino.         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Holiday Service New Year's Day Thankspiring Day and Creasimas Day operate on a Suncay schedule. For all holiday schedules, visit MBTA com or call 617-222-3200. MBTA com or call 617-222-3200.</td>	AM         AM           183         143         105         145         107         117           6155         7.05         145         105         145         107         117           6155         7.05         7.45         5.10         5.33         900         9.45           6155         7.05         7.35         6.10         5.33         900         9.45           7735         8.05         8.20         8.20         8.20         9.20         9.55           721         723         8.06         8.24         8.23         9.23         9.56           721         723         8.07         8.24         8.23         9.00         9.24         0.00           721         723         8.03         8.20         8.24         8.20         9.00         9.24         0.00           721         723         8.20         8.20         9.00         9.24         0.00         9.24         0.00           721         723         8.00         9.00         9.24         9.00         9.24         0.00         9.24         0.00         9.24         0.00         9.24         0.00         9.24         0.00	Ino.         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VEHICLE TRAVEL SPEED DATA

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Accurate Counts 978-664-2565

Page 1

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Accurate Counts 978-664-2565

Page 2

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0       0       2       4       9       22       49       42       22       17       1       1         0       0       0       1       4       5       4       27       18       5       2       2       2       1       1         0       0       1       1       1       2       3       15       12       12       9       2       2       2       2       2       2       2       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1		0	0	5	20	22	28	46	20	67	16	თ	2		294
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0     0     1     1     2     3     15     21     12     9     2     1       0     0     1     1     0     10     1     1     0     0       236     300     466     703     764     1048     1328     1017     339     148     28     77       antie     23 MPH       Pace     565%       MPH     38.6%		0	0	0	1	4	2	4	27	41	18	5	2	2	109
0         1         1         1         0         10         12         6         1         0           236         300         466         703         764         1048         1326         1017         339         148         28         77           antile         23 MPH antile         23 MPH antile         23 MPH antile         33 MPH antile         33 MPH antile         33 MPH antile         23 MPH antile         28         77         23         748         28         77           antile         23 MPH antile         21 MPH antile         23 MPH antile         13 MPH antile         33 MPH antile         33 MPH antile         33 MPH antile         38 MPH antile         38.6%         77         33 MPH antile         38.6%         77         33 MPH antile         77         93 MPH antile         77         93 MPH antile         77         93 MPH antile         77         93 MPH antile         78         77         77         77         77           Pace         56.5%         MPH antile         38.6%         77         33 MPH antile         78         77         77         77         77<		0	0	0	0	-	2	0	15	21	12	0	2	1	66
236         300         465         703         764         1048         1328         1017         339         148         28         77           antile         14 MPH antile         23 MPH antile         28 MPH antile		0	0	0		•77	0	10	6	27	12	9		0	67
antile antile antile antile antile Pace Pace MPH MPH		0	236	300	466	703	764	1048	1328	1017	339	148	28	11	6454
entile entile rage) Pace Pace MPH MPH		151	h Percentile	14 MF	H										
antile antile rage) : Pace Pace MPH MPH		501	h Percentile	23 MF	Hc										
antile rege) Paced Pace MPH MPH		851	h Percentile	28 MF	Ë.									l	
z: peed 21-30 Pace 21-30 MPH MPH MPH		95t	h Percentile	31 MF	H										
Pace Pace MPH MPH		Mean Spe- 10 MPH I	ed(Average) :	23 MF 21-30 MF	H										
MPH MPH		NUT	ther in Pace	96.	48										
MPH :	- E	Peri Pher of Vehicles	sent in Pace	24	94										
	5	ent of Vehicles	s > 25 MPH :	38.6	%										

Page 3

# Accurate Counts 978-664-2565

$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	Location : East of Elm Place City/State: Swampscott, MA														8688SP01
1     1     2     1     1     1     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2     2 <th></th> <th>4 (</th> <th>7 0</th> <th><del>6</del></th> <th>13</th> <th><del>1</del>6</th> <th>19 21</th> <th>22</th> <th>25 27</th> <th>28 30</th> <th>31 33</th> <th>34 36</th> <th>37 30</th> <th>40 999</th> <th>Total</th>		4 (	7 0	<del>6</del>	13	<del>1</del> 6	19 21	22	25 27	28 30	31 33	34 36	37 30	40 999	Total
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15h Percentile       15 MPH         50h Percentile       15 MPH         50h Percentile       23 MPH         50h Percentile       23 MPH         60h Percentile       23 MPH         61h Percentile       21-30 MPH         0 Mrmber in Pace       60.0%         Number in Pace       53 MPH         60h Percentile       1540         21       50 MPH         60h Percentile       21 MPH		00	201	0 294	403	0	776	1130	1561	1086	355	138	47	- 69	1c 6729
Sith Percentile         15 MPH           Sith Percentile         23 MPH           NameSpecifyrersge)         23 MPH           Number of Venciles         53 MPH           Number of Venciles         53 MPH           Sith Percentile         23 MPH           Sith Percentile															
Mean Speed(Average)         23 WPH           10 MPH Pace Speed()         21-30 MPH           Number of Vehicles > 25 MPH         21-30 MPH           Number of Vehicles > 25 MPH         27-30 MPH           Number of Vehicles > 25 MPH         27-30 MPH           0         437         594         372         1540         2138         75           Fercent of Vehicles > 25 MPH         27-30 MPH         27-30 MPH         27-30 MPH         27-30 MPH           Number of Vehicles > 25 MPH         23         594         285         75           I 5th Percentile         23 MPH         23 MPH         23 MPH         23 MPH           Softh Percentile         23 MPH         23 MPH         23 MPH         261         75           Mean Speed(Average)         21-30 MPH         21-30 MPH         23 MPH         23 MPH         23 MPH           Number of Vehicles > 25 MPH         23.35         33.76         33.76         33.76         33.76           Number of Vehicles > 25 MPH         33.36         33.76         33.76         33.76         33.76           Number of Vehicles > 25 MPH         33.76         33.76         33.76         33.76         33.76         33.76         33.76         33.76         33.76		-15 501 951 951	h Percentile h Percentile h Percentile h Percentile h	15 MF 23 MF 28 MP 31 MP	F F F F										
10         MPI Pace Speed         21-30         MPI           Number in Pace:         4036         40.7%         40.7%         40.7%           Number of Vehicles > 25         MPH         2736         40.7%         2736           Percent of Vehicles > 25         MPH         2076         40.7%         2736           Percent of Vehicles > 25         MPH         2076         594         869         1372         1540         2178         269         75           Is th Percentie         15         MPH         16         17%         2889         2103         694         286         75           Is th Percentie         23         MPH         23         MPH         10         MPH Pace Speed (verage)         23         11         10         10         MPH Pace Speed (verage)         23         10         10         10         MPH Pace Speed (verage)         23         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         10         <		Mean Sper	ed(Average) ::	23 MP	H										
Mumber of Vehicles > 25 MPH         400           0         437         594         809         1372         1540         2178         75           Percent in Pace         60.0%         2736         809         1372         1540         2178         2889         2103         69.4         75           Ish Percentile         23 MPH         50 ht Percentile         23 MPH         23 MPH <td< td=""><td></td><td>10 MPH I</td><td>Pace Speed</td><td>21-30 MF</td><td>H ar</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></td<>		10 MPH I	Pace Speed	21-30 MF	H ar										
Number of Vehicles - 25 MPH;         2736           Percent of Vehicles - 25 MPH;         40.7%           0         437         594         869         1372         1540         286         75           15th Percentile;         23 MPH         564         869         1372         1540         286         75           15th Percentile;         23 MPH         561h Percentile;         23 MPH         28 MPH         28 MPH           961h Percentile;         23 MPH         11 MPH         23 MPH         11 MPH         12 MPH           Mean Speed(Average);         21-30 MPH         21-30 MPH         21-30 MPH         13 MPH           Number in Pace;         853%         530%         530%         530%         530%           Number of Vehicles - 25 MPH;         33.7%         530         541         541         541		Perc	toer in Pace	60.0 60.0	۵ <u>۵</u>										
0         437         594         869         1372         1540         2178         2869         2103         694         286         75           15th Percentile         23 MPH         50th Percentile         23 MPH         51 MPH         51 MPH         52 MPH         52 MPH         53 MPH         52 MPH         53 MPH	Per	nber of Vehicle: cent of Vehicle;	s > 25 MPH : s > 25 MPH ::	27: 40.7	36 %										
21-	_	٥	437	594	869	1372	1540	2178	2889	2103	694	286	75	146	13183
23 21-30		95 50 15 957 41 957 41		15 MF 23 MF 28 MF 31 MP	Ϋ́Ϋ́Ϋ́Τ										
		Mean Spe 10 MPH   Nurr Per	ed(Average) 1 Pace Speed 1 hber in Pace 1 cent in Pace 2	23 MF 21-30 MF 76( 58.3	H H SS % S										
	ы Б С	cent of Vehicle	s > 25 MPH	39.7	oc %										

Accurate Counts 978-664-2565

Page 4

Location : Essex Street	Street														
Location : East of Elm Place City/State: Swampscott, MA	f Elm Place pscott, MA														8688SP01
WB, EB	~	4	2	10	13	16	61	22	25	28	34	34	37	40	
Time	- m	- c	- σ.	5 6	<u>, ru</u>	<u>, «</u>	2.5	24	27	300	33	36.	39	666	Total
12/02/20	0	0	-	0	0	0	en en	6	16	25	σ	00	4	2	17
01:00	0	0	0	0	0	0	-	0	ŝ	7	7	4	4	2	32
02:00	0	0	-	0	0	0		-	4	9	e	n	0	0	19
03:00	0	0	0	0	0	0	0	4	9	7	4	3	-	0	25
04:00	0	0	-	2	-	-	2	4	6	16	16	11	-	С	67
05:00	0	0	4	ŝ	0	7	6	14	26	35	35	15	ŋ	2	161
06:00	0	0	-	6	10	32	19	71	148	120	44	10	ę	10	477
02:00	0	0	14	20	14	34	76	140	185	171	47	29	4	S	739
08:00	0	0	18	25	46	47	69	171	170	116	41	23	4	9	736
00:00	0	0	12	18	17	æ	28	123	189	154	83	21	ო	60	695
10:00	0	0	15	21	38	47	55	136	244	181	48	12	ę	<b>ത</b>	809
11:00	0	0	25	29	29	62	1	180	229	131	43	9	c	4	815
12 PM	0	0	6	15	25	55	95	161	278	171	47	26	ę	2	887
13:00	0	0	25	17	36	44	95	207	239	191	58	15	ო	9	936
14:00	0	0	39	29	69	94	201	230	187	67	12	ю	0	2	933
15:00	0	0	27	41	50	116	192	299	280	96	28	S	0	5	1139
16:00	0	0	39	5	06	159	212	294	195	68	7	4	0	4	1126
17:00	0	0	23	30	47	68	129	337	285	121	15	ŝ	+	2	1063
18:00	0	0	80	13	24	41	63	181	264	120	42	12	ო	ы	774
19:00	0	0	10	2 2	22	28	55	124	169	127	25	13	n		582
20:00	0	0	2	0	2	9	14	63	115	109	38	27	*	2	379
21:00	0	0	-	0		2	10	23	61	102	42	16	4	2	267
22:00	0	0	2	0	1	<u></u>	7	Q	31 31	44	25	18	G	4	145
23:00	0	0	0	0		10		15	21	53	23	15	2	-	133
Total	0	0	277	333	523	882	1444	2795	3356	2238	717	304	62	85	13016
Dailv		15	15th Percentile	17 M	Hd										
(		50	50th Percentile	24 M	Hd										
		10	85th Percentile	28 MPH	Hd										
		95	95th Percentile	31 M	HA										
		Mean Spt	Mean Speed(Average)	24 MPH	Hd										
			Number in Pace	ZI-30 MH	8870										
	Nh	Percent in Pace	Percent in Pace	68. 5	68.1% 5643										
	Pe	Percent of Vehicles > 25 MPH	12 > 25 MPH	43.4%	4%										

Page 5

Accurate Counts 978-664-2565

Location @ Essex Street	Street						978-664-2565	65							
Location : East of Elm Place City/State: Swampscott, MA	of Elm Place ipscott, MA														8688SP01
WB, EB															
Start	-	4	7	10	13	16	19	22	25	28	31	34	37	40	
Time	С	9	თ	12	15	18	21	24	27	30	33	36	39	666	Total
12/03/20	00	00	0 0	00	00	00	<b>м</b> т	ω (	40	15	ດເ	° 10	<i>с</i> о с	~ ~	62
00.10		0 0				- C	- 0	n ⊂	יז רי	ם ע	00	- 07	2	7 -	0 ¢
03:00	00	00	00	00		0 M	10	o uo	<b>)</b> က	9.41	1 +	00	0 0		25
04:00	0	0	c)	-	7	6	С	6	ষ	00	2	ę	2	0	53
05:00	0	0	1	ы	e)	10	7	31	33	41	28	19	5	4	185
06:00	0 0	0 0	S S	12	13	25	44	62	137	85	30	<u>9</u>	4 (	41	456
00:70	00	0 0	53	35	31 AF	41	85 0f	146	185	121	4	17	n,	<b>ب</b> ۵	736
00-60	o c	ə c	23	23	64 7.0	40	90 90 90	150 116	207	189	207 70	0 8	4	d r.	791
10:00	0	00	- თ	20	28	43	84	121	188	165	45	9 0	~ ~	) 4	719
11:00	0	0	22	34	43	52	88	177	193	112	33	Ø	(n)	2	791
12 PM	0	0	17	24	35	69	106	255	306	128	29	2	2	9	984
13:00	0 0	0 0	21	17	20	46	84	165	292	108 108	48	22	4	40	911
14:00	00	50	97 97	97 7	40 04 0	122	138	C472	202 202	1110	15	₽ €	- 0	9 5	1013
16-00	) C		18	24	31	52	170	324	996	187	96	2 =	10	~ 00	1183
17:00	0	0	20	26	32	92	166	301	272	146	26	- s	) <del></del>	ο Ω	1092
18:00	0	0	4	17	19	42	66	186	250	142	36	19	4	0	785
19:00	0	0	ო	7	15	31	40	111	186	127	44	16	4	2	586
20:00	0	0	4	9	ю	11	15	52	140	115	57	12	ы	0	418
21:00	0 0	0	21	0	2 0	4	1	32	4	<u>3</u> 3	8	21	ۍ ۱	2	282
22:00 23:00	0 0	00	- c	0 +	0 0	0 0	13	17	53 16	59 47	32	23 12	n n	2 0	206
Total		0	257	331	452	855	1500	2886	3605	2419	727	300	71	78	13481
Daily		15th F 50th F 85th F 95th F	15th Percentile 50th Percentile 85th Percentile 95th Percentile	18 MPH 24 MPH 28 MPH 31 MPH	IIII										
		Mean Speed(Average) 10 MPH Pace Speed Number in Pace Percent in Pace	Speed(Average) IPH Pace Speed Number in Pace	24 MPH 21-30 MPH 9410 69.8%	IIO%										
	Pen	Number of Vehicles > 25 MPH Percent of Vehicles > 25 MPH	25 MPH 25 MPH	5998 44.5%	88 %										
Grand Total	0	0	534	664	975	1737	2944	5681	6961	4657	1444	604	133	163	26497
Overall		15th F 50th F 85th F 95th F	15th Percentile 50th Percentile 85th Percentile 95th Percentile	18 MPH 24 MPH 28 MPH 31 MPH	IIII										
		Mean Speed(Average) 10 MPH Pace Speed Number in Pace Percent in Pace Pumber of Vehicles > 25 MPH	Speed(Average) IPH Pace Speed Number in Pace Percent in Pace	24 MPH 21-30 MPH 18280 69.0% 11642	110%0										
	her	cent of venicles >	HUM CZ .	43,9%	%										

Accurate Counts 978-664-2565

Page 6

MASSDOT CRASH RATE WORKSHEETS AND HIGH CRASH LOCATION MAPPING



INTERSECTION CRASH RATE WORKSHEET

CITY/TOWN :	Swampscott			COUNT DA	TE	Dec-20
DISTRICT :4	UNSIGN	ALIZED :		] SIGNA	LIZED	X
		~ IN	TERSECTION	N DATA ~		
MAJOR STREET :	Essex Stree					
MINOR STREET(S) :	Burpee Road	bb				
INTERSECTION DIAGRAM (Label Approaches)	North	635.6.4	tracts Drive	vy	stipet	
			PEAK HOUP	R VOLUMES		
APPROACH :	1	2	3	4	5	Total Peak Hourly
DIRECTION :	EB	WB	NB	SB		Approach Volume
PEAK HOURLY VOLUMES (PM) ::	653	802	0	97		1,552
"K" FACTOR:	0.090	INTERS	ECTION ADT	( <b>V</b> ) = TOTA I VOLUME :	AL DAILY	17,244
TOTAL # OF CRASHES :	7	# OF YEARS :	5	CRASHES	GE # OF PER YEAR( 、):	1.40
CRASH RATE CALCU	JLATION :	0.22	RATE =	<u>(A*1,</u> (V	000,000) * 365)	
Comments	Below Mass	DOT District	4 crash rate			
Project Title & Date:	Proposed Re	esidential Dev	velopment - Ja	anuary 2021		



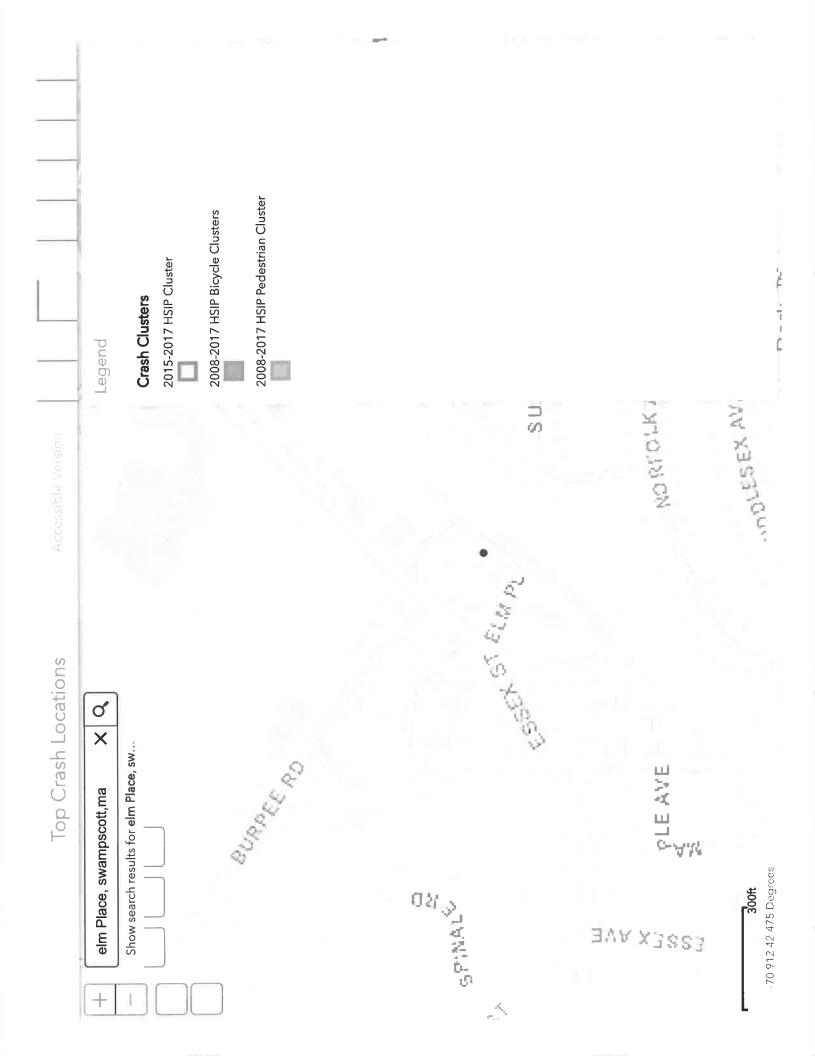
INTERSECTION CRASH RATE WORKSHEET

CITY/TOWN :	Swampscott	_		COUNT DA	.TE ;	Dec-20			
DISTRICT :4	UNSIGN	ALIZED :		SIGNA	SIGNALIZED :				
		~ IN	TERSECTION	I DATA ~	-dududuili-				
MAJOR STREET :	Essex Street								
MINOR STREET(S) :	Burrill Street								
INTERSECTION DIAGRAM (Label Approaches)	North	Sex	S-haet Stract	5	teet				
			PEAK HOUP						
APPROACH :	1	2	3	4	5	Total Peak Hourly			
DIRECTION :	EB	WB	NB	SB		Approach Volume			
PEAK HOURLY VOLUMES (PM) :	489	732	316	1		1,538			
"K" FACTOR :	0.090	] INTERS	ECTION ADT APPROACH		AL DAILY	17,089			
TOTAL # OF CRASHES :	3	# OF YEARS :	5	CRASHES	GE # OF PER YEAR ( \\):	0.60			
CRASH RATE CALCU	ILATION :	0.10	RATE =	<u>(A*1</u> )	000,000) * 365)				
Comments : Project Title & Date:	Below Massl		– 4 crash rate velopment - Ja	2024					

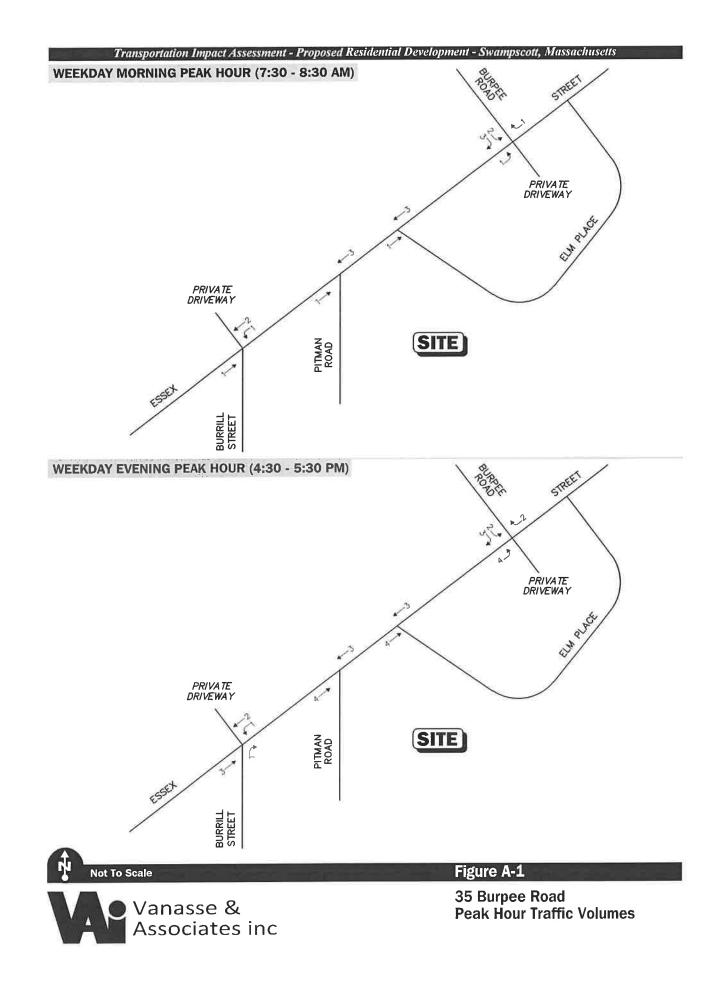


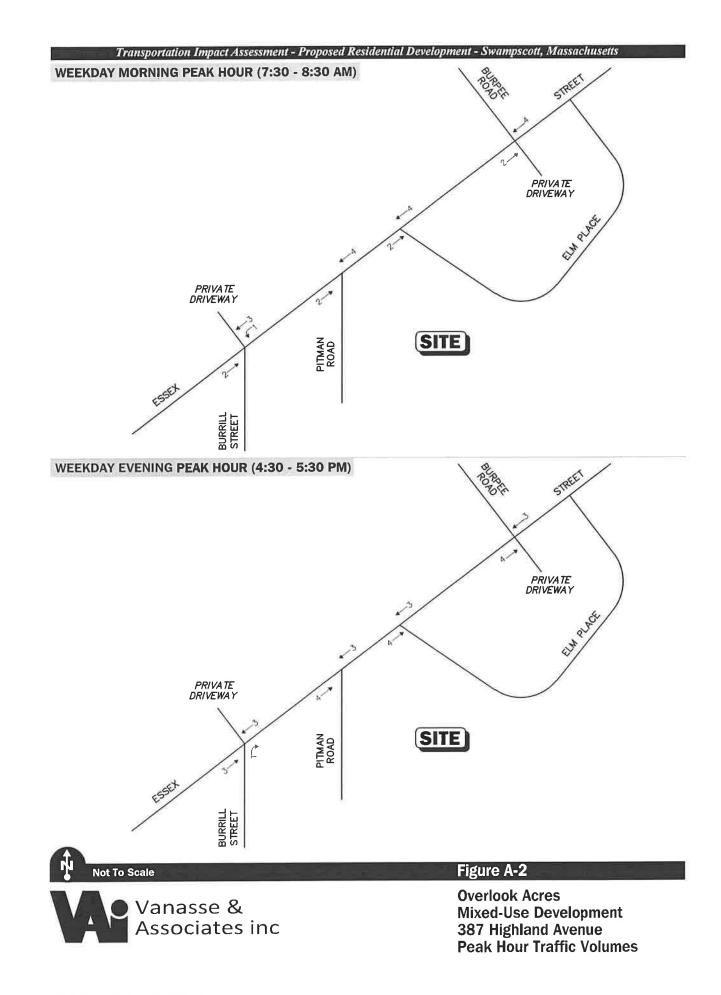
**INTERSECTION CRASH RATE WORKSHEET** 

CITY/TOWN :	Swampscott			COUNT DA	ſE:	Dec-20	
DISTRICT :4	UNSIGN						
		~ IN <sup>-</sup>	TERSECTION	I DATA ~	0.000000	(000000))	
MAJOR STREET :	Essex Street						
MINOR STREET(S) :	Elm Place						
INTERSECTION	↑ North			STVE	et		
DIAGRAM (Label Approaches)	-	ESSEX	Phice				
			PEAK HOUR	VOLUMES			
APPROACH	1	2	3	4	5	Total Peak Hourly	
DIRECTION :	EB	EB WB NB SB					
PEAK HOURLY VOLUMES (PM)	661	741	25			1,427	
"K" FACTOR	0.090	INTERS	ECTION ADT APPROACH		AL DAILY	15,856	
TOTAL # OF CRASHES :	2	# OF YEARS :	5	CRASHES	GE # OF PER YEAR( ):	0.40	
CRASH RATE CALCU	ILATION :	0.07	RATE =	( A * 1,0 ( V	000,000 ) * 365 )		
Comments	Below Mass	DOT District 4	l crash rate				
Project Title & Date:	Proposed Re	sidential Dev	elopment - Ja	nuary 2021			



BACKGROUND DEVELOPMENT TRAFFIC-VOLUME NETWORKS





GENERAL BACKGROUND TRAFFIC GROWTH

# General Background Traffic Growth

Average Annual Growth Rate	1.11%	1.25%
2019	62,467	
2018	60,868	
2017	58,745	
2016	59,364	
14 2015	58,092	
2014	60,089	
2013	53,534	
2012	56,677	
2011	56,942	
2010	55,853	
LOCATION	Boston/Revere Line	
ROUTE/STREET	Route 1A	
CITY/TOWN	Revere	
STA.	8087	

TRIP-GENERATION CALCULATIONS

		the time former diati
👬 Graph Look Up		
Query Filter	Data Plot and Equation	DATA STATISTICS
		Land Uee: Multifamily Housing (Mid-Rise) (221) <u>Click for more</u>
DATA SOURCE: Trip Gen Manual, 10th Ed + Supplement	000°£	<u>details</u> Independent Variable: Ducetter Linha
SEARCH BY LAND USE CODE:	2,500	Time Period: Weekday
LAND URE GROUP: (200-299) Residential	2,000 × ×	Setting/Location: General Urban/Suburban Trip Type:
LAND USE : 221 - Multifamily Housing (Mid-Rise)	X 1,500	Vehicle Number of Studies: 27
LAND USE SUBCATEGORY: All Stres	qнт = т × × ×	Avg. Num. of Dwelling Units: 205
	1,000 × × ×	Average Rate: 5.44
	200 ××	Range of Ratee: 1.27 - 12.50
TIME PERIOD: Workdow	××	Standard Devlation: 2.03
NOLLY	100	
General Urban/Suburban	X = Number of Dwelling Units	R <sup>2</sup> : 0.77
TRIP TYPE: Vohiolo	Reset Zoom Restore	Directional Distribution: 50% entering, 50% extiting
ALUE TO CALCULATE TRIPS: Calculate	X Study Site Fitted Curve Average Rate	Calcularied Trip Ende: Average Rate: 696 (Total), 348 (Entry), 348 (Exit) Fitted Curve: 696 (Total), 348 (Entry), 348 (Exit)
	Use the mouse wheel to Zoom Out or Zoom In.	

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# C Help Jeffrey Dirk Sign out

Multifamily Housing (Mid-Rise) (221) Click for more

Land Use:

Independent Variable:

details

**Dwelling Units** Time Period: Peak Hour of Adjacent Street Traffic One Hour Between 7 and 9 a.m.

Weekday

Setting/Location: General Urban/Suburban

Irlp Type:

Vehicle

DATA STATISTICS

# Graph Look Up

>

Trip Gen Manual, 10th Ed + Supplement

DATA SOURCE:

SEARCH BY LAND USE CODE:

Ø

221

LAND USE GROUP:

(200-299) Residential LAND USE : 221 - Multifemily Housing (Mid-Rise)

>

>

LAND USE SUBCATEGORY: All Sites

>

NDEPENDENT VARIABLE (IV):

**Dwelling Units** TIME PERIOD:

>

Weekday, Peak Hour of Adjacent Street Traffic 🗸

General Urban/Suburban SETTING/LOCATION:

>

TRIP TYPE: Vehicle

ENTER IN VALUE TO CALCULATE TRIPS: Calculate 128

Average Rate: 46 (Total), 12 (Entry), 34 (Exit) Fitted Curve: 44 (Total), 11 (Entry), 33 (Exit)

26% entering, 74% exiting **Directional Distribution:** 

**Calculated Trip Ends:** 

96:0 - (۵)سا 86:0 = (۲)س Fitted Curve Equation:

0.67

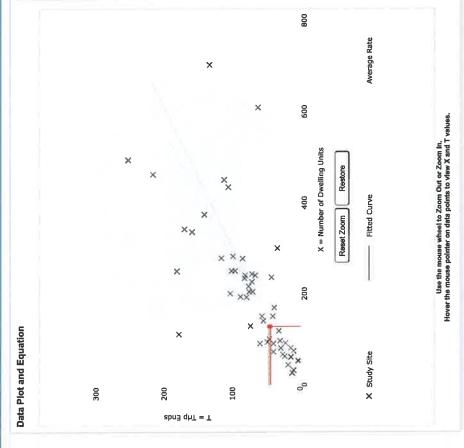
۲ż

Standard Deviation: Range of Rates: Average Rate:

0.19

0.06 - 1.61

0.36



Avg. Num. of Dwelling Units:

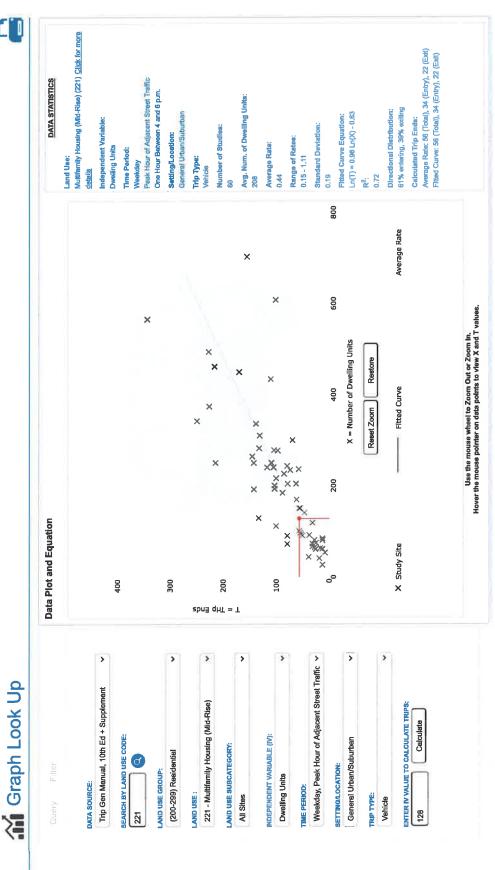
207

Number of Studles:

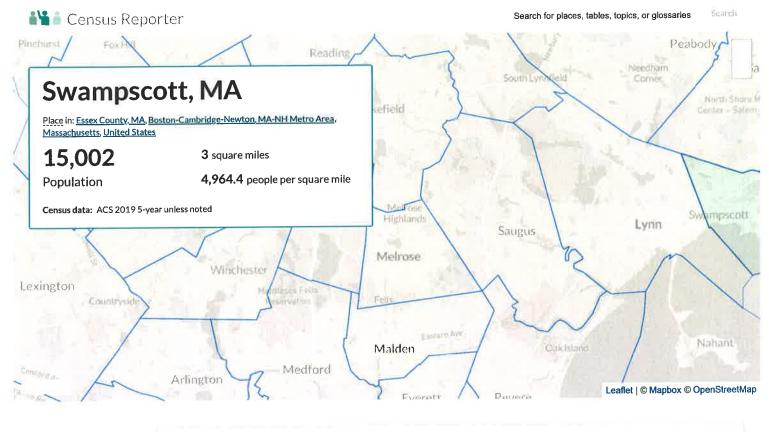
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App	
Web-based	
ITETripGen	

C Help 💹 Jeffrey Dirk 🖪 Sign out



MODE OF TRANSPORTATION FOR THE TOWN OF SWAMPSCOTT



Find data for this place

Search by table or column name ...

Interact with charts and statistics for margins of error and additional information.

#### Demographics

statistic.

<sup>†</sup> Margin of error is at least 10 percent of the total value. Take care with this

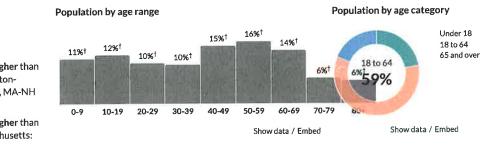


Age

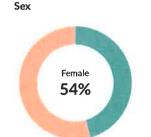
Median age

about 20 percent higher than the figure in the Boston-Cambridge-Newton, MA-NH Metro Area: 38.7

about 10 percent higher than the figure in Massachusetts: 39.5



<sup>†</sup> Margin of error is at least 10 percent of the total value. Take care with this statistic.

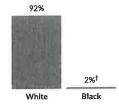


#### **Race & Ethnicity**

Male

Show data / Embed

Female



\* Hispanic includes respondents of any race. Other categories are non-Hispanic.

0%

Native

2%†

Aslan

0%

Islander

Other Show data / Embed

0%

2%†

Two+

**Economics** 

Income

<sup>†</sup> Margin of error is at least 10 percent of the total value. Take care with this statistic.

# \$56,405

Per capita income

about 20 percent higher than the amount in the Boston-Cambridge-Newton, MA-NH Metro Area: \$47,604

about 1.3 times the amount in Massachusetts: \$43,761

Swampscott, MA - Profile data - Census Reporter

## \$113,407

Median household income

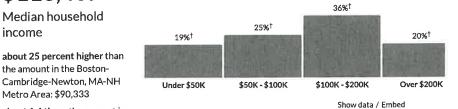
the amount in the Boston-

Metro Area: \$90,333

Cambridge-Newton, MA-NH

about 1.4 times the amount in Massachusetts: \$81,215

Household income



Poverty

<sup>†</sup> Margin of error is at least 10 percent of the total value. Take care with this statistic.

<sup>†</sup> Margin of error is at least 10 percent of the total

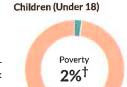
value. Take care with this

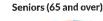
# 3.1%

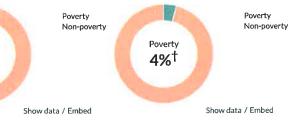
about one-third of the rate in the Boston-Cambridge-Newton, MA-NH Metro Area: 9.4%

Persons below poverty line

about one-third of the rate in Massachusetts: 10.3%







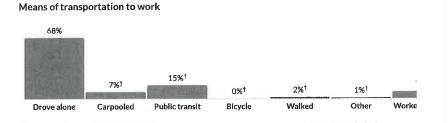
Transportation to work

### 35.4 minutes

Mean travel time to work

about 10 percent higher than the figure in the Boston-Cambridge-Newton, MA-NH Metro Area: 31.8

about 20 percent higher than the figure in Massachusetts: 30.2



\* Universe: Workers 16 years and over

Show data / Embed

Families

statistic.

Households

# 5,780

Number of households

the Boston-Cambridge-Newton, MA-NH Metro Area: 1,831,217 Massachusetts: 2,617,497

2.6

#### Persons per household

about the same as the figure in the Boston-Cambridge-Newton, MA-NH Metro Area: 2.6

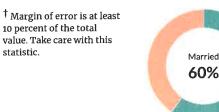
about the same as the figure in Massachusetts: 2.5

#### Population by household type



Show data / Embed

#### **Marital status**



\* Universe: Population 15 years and over

https://censusreporter.org/profiles/16000US2568680-swampscott-ma/

Marital status, by sex

Married

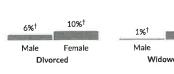
Single



Male Female Now married

55%

66%



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	Burpee Road	(North)													0	%0
		(South) (N	652	661					97				36		1,446	25%
	Essex Street Essex Street Burrill Street	(West)	978	132	869	140		142	146			141	55	87	2,690	46%
	Essex Street	(East)		529		421	310	142		174	154				1,730	29%
		Number	1,630	1,322	869	561	310	284	243	174	154	141	91	87	5,866	
		MCD	Boston city	Swampscott town	Lynn city	Salem city	Beverly city	Peabody city	Cambridge city	Marblehead town	Danvers town	Burlington town	Chelsea city	Woburn city		
Workplace		County	Suffolk County	Essex County	achusetts Essex County	Essex County	Essex County	Essex County	Middlesex County	Essex County	Essex County	Middlesex County	Suffolk County	Middlesex County		
	State/U.S. Island Area/Foreign	, ,	Massachusetts	Massachusetts	Massachusetts	Massachusetts	Massachusetts	Massachusetts	Massachusetts	Massachusetts	Massachusetts	Massachusetts	Massachusetts	Massachusetts		
		MCD	Swampscott town Massachusetts Suffolk County	Swampscott town Massachusetts Essex County	Swampscott town Mass	Swampscott town Massachusetts Essex County	Swampscott town Massachusetts Essex County	Swampscott town Massachusetts Essex County	Swampscott town Massachusetts Middlesex County	Swampscott town Massachusetts Essex County	Massachusetts Essex County Swampscott town Massachusetts Essex County	Massachusetts Essex County Swampscott town Massachusetts Middlesex County	Swampscott town Massachusetts Suffolk County	Massachusetts Essex County Swampscott town Massachusetts Middlesex County Woburn city		
Residence		County	Massachusetts Essex County	Massachusetts Essex County	Aassachusetts Essex County	Aassachusetts Essex County	Massachusetts Essex County	Massachusetts Essex County	Massachusetts Essex County	Massachusetts Essex County	Essex County	Essex County	Massachusetts Essex County	Essex County		
		State	Massachusetts	Massachusetts	Massachusetts	Massachusetts	Massachusetts	Massachusetts	Massachusetts	Massachusetts	Massachusetts	Massachusetts	Massachusetts	Massachusetts		

SAY

25%

45%

30%

S:\Jobs\8688\Journey\_to\_Work\_Swampscott.xlsx

#### CAPACITY ANALYSIS WORKSHEETS

Essex Street at Burrill Street Essex Street at Burpee Road Essex Street at Pitman Road Essex Street at Elm Place (South) Elm Place at the Project Site Driveway Essex Street at Burrill Street

### 2020 Existing Wkdy AM 1: Burrill Street & Essex Street

Lane Group         EBL         EBR         WBL         WBT         WBR         NBT         NBT         NBR         SBL         SBT         SBR           Lane Configurations         4         7         7         7         4         4         4         4           Tradit Volume (vph)         0         395         54         168         281         0         41         3         152         0         1         0           Lane Middlin (th)         12         16         12         11         12         12         14         122         10         120         100         1000         1000         1000         1000         1000         100         100         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         <		٦	-	$\mathbf{r}$	*	-	*	*	†	1	1	Ļ	4
Lane Configurations $\Phi$ $\Phi$ $\Phi$ $\Phi$ Trafic Volume (vph)         0         395         54         168         281         0         41         3         152         0         1         0           Ichure Volume (vph)         0         395         54         168         281         0         41         3         152         0         1         0           Ichure Volume (vph)         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1800         180         180         100         100         1500         190         100         100         100         100         100         100         100         100         100         100         100         100         110         110         110         110         110         110         110         110         110         110 <td>Lane Group</td> <td>EBL</td> <td>EBT</td> <td>EBR</td> <td>WBL</td> <td>WBT</td> <td>WBR</td> <td>NBL</td> <td>NBT</td> <td>NBR</td> <td>SBL</td> <td>SBT</td> <td>SBR</td>	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)         0         395         54         168         281         0         41         3         152         0         1         0           Future Volume (vph)         0         395         54         168         281         0         41         3         152         0         1         0           Lane Width (ft)         12         16         12         11         12         12         14         12         10         12           Storage Length (ft)         0         0         175         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0 </td <td></td> <td></td> <td>4</td> <td></td> <td>ሻ</td> <td>t,</td> <td></td> <td></td> <td>4</td> <td></td> <td></td> <td>4</td> <td></td>			4		ሻ	t,			4			4	
Future Volume (vph)         0         335         54         168         281         0         41         3         152         0         1         0           ideal Flow (vphp)         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         180         180         180         180		0		54	168	281	0	41	3	152	0	1	0
ideal Flow (uphp)         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         1900         100         120         120         120         120         120         120         120         120					168	281	0	41	3	152	0	1	0
Lane Width (ft)         12         16         12         11         12         12         14         12         12         10         12           Storage Length (ft)         0         0         1         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>1900</td> <td>1900</td> <td>1900</td> <td>1900</td> <td>1900</td> <td>1900</td> <td>1900</td>							1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)         0         0         175         0         0         0         0         0           Storage Lanes         0         0         1         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0									14	12	12	10	12
Storage Lange         0         0         1         0         0         0         0         0         0           Taper Length (ft)         25         25         25         25         25         25         10           Right Turn on Red         Yes         Yes         Yes         Yes         Yes         Yes         Yes         Yes           Link Distance (ft)         475         300         339         94         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30         30	1 2		12255					0		0	0		0
Taper Length (ft)         25         25         25         25         25         25           Right Turn on Red         Yes         Yes         Yes         Yes         Yes         Yes           Right Turn on Red         00         30         30         30         30         30           Link Speed (mph)         0.0         0.90         0.90         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.85         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.20         0.5         0.5										0	0		0
Right Turn on Red         Yes				tere i	25						25		
Link Speed (mph)         30         30         30         30         30         30           Link Distance (ft)         475         300         389         94           Travel Time (s)         10.8         6.8         8.8         2.1           Peak Hour Factor         0.90         0.90         0.90         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.83         0		20		Yes			Yes			Yes			Yes
Link Distance (t)         475         300         389         94           Travel Time (s)         10.8         6.8         8.8         2.1         2           Peak Hour Factor         0.90         0.90         0.90         0.84         0.84         0.84         0.84         0.84         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.25         0.8         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.85         0.25         0.25         0.25         0.25         0.50         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0 <td< td=""><td>•</td><td></td><td>30</td><td>100</td><td></td><td>- 30</td><td></td><td></td><td>30</td><td></td><td></td><td>30</td><td></td></td<>	•		30	100		- 30			30			30	
Link Data (y)         10.8         6.8         8.8         2.1           Peak Hour Factor         0.90         0.80         0.90         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84 </td <td></td>													
Peak Hour Factor         0.90         0.90         0.90         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.84         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85         0.85		N. 148											
Heavy Vehicles (%)         2%         4%         0%         1%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%         0%		0.00		0.00	0.84		0.84	0.84		0.84	0.25		0.25
Shared Lane Traffic (%)         In         In         In           Lane Group Flow (vph)         0         499         0         200         335         0         0         234         0         0         4         0           Tum Type         NA         pm+pt         NA         Split         NA         NA         Premitted Phases         4         3         8         2         2         6           Permitted Phases         4         4         3         8         2         2         6         6           Switch Phase													
Lane Group Flow (vph)         0         499         0         200         335         0         0         234         0         0         4         0           Turn Type         NA         primet         NA         split         NA         NA         NA           Protected Phases         4         3         8         2         2         6           Permitted Phases         4         3         8         2         2         6         6           Switch Phase         4         4         3         8         2         2         6         6           Switch Phase         4         4         3         8         2         2         6         6           Minimum Initial (s)         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0		270	4%	0%	1 /0	4 /0	070	070	070	170	0 /0	070	070
Date of outper form (rpn)         NA         pm+pt         NA         Split         NA         NA           Protected Phases         4         3         8         2         2         6           Permitted Phases         4         3         8         2         2         6           Detector Phase         4         4         3         8         2         2         6           Switch Phase         4         4         3         8         2         2         6         6           Switch Phase         4         4         3         8         2         2         6         6           Switch Phase         4         4         3         8         2         2         6         6           Switch Phase         4         4         3         8         2         2         6         6           Minimum Initial (s)         30.0         30.0         15.0         54.0         19.0         12.0         12.0           Total Split (s)         37.1%         37.1%         14.3%         51.4%         18.1%         18.1%         11.4%         11.4%           Vellow Time (s)         1.0         1.0		0	100	0	200	225	0	0	224	0	0		0
Protected Phases         4         3         8         2         2         6           Permitted Phases         4         8         6         6           Detector Phase         4         3         8         2         2         6         6           Switch Phase         5         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50         50		0		0			U			0	0		0
Permitted Phases         4         8         6           Detector Phase         4         4         3         8         2         2         6         6           Switch Phase           50         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0 <td></td> <td>PL1, 202, 125</td> <td></td>												PL1, 202, 125	
Detector Phase         4         4         3         8         2         2         6         6           Switch Phase         Minimum Initial (s)         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0			4			8		2	2		0	0	
Switch Phase         Minimum Initial (s)         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0								0	0			0	
Minimum Initial (s)         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0         5.0		4	4		3	8		2	2		6	6	
Minimum Split (s)         22.5         22.5         10.0         22.5         19.0         19.0         12.0         12.0           Total Split (s)         39.0         39.0         15.0         54.0         19.0         19.0         12.0         12.0           Total Split (%)         37.1%         37.1%         14.3%         51.4%         18.1%         18.1%         11.4%         11.4%           Yellow Time (s)         4.0         4.0         4.0         3.0         3.0         3.0         3.0           All-Red Time (s)         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0<											1.000		
Total Split (s)         39.0         39.0         15.0         54.0         19.0         19.0         12.0         12.0           Total Split (%)         37.1%         37.1%         14.3%         51.4%         18.1%         18.1%         11.4%         11.4%           Yellow Time (s)         4.0         4.0         4.0         3.0         3.0         3.0         3.0           All-Red Time (s)         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0           Lost Time Adjust (s)         -1.0         -1.0         -1.0         0.0         0.0           Total Lost Time (s)         4.0         4.0         4.0         4.0         4.0           Lead/Lag         Lag         Lag         Lag         Lead         Lead         4.0         4.0           Lead/Lag         Lag         Lag </td <td></td>													
Solution (N)         37.1%         37.1%         14.3%         51.4%         18.1%         11.4%         11.4%           Yellow Time (s)         4.0         4.0         4.0         3.0         3.0         3.0         3.0           All-Red Time (s)         1.0         1.0         1.0         1.0         1.0         1.0         1.0         1.0           Lost Time Adjust (s)         -1.0         -1.0         -1.0         0.0         0.0         0.0           Total Lost Time (s)         4.0         4.0         4.0         4.0         4.0         4.0           Lead/Lag         Lag         Lag         Lead         Lag         Lead         4.0         4.0         4.0           Lead/Lag Optimize?         Yes         <	Minimum Split (s)												
Yellow Time (s)       4.0       4.0       4.0       3.0       3.0       3.0       3.0         All-Red Time (s)       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0	Total Split (s)	39.0										a construction of the second se	
All-Red Time (s)       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0       1.0 <td>Total Split (%)</td> <td>37.1%</td> <td>37.1%</td> <td></td> <td>14.3%</td> <td>51.4%</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	Total Split (%)	37.1%	37.1%		14.3%	51.4%							
Lost Time Adjust (s)         -1.0         -1.0         -1.0         -1.0         0.0         0.0           Total Lost Time (s)         4.0         4.0         4.0         4.0         4.0         4.0           Lead/Lag         Lag         Lag         Lead         Lead         4.0         4.0         4.0           Lead-Lag Optimize?         Yes         Yes         Yes         Yes         Recall Mode         None	Yellow Time (s)	4.0	4.0		4.0	4.0							
Total Lost Time (s)         4.0         4.0         4.0         4.0           Lead/Lag         Lag         Lag         Lead         Lead           Lead/Lag         Ves         Yes         Yes         Yes           Recall Mode         None	All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0		
Total Lost Time (s)         4.0         4.0         4.0         4.0         4.0           Lead/Lag         Lag         Lag         Lead         Lead <thlead< th="">         Lead         <thlead< th=""></thlead<></thlead<>	Lost Time Adjust (s)		-1.0		-1.0	-1.0			0.0			0.0	
Lead/Lag         Lag         Lag         Lead           Lead-Lag Optimize?         Yes         Yes         Yes           Recall Mode         None         None         None         None         None         None         None           V/c Ratio         0.67         0.46         0.30         0.57         0.02           Control Delay         23.0         12.3         9.2         19.1         36.0           Queue Delay         0.0         0.0         0.0         0.0         0.0           Total Delay         23.0         12.3         9.2         19.1         36.0           Queue Delay         0.0         0.0         0.0         0.0         0.0           Queue Length 50th (ft)         109         17         32         25         1           Queue Length 95th (ft)         418         117         195         125         4           Internal Link Dist (ft)         395         220         309         14         14           Turn Bay Length (ft)         175         1594         619         277         277           Starvation Cap Reductn         0         0         0         0         0         0         0			4.0		4.0	4.0			4.0			4.0	
Lead-Lag Optimize?         Yes         Yes         Yes           Recall Mode         None         None </td <td></td> <td>Lag</td> <td>Lag</td> <td></td> <td>Lead</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>		Lag	Lag		Lead								
Recall Mode         None					Yes								
v/c Ratio         0.67         0.46         0.30         0.57         0.02           Control Delay         23.0         12.3         9.2         19.1         36.0           Queue Delay         0.0         0.0         0.0         0.0         0.0           Total Delay         23.0         12.3         9.2         19.1         36.0           Queue Length Soth (ft)         109         17         32         25         1           Queue Length Soth (ft)         418         117         195         125         4           Internal Link Dist (ft)         395         220         309         14           Turn Bay Length (ft)         1402         497         1594         619         277           Base Capacity (vph)         1402         497         1594         619         277           Starvation Cap Reductn         0         0         0         0         0         0           Storage Cap Reductn         0         0         0         0         0         0         0           Intersection Summary         Zitesection Summary         Zitesection Summary         Zitesection Summary         Zitesection Summary         Zitesection Summary         Zitesection Summary <td></td> <td></td> <td>None</td> <td></td> <td>None</td> <td>None</td> <td></td> <td>None</td> <td>None</td> <td></td> <td>None</td> <td>None</td> <td></td>			None		None	None		None	None		None	None	
Control Delay         23.0         12.3         9.2         19.1         36.0           Queue Delay         0.0         0.0         0.0         0.0         0.0           Total Delay         23.0         12.3         9.2         19.1         36.0           Queue Length 50th (ft)         109         17         32         25         1           Queue Length 95th (ft)         418         117         195         125         4           Internal Link Dist (ft)         395         220         309         14           Turn Bay Length (ft)         1402         497         1594         619         277           Starvation Cap Reductn         0         0         0         0         0         0           Storage Cap Reductn         0         0         0         0         0         0         0           Reduced v/c Ratio         0.36         0.40         0.21         0.38         0.01         0						0.30			0.57			0.02	
Queue Delay         0.0         0.0         0.0         0.0         0.0           Total Delay         23.0         12.3         9.2         19.1         36.0           Queue Length 50th (ft)         109         17         32         25         1           Queue Length 95th (ft)         418         117         195         125         4           Internal Link Dist (ft)         395         220         309         14           Turn Bay Length (ft)         175         1594         619         277           Base Capacity (vph)         1402         497         1594         619         277           Starvation Cap Reductn         0         0         0         0         0         0           Spillback Cap Reductn         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0					12.3	9.2			19.1			36.0	
Total Delay         23.0         12.3         9.2         19.1         36.0           Queue Length 50th (ft)         109         17         32         25         1           Queue Length 95th (ft)         418         117         195         125         4           Internal Link Dist (ft)         395         220         309         14           Turn Bay Length (ft)         175         1594         619         277           Base Capacity (vph)         1402         497         1594         619         277           Starvation Cap Reductn         0         0         0         0         0         0           Spillback Cap Reductn         0         0         0         0         0         0           Reduced v/c Ratio         0.36         0.40         0.21         0.38         0.01													
Queue Length 50th (ft)         109         17         32         25         1           Queue Length 95th (ft)         418         117         195         125         4           Queue Length 95th (ft)         418         117         195         125         4           Internal Link Dist (ft)         395         220         309         14           Turn Bay Length (ft)         175         7         7         7           Base Capacity (vph)         1402         497         1594         619         277           Starvation Cap Reductn         0         0         0         0         0         0           Spillback Cap Reductn         0         0         0         0         0         0         0           Storage Cap Reductn         0         0.36         0.40         0.21         0.38         0.01           Intersection Summary         Other         Other         Other         Other         Other         Other													
Queue Length 95th (ft)         418         117         195         125         4           Internal Link Dist (ft)         395         220         309         14           Turn Bay Length (ft)         175         175         175         175           Base Capacity (vph)         1402         497         1594         619         277           Starvation Cap Reductn         0         0         0         0         0         0           Spillback Cap Reductn         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0 <td></td> <td>1</td> <td></td>												1	
Internal Link Dist (ft)         395         220         309         14           Turn Bay Length (ft)         175												4	
Turn Bay Length (ft)       175         Base Capacity (vph)       1402       497       1594       619       277         Starvation Cap Reductn       0       0       0       0       0         Spillback Cap Reductn       0       0       0       0       0         Storage Cap Reductn       0       0       0       0       0         Reduced v/c Ratio       0.36       0.40       0.21       0.38       0.01         Intersection Summary         Area Type:       Other					117								
Base Capacity (vph)         1402         497         1594         619         277           Starvation Cap Reductn         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0			390		175	220			000			17	
Starvation Cap Reductn         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0			1400			1504			610			277	
Spillback Cap Reductin         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0													
Storage Cap Reductn         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0													
Reduced v/c Ratio     0.36     0.40     0.21     0.38     0.01       Intersection Summary     Area Type:     Other						-						-	
Intersection Summary Area Type: Other					-	-							
Area Type: Other			0.36		0.40	0.21			0.38	in the second second		0.01	
		Other			Ten an		192121				3		1 martin
		Other											

Lane Group	Ø9	1월 11 - Selen 2014년 19 - Selen 2017년 11 - 201 11 - 2017년 11
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Lane Width (ft)		
Storage Length (ft)		
Storage Lanes		
Taper Length (ft)		
Right Turn on Red		
Link Speed (mph)		
Link Distance (ft)		
Travel Time (s)	a state and a	
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	5.0	
Minimum Split (s)	22.5	
Total Split (s)	20.0	
Total Split (%)	19%	
Yellow Time (s)	3.5	
All-Red Time (s)	1.0	
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag		
Lead-Lag Optimize?	New	
Recall Mode	None	
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
Queue Length 50th (ft) Queue Length 95th (ft)		
Internal Link Dist (ft)		
Turn Bay Length (ft)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
	The Laboratory of the	
Intersection Summary	and Excerney of	에 이렇게 가져있는 데이터 상황을 수가 물러가 하며 가슴 가려에 가슴 데이터, 데이터, 이는 가지 않는 것이다. 가져 물러가지 

Actuated Cycle Length: 58.3 Natural Cycle: 90 Control Type: Actuated-Uncoordinated

Splits and Phases: 1: Burrill Street & Essex Street

<b>1</b> Ø2	↓ Ø6	<b>√</b> Ø3		Ako9
19 s	12 s	15 5	39 s	20 s
		Ø8		H Company of the second se

## 2020 Existing Wkdy AM 1: Burrill Street & Essex Street

	≯	-	$\rightarrow$	-	-		*	<b>†</b>	1	1	÷.	-
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	f,			<b>*</b>			4	
Traffic Volume (vph)	0	395	54	168	281	0	41	3	152	0	1	0
Future Volume (vph)	0	395	54	168	281	0	41	3	152	0	1	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	16	12	11	12	12	12	14	12	12	10	12
Total Lost time (s)		4.0		4.0	4.0			4.0			4.0	
Lane Util. Factor		1.00		1.00	1.00			1.00			1.00	
Frt		0.98		1.00	1.00			0.90			1.00	
Fit Protected		1.00		0.95	1.00			0.99			1.00	
Satd. Flow (prot)		2046		1728	1827			1782			1773	
FIt Permitted		1.00		0.17	1.00			0.99			1.00	
Satd. Flow (perm)		2046	1.22	314	1827			1782	1000	1.1	1773	1.5
Peak-hour factor, PHF	0.90	0.90	0.90	0.84	0.84	0.84	0.84	0.84	0.84	0.25	0.25	0.25
Adj. Flow (vph)	0	439	60	200	335	0	49	4	181	0	4	0
RTOR Reduction (vph)	0	5	0	0	0	0	0	116	0	0	0	0
Lane Group Flow (vph)	0	494	0	200	335	0	0	118	0	0	4	0
Heavy Vehicles (%)	2%	4%	0%	1%	4%	0%	0%	0%	1%	0%	0%	0%
Turn Type		NA		pm+pt	NA		Split	NA			NA	
Protected Phases		4		3	8		2	2			6	
Permitted Phases	4			8						6		
Actuated Green, G (s)		20.7		34.7	34.7			9.7			0.7	
Effective Green, g (s)		21.7		35.7	35.7			9.7			0.7	
Actuated g/C Ratio		0.34		0.55	0.55			0.15			0.01	
Clearance Time (s)		5.0		5.0	5.0			4.0			4.0	
Vehicle Extension (s)		3.0		3.0	3.0			3.0			3.0	
Lane Grp Cap (vph)		687		392	1009			267			19	
v/s Ratio Prot		c0.24		c0.08	0.18			c0.07			c0.00	
v/s Ratio Perm				0.20							Sec.	
v/c Ratio		0.72		0.51	0.33			0.44			0.21	
Uniform Delay, d1		18.8		10.0	7.9			25.0			31.7	
Progression Factor		1.00		1.00	1.00			1.00			1.00	
Incremental Delay, d2		3.6		1.1	0.2			1.2			5.5	
Delay (s)		22.4		11.2	8.1			26.1			37.1	
Level of Service		С		В	A			С			D	
Approach Delay (s)		22.4			9.2			26.1			37.1	
Approach LOS		С			A			С			D	
Intersection Summary	S. Mar	ind Visto	1-16-523	19 <sup>21</sup>   1922   1			nimi - m		s i una	MAR 4-6		
HCM 2000 Control Delay			17.6	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capacity	ratio		0.57									
Actuated Cycle Length (s)			64.6		um of los				20.5			
Intersection Capacity Utilization	ſ		67.3%	IC	U Level	of Service			С			
Analysis Period (min)			15									

# 2020 Existing Wkdy PM 1: Burrill Street & Essex Street

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	Þ			4			4	
Traffic Volume (vph)	1	431	57	192	540	0	88	1	227	0	0	1
Future Volume (vph)	1	431	57	192	540	0	88	1	227	0	0	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	16	12	11	12	12	12	14	12	12	10	12
Storage Length (ft)	0		0	175		0	0		0	0		C
Storage Lanes	0		0	1		0	0		0	0		C
Taper Length (ft)	25			25			25			25		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		475			300			389			94	
Travel Time (s)		10.8	1.76 M.C.		6.8			8.8			2.1	
Peak Hour Factor	0.91	0.91	0.91	0.92	0.92	0.92	0.81	0.81	0.81	0.25	0.25	0.25
Heavy Vehicles (%)	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)		114T (54)										
Lane Group Flow (vph)	0	538	0	209	587	0	0	390	0	0	4	C
Turn Type	Perm	NA	1.000	pm+pt	NA		Split	NA			NA	
Protected Phases	1.01111	4		3	8		2	2			6	
Permitted Phases	4			8						6		
Detector Phase	4	4	1. 22	3	8		2	2		6	6	
Switch Phase							_					
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	22.5	22.5		10.0	22.5		19.0	19.0		12.0	12.0	
Total Split (s)	39.0	39.0		15.0	54.0		19.0	19.0		12.0	12.0	
Total Split (%)	37.1%	37.1%		14.3%	51.4%		18.1%	18.1%		11.4%	11.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	1.0	-1.0		-1.0	-1.0		1.0	0.0			0.0	
Total Lost Time (s)		4.0		4.0	4.0			4.0			4.0	
Lead/Lag	Lag	Lag		Lead	4.0			1.0			1.0	
Lead-Lag Optimize?	Yes	Yes		Yes								
Recall Mode	None	None		None	None		None	None		None	None	
v/c Ratio	NULLE	0.73		0.51	0.55		None	0.75		Hone	0.01	
		27.0		14.1	13.5			31.4			0.0	
Control Delay		0.0		0.0	0.0			0.0			0.0	
Queue Delay		27.0		14.1	13.5			31.4			0.0	
Total Delay		152		28	101			90			0.0	
Queue Length 50th (ft)		447		130	414			#357			0	
Queue Length 95th (ft)				150				#337 309			14	
Internal Link Dist (ft)		395		475	220			209			14	
Turn Bay Length (ft)		4000		175	4500			E10			580	
Base Capacity (vph)		1200		448	1529			518				
Starvation Cap Reductn		0		0	28			0			0	
Spillback Cap Reductn		0		0	0			0			0	
Storage Cap Reductn		0		0	0			0			0	
Reduced v/c Ratio		0.45		0.47	0.39			0,75			0.01	
Intersection Summary Area Type:	Other				M0			Section			문지하	2.14

Cycle Length: 105

Lane Group	Ø9				
Lane Configurations					
Traffic Volume (vph)					
Future Volume (vph)					
Ideal Flow (vphpl)			- Ash and a second		
Lane Width (ft)					
Storage Length (ft)					
Storage Lanes					
Taper Length (ft)					
Right Turn on Red					
Link Speed (mph)					
Link Distance (ft)					
Travel Time (s)					
Peak Hour Factor					
Heavy Vehicles (%)					
Shared Lane Traffic (%)					
Lane Group Flow (vph)	1.4				
Turn Type					
Protected Phases	9				
Permitted Phases					
Detector Phase	2. 19.0 11.80				
Switch Phase					
Minimum Initial (s)	5.0	State State State			
Minimum Split (s)	22.5				
Total Split (s)	20.0	Carlos and the set of			
Total Split (%)	19%				
Yellow Time (s)	3.5	n fils, and account of			
All-Red Time (s)	1.0				
Lost Time Adjust (s)	12-14 · 12-14-14-14-14-14-14-14-14-14-14-14-14-14-				
Total Lost Time (s)					
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode	None				
v/c Ratio					
Control Delay					
Queue Delay					
Total Delay					
Queue Length 50th (ft)					
Queue Length 95th (ft)					
Internal Link Dist (ft)					
Turn Bay Length (ft)					
Base Capacity (vph)					
Starvation Cap Reductn					
Spillback Cap Reductn					
Storage Cap Reductn					
Reduced v/c Ratio					
Intersection Summary		11-2 - 14 - 14 - 19/8		AUGERCI-Star	
intersection Summary					

Actuated Cycle Length: 65.8

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

Splits and Phases:	1: Burrill Street & Essex Street
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<b>↑</b> ø2	Ø6	<b>√</b> Ø3		Ak <sub>09</sub>
19 s	12 s	15 s	39 5	20 s
		Ø8		
		54 s	Har Strange group and the strange of the	

# 2020 Existing Wkdy PM 1: Burrill Street & Essex Street

	٨		7	4	+	*	•	1	1	1	Ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	4Î			4			<del>(</del> )	
Traffic Volume (vph)	1	431	57	192	540	0	88	1	227	0	0	1
Future Volume (vph)	1	431	57	192	540	0	88	1	227	0	0	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	16	12	11	12	12	12	14	12	12	10	12
Total Lost time (s)	62040	4.0		4.0	4.0			4.0			4.0	
Lane Util. Factor		1.00		1.00	1.00			1.00			1.00	
Frt		0.98		1.00	1.00			0.90			0.86	
Flt Protected		1.00		0.95	1.00			0.99			1.00	
Satd. Flow (prot)		2101		1745	1881			1805			1534	
Flt Permitted		1.00		0.19	1.00			0.99			1.00	
Satd. Flow (perm)		2099		352	1881		13. 14	1805	N 24	31, ka <sup>10</sup>	1534	34. A. T
Peak-hour factor, PHF	0.91	0.91	0.91	0.92	0.92	0.92	0.81	0.81	0.81	0.25	0.25	0.25
Adj. Flow (vph)	1	474	63	209	587	0	109	1	280	0	0	4
RTOR Reduction (vph)	0	5	0	0	0	0	0	79	0	0	4	0
Lane Group Flow (vph)	0	533	0	209	587	0	0	311	0	0	0	0
Heavy Vehicles (%)	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Perm	NA		pm+pt	NA	1.8.5	Split	NA	S-TEN	1 X. 1	NA	
Protected Phases		4		3	8		2	2			6	
Permitted Phases	4		mr S	8						6		
Actuated Green, G (s)		22.1		36.0	36.0			16.1			0.7	
Effective Green, g (s)		23.1		37.0	37.0			16.1			0.7	
Actuated g/C Ratio		0.32		0.51	0.51			0.22			0.01	
Clearance Time (s)		5.0		5.0	5.0			4.0			4.0	
Vehicle Extension (s)		3.0		3.0	3.0			3.0			3.0	
Lane Grp Cap (vph)	1242	667	sup -mi	369	958			400			14	
v/s Ratio Prot				0.08	c0.31			c0.17			c0.00	
v/s Ratio Perm		c0.25		0.21								
v/c Ratio		0.80		0.57	0.61			0.78			0.00	
Uniform Delay, d1		22.6		13.2	12.7			26.6			35.6	
Progression Factor		1.00		1.00	1.00			1.00			1.00	
Incremental Delay, d2		6.7		2.0	1.2			9.1			0.1	
Delay (s)		29.3		15.2	13.9			35.7			35.7	
Level of Service		С		В	В			D			D	
Approach Delay (s)		29.3			14.2			35.7			35.7	
Approach LOS		С			В			D			D	
Intersection Summary		ala con l		in the second		방송자리는	W.S. K	$d \sim 10^{-1}$			Stand III	
HCM 2000 Control Delay	1.1		23.8	Н	CM 2000	Level of S	Service		С			
HCM 2000 Volume to Capa	city ratio		0.73									
Actuated Cycle Length (s)	1 A 1		72.6	S	um of los	t time (s)			20.5			
Intersection Capacity Utiliza	ation		90.2%	IC	CU Level	of Service			E			
Analysis Period (min)			15									
o Critical Lano Group												

## 2028 No-Build Wkdy AM 1: Burrill Street & Essex Street

01/06/2021

Lane Group Lane Configurations Traffic Volume (vph)	EBL							•				
Lane Configurations	bas had bee	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBP
		4		ሻ	4Î			\$			4	
	0	439	60	182	315	0	45	3	168	0	1	(
Future Volume (vph)	0	439	60	182	315	0	45	3	168	0	1	(
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	16	12	11	12	12	12	14	12	12	10	12
Storage Length (ft)	0		0	175		0	0		0	0		C
Storage Lanes	0		0	1		0	0		0	0		(
Taper Length (ft)	25			25			25			25		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		475			300			389			94	
Travel Time (s)		10.8			6.8			8.8			2.1	
Peak Hour Factor	0.90	0.90	0.90	0.84	0.84	0.84	0.84	0.84	0.84	0.25	0.25	0.25
Heavy Vehicles (%)	2%	4%	0%	1%	4%	0%	0%	0%	1%	0%	0%	0%
Shared Lane Traffic (%)	270	7/0	070	170	170	0,0	0,0	010		070		
Lane Group Flow (vph)	0	555	0	217	375	0	0	258	0	0	4	- 0
	0	NA	U	pm+pt	NA	U	Split	NA	Ū	v	NA	,
Turn Type		4		3	8		2	2			6	
Protected Phases	A	4		8	0		2	4		6	0	
Permitted Phases	4	4		3	8		2	2		6	6	
Detector Phase	4	4		3	0		4	4		0	0	
Switch Phase	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Initial (s)	5.0	5.0		5.0			5.0 19.0	19.0		12.0	12.0	
Minimum Split (s)	22.5	22.5		10.0	22.5						12.0	
Total Split (s)	39.0	39.0		15.0	54.0		19.0	19.0		12.0		
Total Split (%)	37.1%	37.1%		14.3%	51.4%		18.1%	18.1%		11.4%	11.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		-1.0		-1.0	-1.0			0.0			0.0	
Total Lost Time (s)		4.0		4.0	4.0			4.0			4.0	
Lead/Lag	Lag	Lag		Lead								
Lead-Lag Optimize?	Yes	Yes		Yes								
Recall Mode	None	None		None	None		None	None		None	None	
v/c Ratio		0.71		0.50	0.33			0.63			0.02	
Control Delay		25.0		15.3	9.4			22.4			37.0	
Queue Delay		0.0		0.0	0.0			0.0			0.0	
Total Delay		25.0		15.3	9.4			22.4			37.0	
Queue Length 50th (ft)		142		21	41			35			1	
Queue Length 95th (ft)		#512		#164	221			145			4	
Internal Link Dist (ft)		395			220			309			14	
Turn Bay Length (ft)				175								
Base Capacity (vph)		1262		447	1524			571			249	
Starvation Cap Reductn		0		0	0			0			0	
Spillback Cap Reductn		0		0	0			0			0	
Storage Cap Reductn		Õ		0	Ő			0			0	
Reduced v/c Ratio		0.44		0.49	0.25			0.45			0.02	
Intersection Summary					un in Stati	n	11.15.50	a Sir a	ni pe u			

Cycle Length: 105

Lane Group	Ø9	Suit and suit	的。這次也是		
Lane Configurations					
Traffic Volume (vph)					
Future Volume (vph)					
Ideal Flow (vphpl) Lane Width (ft)					
Storage Length (ft)					
Storage Lanes					
Taper Length (ft)					
Right Turn on Red					
Link Speed (mph)					
Link Distance (ft)					
Travel Time (s)					
Peak Hour Factor					
Heavy Vehicles (%) Shared Lane Traffic (%)					
Lane Group Flow (vph)					
Turn Type					
Protected Phases	9				
Permitted Phases					
Detector Phase					54
Switch Phase					
Minimum Initial (s)	5.0				
Minimum Split (s)	22.5				
Total Split (s)	<b>20.0</b> 19%				
Total Split (%) Yellow Time (s)	3.5				
All-Red Time (s)	1.0				
Lost Time Adjust (s)					
Total Lost Time (s)					
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode	None				
v/c Ratio					
Control Delay Queue Delay					
Total Delay					
Queue Length 50th (ft)					
Queue Length 95th (ft)					
Internal Link Dist (ft)					
Turn Bay Length (ft)					
Base Capacity (vph)					
Starvation Cap Reductn					
Spillback Cap Reductn					
Storage Cap Reductn Reduced v/c Ratio					
Intersection Summary	n an start start a	internation of the	in the second		

Actuated Cycle Length: 62.9

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

Splits and Phases:	1: Burrill Street & Essex Street
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<b>√</b> ø2	↓ Ø6	<b>√</b> Ø3	<b>→</b> <sub>Ø4</sub>	<b>#k</b> ø9
19 s	12.s	15 s	39 s	20 s
		<b>₩</b> Ø8		
		54 s		Metro available in the

## 2028 No-Build Wkdy AM 1: Burrill Street & Essex Street

01/06/2021

	≯	-	*	4	+	*	-	1	1	\$	÷.	-
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	¢Î			4			\$	
Traffic Volume (vph)	0	439	60	182	315	0	45	3	168	0	1	0
Future Volume (vph)	0	439	60	182	315	0	45	3	168	0	1	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	16	12	11	12	12	12	14	12	12	10	12
Total Lost time (s)		4.0		4.0	4.0			4.0			4.0	
Lane Util. Factor		1.00		1.00	1.00			1.00			1.00	
Frt		0.98		1.00	1.00			0.90			1.00	
Flt Protected		1.00		0.95	1.00			0.99			1.00	
Satd. Flow (prot)		2046		1728	1827			1782			1773	
Flt Permitted		1.00		0.14	1.00			0.99			1.00	
Satd. Flow (perm)		2046		258	1827			1782			1773	1.1
Peak-hour factor, PHF	0.90	0.90	0.90	0.84	0.84	0.84	0.84	0.84	0.84	0.25	0.25	0.25
Adj. Flow (vph)	0	488	67	217	375	0	54	4	200	0	4	0
RTOR Reduction (vph)	0	5	0	0	0	0	0	117	0	0	0	0
Lane Group Flow (vph)	0	550	0	217	375	0	0	141	0	0	4	0
Heavy Vehicles (%)	2%	4%	0%	1%	4%	0%	0%	0%	1%	0%	0%	0%
Turn Type		NA	1.00	pm+pt	NA	a bati	Split	NA			NA	249.1
Protected Phases		4		3	8		2	2			6	
Permitted Phases	4			8						6		
Actuated Green, G (s)		23.2		38.6	38.6			10.4			0.8	
Effective Green, g (s)		24.2		39.6	39.6			10.4			0.8	
Actuated g/C Ratio		0.35		0.57	0.57			0.15			0.01	
Clearance Time (s)		5.0		5.0	5.0			4.0			4.0	
Vehicle Extension (s)		3.0		3.0	3.0			3.0			3.0	
Lane Grp Cap (vph)		713		388	1042	1.100		267	181	1.1	20	
v/s Ratio Prot		c0.27		c0.09	0.21			c0.08			c0.00	
v/s Ratio Perm				0.23								
v/c Ratio		0.77		0.56	0.36			0.53			0.20	
Uniform Delay, d1		20.1		11.2	8.1			27.2			34.0	
Progression Factor		1.00		1.00	1.00			1.00			1.00	
Incremental Delay, d2		5.2		1.8	0.2			1.9			4.9	
Delay (s)		25.3		12.9	8.3			29.1			38.9	
Level of Service		С		В	Α			С			D	
Approach Delay (s)		25.3			10.0			29.1			38.9	
Approach LOS		С			A			С			D	
Intersection Summary		- 10 T			5				U	이 (네 1.		NO REST
HCM 2000 Control Delay			19.6	Н	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capac	city ratio		0.63									
Actuated Cycle Length (s)			69.4	S	um of lost	t time (s)			20.5			
Intersection Capacity Utilization	tion		73.0%			of Service			С			
Analysis Period (min)			15									
c Critical Lane Group												

# 2028 No-Build Wkdy PM 1: Burrill Street & Essex Street

	٦	-	$\mathbf{i}$	*	-	*	1	1	1	1	Ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$		٦	f)			4			4	
Traffic Volume (vph)	1	482	63	213	601	0	97	1	253	0	0	1
Future Volume (vph)	1	482	63	213	601	0	97	1	253	0	0	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	16	12	11	12	12	12	14	12	12	10	12
Storage Length (ft)	0		0	175		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph)		30			30			30		100 11	30	
Link Distance (ft)		475			300			389			94	
Travel Time (s)		10.8			6.8			8.8			2.1	
Peak Hour Factor	0.91	0.91	0.91	0.92	0.92	0.92	0.81	0.81	0.81	0.25	0.25	0.25
Heavy Vehicles (%)	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)	070	170	070	070	170	010	010	010	0.10	0.10		•
Lane Group Flow (vph)	0	600	0	232	653	0	0	433	0	0	4	0
Turn Type	Perm	NA	0	pm+pt	NA	Ū	Split	NA	•		NA	
Protected Phases	I CIIII	4		3	8		2	2			6	
Permitted Phases	1	7		8	U		2	~		6	v	
Detector Phase	4	- 4		3	8		2	2		6	6	
Switch Phase	4	4		5	0		2	4		0	0	
	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Initial (s)		22.5		10.0	22.5		19.0	19.0		12.0	12.0	
Minimum Split (s)	22.5			15.0	54.0		19.0	19.0		12.0	12.0	
Total Split (s)	39.0	39.0					18.1%	18.1%		11.4%	11.4%	
Total Split (%)	37.1%	37.1%		14.3%	51.4%			3.0		3.0	3.0	
Yellow Time (s)	4.0	4.0		4.0	4.0		3.0				1.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	0.0	
Lost Time Adjust (s)		-1.0		-1.0	-1.0			0.0				
Total Lost Time (s)		4.0		4.0	4.0			4.0			4.0	
Lead/Lag	Lag	Lag		Lead								
Lead-Lag Optimize?	Yes	Yes		Yes								
Recall Mode	None	None		None	None		None	None		None	None	
v/c Ratio		0.76		0.55	0.59			0.89			0.01	
Control Delay		27.8		15.5	13.9			44.9			0.0	
Queue Delay		0.0		0.0	0.0			0.0			0.0	
Total Delay		27.8		15.5	13.9			44.9			0.0	
Queue Length 50th (ft)		183		32	118			121			0	
Queue Length 95th (ft)		#559		#167	483			#417			0	
Internal Link Dist (ft)		395			220			309			14	
Turn Bay Length (ft)				175								
Base Capacity (vph)		1115		436	1423			489			543	
Starvation Cap Reductn		0		0	26			0			0	
Spillback Cap Reductn		0		0	0			0			0	
Storage Cap Reductn		0		0	0			0			0	
Reduced v/c Ratio		0.54		0.53	0,47			0.89			0.01	
Intersection Summary			en e.			E Participation		S. S. #	NEL an a			120
Area Type:	Other											
Cycle Length: 105												

Cycle Length: 105

Lane Group Ø9	
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Lane Width (ft)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Right Turn on Red	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	
Heavy Vehicles (%)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Turn Type	
Protected Phases 9	
Permitted Phases	
Detector Phase	
Switch Phase	
Minimum Initial (s) 5.0	
Minimum Split (s) 22.5	
Total Split (s) 20.0	
Total Split (%) 19%	
Yellow Time (s) 3.5	
All-Red Time (s) 1.0	
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Recall Mode None	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	에는 것이 있는 것은 것이 같은 것을 모인 것이 있다. 이상 가지만 것이 있는 것은 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있다. 것이 있는 것이 있는 것이 있는 것이 있

### Actuated Cycle Length: 69.7

Natural Cycle: 100

Control Type: Actuated-Uncoordinated

Splits and Phases:	1: Burrill Street & Essex Street

<b>√</b> ø2	<b>↓</b> Ø6	<b>√</b> Ø3		Ak <sub>Ø9</sub>
19 s	12.5	15 s	39 s	20 s
		₹ø8		
		54 s		

## 2028 No-Build Wkdy PM 1: Burrill Street & Essex Street

	۶	-	$\mathbf{r}$	4	-	×.	1	Ť	1	1	¥.	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		٣	4			4			44	
Traffic Volume (vph)	1	482	63	213	601	0	97	1	253	0	0	1
Future Volume (vph)	1	482	63	213	601	0	97	1	253	0	0	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	16	12	11	12	12	12	14	12	12	10	12
Total Lost time (s)		4.0		4.0	4.0			4.0			4.0	
Lane Util. Factor		1.00		1.00	1.00			1.00			1.00	
Frt		0.98		1.00	1.00		- Shik	0.90			0.86	
Fit Protected		1.00		0.95	1.00			0.99			1.00	
Satd. Flow (prot)		2101		1745	1881			1805			1534	
Flt Permitted		1.00		0.19	1.00			0.99			1.00	
Satd. Flow (perm)	nx <sup>21</sup>	2100	1 - 3	345	1881	T 🗞 T	PENU .	1805	22575	e Chastre	1534	
Peak-hour factor, PHF	0.91	0.91	0.91	0.92	0.92	0.92	0.81	0.81	0.81	0.25	0.25	0.25
Adj. Flow (vph)	1	530	69	232	653	0	120	1	312	0	0	4
RTOR Reduction (vph)	0	5	0	0	0	0	0	82	0	0	4	0
Lane Group Flow (vph)	0	595	0	232	653	0	0	351	0	0	0	0
Heavy Vehicles (%)	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Perm	NA		pm+pt	NA		Split	NA			NA	
Protected Phases		4		3	8		2	2			6	
Permitted Phases	4			8						6		
Actuated Green, G (s)		25.4		40.1	40.1			15.8			0.7	
Effective Green, g (s)		26.4		41.1	41.1			15.8			0.7	
Actuated g/C Ratio		0.35		0.54	0.54			0.21			0.01	
Clearance Time (s)		5.0		5.0	5.0			4.0			4.0	
Vehicle Extension (s)		3.0		3.0	3.0			3.0			3.0	
Lane Grp Cap (vph)		724		381	1010			372			14	
v/s Ratio Prot				0.09	c0.35			c0.19			c0.00	
v/s Ratio Perm		c0.28		0.24								
v/c Ratio		0.82		0.61	0.65			0.94			0.00	
Uniform Delay, d1		22.9		13.3	12.5			29.9			37.6	
Progression Factor		1.00		1.00	1.00			1.00			1.00	
Incremental Delay, d2		7.5		2.8	1.4			32.4			0.1	
Delay (s)		30.4		16.1	14.0			62.3			37.6	
Level of Service		С		В	В			E			D	
Approach Delay (s)		30.4			14.5			62.3			37.6	
Approach LOS		С			В			E			D	
Intersection Summary	25-11-11-11					т. н. п. <b>1</b>	NUT A	(Eduarda)	(dite)=")		n ya Xik	
HCM 2000 Control Delay			30.3	Н	ICM 2000	Level of S	Service		С			
HCM 2000 Volume to Capac	ity ratio		0.79									
Actuated Cycle Length (s)			76.5		um of los				20,5			
Intersection Capacity Utilizat	ion		98.5%	10	CU Level	of Service			F			
Analysis Period (min)			15									

### 2028 Build Wkdy AM 1: Burrill Street & Essex Street

01/07/2021

	≯	$\rightarrow$	7	1	-	~	1	Ť	1	-	÷	-
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻ	4			\$			\$	
Traffic Volume (vph)	0	444	60	188	327	0	45	3	170	0	1	0
Future Volume (vph)	0	444	60	188	327	0	45	3	170	0	1	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	16	12	11	12	12	12	14	12	12	10	12
Storage Length (ft)	0		0	175		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		475			300			389			94	
Travel Time (s)		10.8			6.8			8.8			2.1	
Peak Hour Factor	0.90	0.90	0.90	0.84	0.84	0.84	0.84	0.84	0.84	0.25	0.25	0.25
Heavy Vehicles (%)	2%	4%	0%	1%	4%	0%	0%	0%	1%	0%	0%	0%
Shared Lane Traffic (%)	270	170	0,0	170	1.0	0.0	0,0	0,0				
Lane Group Flow (vph)	0	560	0	224	389	0	0	260	0	0	4	0
Turn Type	v	NA	U	pm+pt	NA	v	Split	NA	Ū		NA	Ū
Protected Phases		4		3	8		2	2		1 Cartan	6	
Permitted Phases	4			8	Ū		2	-		6	Ū	
Detector Phase	4	4		3	8		2	2		6	6	
Switch Phase				U	0		-	-		U	0	
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	22.5	22.5		10.0	22.5		19.0	19.0		12.0	12.0	
Total Split (s)	39.0	39.0		15.0	54.0		19.0	19.0		12.0	12.0	0200
Total Split (%)	37.1%	37.1%		14.3%	51.4%		18.1%	18.1%		11.4%	11.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	4.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	1.0	-1.0		-1.0	-1.0		1.0	0.0		1.0	0.0	
		4.0		4.0	4.0			4.0			4.0	
Total Lost Time (s) Lead/Lag	Log	Lag		Lead	4.0			4.0			4.0	
	Lag Yes	Yes		Yes								
Lead-Lag Optimize? Recall Mode	None	None		None	None		None	None		None	None	
v/c Ratio	None	0.71		0.52	0.34		NONE	0.64		None	0.02	
		25.1		16.1	9.5			22.8			37.0	
Control Delay		25.1		0.0	9.5 0.0			0.0			0.0	
Queue Delay		25.1		16.1	9.5			22.8			37.0	
Total Delay				22	9.5 43			36			37.0	
Queue Length 50th (ft)		145						147			4	
Queue Length 95th (ft)		#519		#176	230			309			4 14	
Internal Link Dist (ft)		395		475	220			209			14	
Turn Bay Length (ft)		1011		175	1510			ECE			045	
Base Capacity (vph)		1241		441	1510			565			245	
Starvation Cap Reductn		0		0	0			0			0	
Spillback Cap Reductn		0		0	0			0			0	
Storage Cap Reductn		0		0	0			0			0	
Reduced v/c Ratio		0.45		0.51	0.26			0.46			0.02	
Intersection Summary	1052									A., 1987		
Area Type:	Other											
Cycle Length: 105												

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Lane Group	Ø9			- 100 Sed	di sila ya
Lane Configurations					
Traffic Volume (vph)					
Future Volume (vph)					
Ideal Flow (vphpl)					
Lane Width (ft)					
Storage Length (ft)					
Storage Lanes					
Taper Length (ft)					
Right Turn on Red					
Link Speed (mph)					
Link Distance (ft)					
Travel Time (s)					
Peak Hour Factor					
Heavy Vehicles (%)					
Shared Lane Traffic (%)					
Lane Group Flow (vph)					
Turn Type					
Protected Phases	9				
Permitted Phases					
Detector Phase				215	1.5.
Switch Phase					
Minimum Initial (s)	5.0				
Minimum Split (s)	22.5				
Total Split (s)	20.0				
Total Split (%)	19%				
Yellow Time (s)	3.5				
All-Red Time (s)	1.0				
Lost Time Adjust (s)					
Total Lost Time (s)					
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode	None				
v/c Ratio					
Control Delay					
Queue Delay					
Total Delay					
Queue Length 50th (ft)					
Queue Length 95th (ft)					
Internal Link Dist (ft)					
Turn Bay Length (ft)					
Base Capacity (vph)					
Starvation Cap Reductn					
Spillback Cap Reductn					
Storage Cap Reductn					
Reduced v/c Ratio					
Intersection Summary		March 1992	580 F 1 - N -		

### 2028 Build Wkdy AM 1: Burrill Street & Essex Street

Actuated Cycle Length: 63.7 Natural Cycle: 90

Control Type: Actuated-Uncoordinated

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 1: Burrill Street & Essex Street

<b>√1</b> ø2	↓ Ø6	<b>√</b> Ø3		AR <sub>Ø9</sub>
19 s	12 s	15 s	39 s	20 s
		₹_Ø8		
		54 s		Contraction and Contraction of the

### 2028 Build Wkdy AM 1: Burrill Street & Essex Street

01/07/2021

	٠	-	7	*	-	*	1	1	1	1	Ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		7	1+			4			4	
Traffic Volume (vph)	0	444	60	188	327	0	45	3	170	0	1	0
Future Volume (vph)	0	444	60	188	327	0	45	3	170	0	1	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	16	12	11	12	12	12	14	12	12	10	12
Total Lost time (s)		4.0		4.0	4.0			4.0			4.0	16.51
Lane Util. Factor		1.00		1.00	1.00			1.00			1.00	
Frt		0.98		1.00	1.00			0.90			1.00	Q., 10
Flt Protected		1.00		0.95	1.00			0.99			1.00	
Satd. Flow (prot)		2046		1728	1827			1782			1773	
Flt Permitted		1.00		0.14	1.00			0.99			1.00	
Satd. Flow (perm)	200.0	2046	14	253	1827	1		1782	1.18	1.00	1773	
Peak-hour factor, PHF	0.90	0.90	0.90	0.84	0.84	0.84	0.84	0.84	0.84	0.25	0.25	0.25
Adj. Flow (vph)	0	493	67	224	389	0	54	4	202	0	4	0
RTOR Reduction (vph)	0	5	0	0	0	0	0	118	0	0	0	0
Lane Group Flow (vph)	0	555	0	224	389	0	0	142	0	0	4	0
Heavy Vehicles (%)	2%	4%	0%	1%	4%	0%	0%	0%	1%	0%	0%	0%
Turn Type	e. 17	NA		pm+pt	NA		Split	NA			NA	
Protected Phases		4		3	8		2	2			6	
Permitted Phases	4			8						6		- S
Actuated Green, G (s)		23.7		39.4	39.4			10.4			0.8	
Effective Green, g (s)		24.7		40.4	40.4			10.4			0.8	
Actuated g/C Ratio		0.35		0.57	0.57			0.15			0.01	
Clearance Time (s)		5.0		5.0	5.0			4.0			4.0	123
Vehicle Extension (s)		3.0		3.0	3.0			3.0			3.0	
Lane Grp Cap (vph)		718		390	1049			263			20	
v/s Ratio Prot		c0.27		c0.10	0.21			c0.08	_		c0.00	
v/s Ratio Perm				0.23								
v/c Ratio		0.77		0.57	0.37			0.54			0.20	
Uniform Delay, d1		20.3		11.3	8.1			27.7			34.4	
Progression Factor		1.00		1.00	1.00			1.00			1.00	
Incremental Delay, d2		5.2		2.0	0.2			2.1			4.9	
Delay (s)		25.5		13,4	8.3			29.8			39.3	
Level of Service		С		В	А			С			D	
Approach Delay (s)		25.5			10.2			29.8			39.3	
Approach LOS		С			В			С			D	
Intersection Summary	ê jîye	3.51		Tis, le	$\mathbb{E}_{\mathbb{F}}$	1.1.1	118.21	101917	10 10	G L L	11-2-14	144
HCM 2000 Control Delay			19.8	H	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capacity	ratio		0.64									
Actuated Cycle Length (s)			70.3		um of lost				20.5			
Intersection Capacity Utilization	1		74.0%	IC	U Level o	of Service			D			
Analysis Period (min)			15									
c Critical Lane Group												

# 2028 Build Wkdy PM 1: Burrill Street & Essex Street

01/07/2021

	۶	-+	$\mathbf{F}$	4	-	×.	*	Ť	1	1	Ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		¢.		۲	f,			4			\$	
Traffic Volume (vph)	1	494	63	218	609	0	97	1	260	0	0	1
Future Volume (vph)	1	494	63	218	609	0	97	1	260	0	0	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	16	12	11	12	12	12	14	12	12	10	12
Storage Length (ft)	0		0	175		0	0		0	0		0
Storage Lanes	0		0	1		0	0		0	0		0
Taper Length (ft)	25			25			25		15 13	25		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph)	80 BL 3 B	30	100		30			30			30	
Link Distance (ft)		475			300			389			94	
Travel Time (s)		10.8			6.8			8.8			2.1	
Peak Hour Factor	0.91	0.91	0.91	0.92	0.92	0.92	0.81	0.81	0.81	0.25	0.25	0.25
Heavy Vehicles (%)	0.91	1%	0.91	0.52	1%	0%	0%	0%	0%	0%	0%	0%
	070	1 70	0 /0	070	170	0 /0	070	070	070	070	070	070
Shared Lane Traffic (%)	0	613	0	237	662	0	0	442	0	0	4	0
Lane Group Flow (vph)		NA	0		NA	0		NA	U	U	NA	0
Turn Type	Perm			pm+pt			Split				NA 6	
Protected Phases	NO COM	4		3	8		2	2		0	0	
Permitted Phases	4			8	0		0	0		6	c	
Detector Phase	4	4		3	8		2	2		6	6	
Switch Phase										5.0	5.0	
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	22.5	22.5		10.0	22.5		19.0	19.0		12.0	12.0	
Total Split (s)	39.0	39.0		15.0	54.0		19.0	19.0		12.0	12.0	
Total Split (%)	37.1%	37.1%		14.3%	51.4%		18.1%	18.1%		11.4%	11.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		-1.0		-1.0	-1.0			0.0			0.0	
Total Lost Time (s)		4.0		4.0	4.0			4.0			4.0	
Lead/Lag	Lag	Lag		Lead								
Lead-Lag Optimize?	Yes	Yes		Yes								
Recall Mode	None	None		None	None		None	None		None	None	
v/c Ratio		0.76		0.56	0.59			0.91			0.01	
Control Delay		28.0		15.8	13.9			48.8			0.0	
Queue Delay		0.0		0.0	0.0			0.0			0.0	
Total Delay		28.0		15.8	13.9			48.8			0.0	
Queue Length 50th (ft)		190		33	121			128			0	
Queue Length 95th (ft)		#579		#176	494			#427			0	
Internal Link Dist (ft)		395			220			309			14	
Turn Bay Length (ft)				175								
Base Capacity (vph)		1097		435	1400			485			539	
Starvation Cap Reductn		0		0	25			0			0	
Spillback Cap Reductn		0		0	0			0			0	
Storage Cap Reductn		0		0	0			0			Ő	
Reduced v/c Ratio		0.56		0.54	0.48			0.91			0.01	
										2.5		
Intersection Summary Area Type:	Other	arts - As	- C - C			4				2.2.1		-
Cycle Length: 105	onio											

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Lane Group	Ø9	See Star Hitter of	5.42	ing 23 x	1.24.64		Suffrage States
Lane Configurations							
Traffic Volume (vph)							
Future Volume (vph)							
Ideal Flow (vphpl)							
Lane Width (ft)							
Storage Length (ft)							
Storage Lanes							
Taper Length (ft)							
Right Turn on Red							
Link Speed (mph)							
Link Distance (ft)							
Travel Time (s)							
Peak Hour Factor							
Heavy Vehicles (%)						1-12-1-1	
Shared Lane Traffic (%)							
Lane Group Flow (vph)							
Turn Type							
Protected Phases	9						
Permitted Phases	3						
Detector Phase	with the state of the				74.335		
Switch Phase							
	5.0						
Minimum Initial (s)	22.5						
Minimum Split (s)							
Total Split (s)	20.0						
Total Split (%)	19%						
Yellow Time (s)	3.5 1.0						
All-Red Time (s)	1.0						
Lost Time Adjust (s)							
Total Lost Time (s)							
Lead/Lag							
Lead-Lag Optimize?	N						
Recall Mode	None						
v/c Ratio							
Control Delay							
Queue Delay							
Total Delay							
Queue Length 50th (ft)							
Queue Length 95th (ft)							
Internal Link Dist (ft)							
Turn Bay Length (ft)							
Base Capacity (vph)							
Starvation Cap Reductn							
Spillback Cap Reductn							
Storage Cap Reductn							
Reduced v/c Ratio							
Intersection Summary			Y III B		"" i d		

### 2028 Build Wkdy PM 1: Burrill Street & Essex Street

Actuated Cycle Length: 70.6

Natural Cycle: 100

Control Type: Actuated-Uncoordinated

95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 1: Burrill Street & Essex Street

<b>1</b> Ø2	↓ Ø6	<b>√</b> Ø3		A Bog
19 s	12 5	15 s	39 \$	20 s
		Ø8		
		54 5		

### 2028 Build Wkdy PM 1: Burrill Street & Essex Street

01/07/2021

	٠		7	*	-	*	1	1	1	1	÷.	-
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$		۳	\$⇒			4			4	
Traffic Volume (vph)	1	494	63	218	609	0	97	1	260	0	0	1
Future Volume (vph)	1	494	63	218	609	0	97	1	260	0	0	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	16	12	11	12	12	12	14	12	12	10	12
Total Lost time (s)		4.0		4.0	4.0			4.0			4.0	
Lane Util. Factor		1.00		1.00	1.00			1.00			1.00	
Frt		0.98		1.00	1.00			0.90			0.86	
Flt Protected		1.00		0.95	1.00			0.99			1.00	
Satd. Flow (prot)		2102		1745	1881			1803			1534	
Flt Permitted		1.00		0.19	1.00			0.99			1.00	
Satd. Flow (perm)		2101		345	1881			1803	Sec. 1	- dia	1534	E0
Peak-hour factor, PHF	0.91	0.91	0,91	0.92	0.92	0.92	0.81	0.81	0.81	0.25	0.25	0.25
Adj. Flow (vph)	1	543	69	237	662	0	120	1	321	0	0	4
RTOR Reduction (vph)	0	5	0	0	0	0	0	84	0	0	4	0
Lane Group Flow (vph)	0	608	0	237	662	0	0	358	0	0	0	0
Heavy Vehicles (%)	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Perm	NA		pm+pt	NA	No.	Split	NA	CUIQT,	Concertion of	NA	
Protected Phases		4		3	8		2	2			6	
Permitted Phases	4			8						6		
Actuated Green, G (s)		26.2		41.1	41.1			15.8			0.7	
Effective Green, g (s)		27.2		42.1	42.1			15.8			0.7	
Actuated g/C Ratio		0.35		0.54	0.54			0.20			0.01	
Clearance Time (s)		5.0		5.0	5.0			4.0			4.0	
Vehicle Extension (s)		3.0		3.0	3.0			3.0			3.0	
Lane Grp Cap (vph)		737	- U.V	384	1021	100		367			13	
v/s Ratio Prot				0.09	c0.35			c0.20			c0.00	
v/s Ratio Perm		c0.29		0.25								
v/c Ratio		0.83		0.62	0.65			0.97			0.00	
Uniform Delay, d1		23.0		13.3	12.5			30.6			38.1	
Progression Factor		1.00		1.00	1.00			1.00			1.00	
Incremental Delay, d2		7.5		2.9	1.4			39.9			0.1	
Delay (s)		30.5		16.3	13.9			70.5			38.1	
Level of Service		С		В	В			E			D	
Approach Delay (s)		30.5			14.5			70.5			38.1	
Approach LOS		С			В			E			D	
Intersection Summary			iles III.	v kul o s		1.10	d el	12	1.18		miez <sup>en</sup>	THE
HCM 2000 Control Delay			32.2	Н	CM 2000	Level of S	Service		С			
HCM 2000 Volume to Capacity	/ ratio		0.80									
Actuated Cycle Length (s)			77.5		um of losi				20.5			
Intersection Capacity Utilization	n		100.0%	IC	CU Level o	of Service			G			
Analysis Period (min)			15									
c Critical Lane Group												

Essex Street at Burpee Road

# 2020 Existing Wkdy AM 4: Essex Street & Burpee Road

	≯	-	$\rightarrow$	1	-	*	1	<b>†</b>	1	1	Ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations	ሻ	ţ,			\$			\$			\$	
Traffic Volume (vph)	27	521	0	0	397	40	0	0	1	69	0	46
Future Volume (vph)	27	521	0	0	397	40	0	0	1	69	0	46
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	12	12	16	12	12	12	12	12	12	12
Storage Length (ft)	75		0	0		0	0		0	0		C
Storage Lanes	1		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		228			590			449			248	
Travel Time (s)		5.2			13.4			10.2			5.6	
Peak Hour Factor	0.85	0.85	0.85	0.87	0.87	0.87	0.25	0.25	0.25	0.73	0.73	0.73
Heavy Vehicles (%)	14%	3%	0%	0%	3%	3%	0%	0%	0%	0%	0%	3%
Shared Lane Traffic (%)	1470	070	070	070	0,0	0,0	0,0		0,0			
Lane Group Flow (vph)	32	613	0	0	502	0	0	4	0	0	158	0
Turn Type	Perm	NA	U	U	NA	v	0	NA	Ū	Split	NA	
Protected Phases	I CIIII	4			8			2		6	6	
Permitted Phases	4	4		8	0		2	4		v	v	
	4	4		8	8		2	2		6	6	
Detector Phase	4	4	1 A II	0	0		2	2		0	0	
Switch Phase	50	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Initial (s)	5.0	22.5		22.5	22.5		12.0	12.0		19.0	19.0	
Minimum Split (s)	22.5			39.0	39.0		12.0	12.0		19.0	19.0	
Total Split (s)	39.0	39.0			44.8%		13.8%	13.8%		21.8%	21.8%	
Total Split (%)	44.8%	44.8%	-	44.8%			3.0	3.0		3.0	3.0	
Yellow Time (s)	3.0	3.0		3.0	3.0						1.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0		
Lost Time Adjust (s)	0.0	0.0	1000		0.0			0.0			0.0	
Total Lost Time (s)	4.0	4.0			4.0			4.0			4.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		None	None	
v/c Ratio	0.09	0.56			0.39			0.01			0.34	
Control Delay	10.7	13.7			10.2			0.0			11.5	
Queue Delay	0.0	0.0			0.0			0.0			0.0	
Total Delay	10.7	13.7			10.2			0.0			11.5	
Queue Length 50th (ft)	2	57			40			0			7	
Queue Length 95th (ft)	29	#451			299			0			50	
Internal Link Dist (ft)		148			510			369			168	
Turn Bay Length (ft)	75											
Base Capacity (vph)	508	1537			1782			754			910	
Starvation Cap Reductn	0	0			_ 0			0			0	
Spillback Cap Reductn	0	0			0			0			0	
Storage Cap Reductn	0	0			0			0			0	
Reduced v/c Ratio	0.06	0.40			0.28			0.01			0.17	
Intersection Summary	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1			B-5	525198	Turba S.	Level Mari					MILLE!
Area Type:	Other											
Cycle Length: 87												

Cycle Length: 87

Lane Group	Ø9		The Brite	
Lane Configurations				
Traffic Volume (vph) Future Volume (vph)				
Ideal Flow (vphpl)				
Lane Width (ft)				
Storage Length (ft)				
Storage Lanes				
Taper Length (ft)				
Right Turn on Red				
Link Speed (mph)				
Link Distance (ft) Travel Time (s)				
Peak Hour Factor				
Heavy Vehicles (%)				
Shared Lane Traffic (%)				
Lane Group Flow (vph)				
Turn Type	•			
Protected Phases	9			
Permitted Phases Detector Phase				
Switch Phase				
Minimum Initial (s)	5.0			
Minimum Split (s)	17.0			
Total Split (s)	17.0			
Total Split (%)	20%			
Yellow Time (s)	3.0			
All-Red Time (s)	1.0			
Lost Time Adjust (s) Total Lost Time (s)				
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	None			
v/c Ratio				
Control Delay				
Queue Delay				
Total Delay Queue Length 50th (ft)				
Queue Length 95th (ft)				
Internal Link Dist (ft)				
Turn Bay Length (ft)				
Base Capacity (vph)				
Starvation Cap Reductn				
Spillback Cap Reductn				
Storage Cap Reductn Reduced v/c Ratio				
Intersection Summary		in the second		

Actuated Cycle Length: 40.9

Natural Cycle: 80

Control Type: Actuated-Uncoordinated

Splits and Phases:	4: Essex Street	& Burpee Road
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12 s.	19 s	39 s	17 s
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		39 s	The second s

# 2020 Existing Wkdy AM 4: Essex Street & Burpee Road

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	¢Î			\$			4			\$	
Traffic Volume (vph)	27	521	0	0	397	40	0	0	1	69	0	46
Future Volume (vph)	27	521	0	0	397	40	0	0	1	69	0	46
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	11	11	12	12	16	12	12	12	12	12	12	12
Total Lost time (s)	4.0	4.0			4.0			4.0			4.0	
Lane Util. Factor	1.00	1.00			1.00			1.00			1.00	
Frt	1.00	1.00			0.99			0.86			0.95	
FIt Protected	0.95	1.00			1.00			1.00			0.97	
Satd. Flow (prot)	1531	1783			2065			1644			1725	
Flt Permitted	0.37	1.00			1.00			1.00			0.97	
Satd. Flow (perm)	590	1783			2065	12.1	1.1	1644	See 24	1 North	1725	335.74
Peak-hour factor, PHF	0.85	0.85	0.85	0.87	0.87	0.87	0,25	0.25	0.25	0.73	0.73	0.73
Adj. Flow (vph)	32	613	0	0	456	46	0	0	4	95	0	63
RTOR Reduction (vph)	0	0	0	0	4	0	0	4	0	0	98	0
Lane Group Flow (vph)	32	613	0	0	498	0	0	0	0	0	60	0
Heavy Vehicles (%)	14%	3%	0%	0%	3%	3%	0%	0%	0%	0%	0%	3%
Turn Type	Perm	NA	Sec. 17.5.	127 193	NA	1.5	1.1	NA	1.31	Split	NA	1.0
Protected Phases		4			8			2		6	6	
Permitted Phases	4			8			2					
Actuated Green, G (s)	23.0	23.0			23.0			0.5			6.2	
Effective Green, g (s)	23.0	23.0			23.0			0.5			6.2	
Actuated g/C Ratio	0.49	0.49			0.49			0.01			0.13	
Clearance Time (s)	4.0	4.0			4.0			4.0			4.0	-1011
Vehicle Extension (s)	3.0	3.0			3.0			3.0			3.0	
Lane Grp Cap (vph)	286	865		- A.	1002	1.1		17			225	12.1
v/s Ratio Prot		c0.34			0.24			c0.00			c0.03	
v/s Ratio Perm	0.05											12.3
v/c Ratio	0.11	0.71			0.50			0.00			0.27	
Uniform Delay, d1	6.6	9.6			8.3			23.2			18.6	
Progression Factor	1.00	1.00			1.00			1.00			1.00	
Incremental Delay, d2	0.2	2.7			0.4			0.1			0.6	
Delay (s)	6.8	12.3			8.7			23.3			19.2	
Level of Service	Α	В			А			С			В	
Approach Delay (s)		12.0			8.7			23.3			19.2	
Approach LOS		В			А			С			В	
Intersection Summary	Rushille 107		a ne a	TS and its	y ny titos		T- LUN	C-State				
HCM 2000 Control Delay			11.6	Н	CM 2000	Level of \$	Service		В			
HCM 2000 Volume to Capa	city ratio		0.57									
Actuated Cycle Length (s)	1.0		47.4	S	um of losi	time (s)			16.0			
Intersection Capacity Utiliza	ation		47.4%	IC	CU Level of	of Service			А			
Analysis Period (min)			15									

## 2020 Existing Wkdy PM 4: Essex Street & Burpee Road

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	4Î			4			4			<b>↔</b>	
Traffic Volume (vph)	50	603	0	0	695	107	0	0	0	56	0	41
Future Volume (vph)	50	603	0	0	695	107	0	0	0	56	0	41
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	12	12	16	12	12	12	12	12	12	12
Storage Length (ft)	75		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph)		30	100		30			30			30	
Link Distance (ft)		228			590			449			248	
Travel Time (s)		5.2			13.4		No.	10.2			5.6	
Peak Hour Factor	0.85	0.85	0.85	0.93	0.93	0.93	0.25	0.25	0.25	0.95	0.95	0.95
	2%	1%	0.05	1%	0%	0.00	0%	0%	0%	0%	0%	0%
Heavy Vehicles (%)	270	1 70	0 /0	1 /0	070	070	070	070	070	070	070	0.0
Shared Lane Traffic (%)	50	700	0	0	862	0	0	0	0	0	102	0
Lane Group Flow (vph)	59	709	0	0		0	0	0	0		NA	U
Turn Type	Perm	NA			NA			2		Split 6	6	
Protected Phases		4		0	8		•	2		0	0	
Permitted Phases	4			8	•		2	0		0	0	
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	Same r	5.0	5.0		5.0	5.0	
Minimum Split (s)	22.5	22.5		22.5	22.5		12.0	12.0		19.0	19.0	
Total Split (s)	39.0	39.0		39.0	39.0		12.0	12.0		19.0	19.0	
Total Split (%)	44.8%	44.8%		44.8%	44.8%		13.8%	13.8%		21.8%	21.8%	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	
Total Lost Time (s)	4.0	4.0			4.0			4.0			4.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		None	None	
v/c Ratio	0.25	0.56			0.58						0.29	
Control Delay	10.9	10.4			10.3						8.2	
Queue Delay	0.0	0.0			0.0						0.0	
Total Delay	10.9	10.4			10.3						8.2	
Queue Length 50th (ft)	4	61			75						0	
Queue Length 95th (ft)	45	372			#554						36	
Internal Link Dist (ft)	÷J	148			510			369			168	
	75	140			010			000			100	
Turn Bay Length (ft)		1157			1697						702	
Base Capacity (vph)	275	1457									0	
Starvation Cap Reductn	0	0			0							
Spillback Cap Reductn	0	0			0						0 0	
Storage Cap Reductn	0	0			0							
Reduced v/c Ratio	0.21	0.49			0.51						0.15	
Intersection Summary	01			242	office the	l dia na		- 1 H			1-32	ibit effe
Area Type: Cycle Length: 87	Other											

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl) Lane Width (ft)		
Storage Length (ft)		
Storage Lanes		
Taper Length (ft)		
Right Turn on Red		
Link Speed (mph)		
Link Distance (ft)		
Travel Time (s)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph) Turn Type		
Protected Phases	9	
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	5.0	
Minimum Split (s)	17.0	
Total Split (s)	17.0	
Total Split (%)	20%	
Yellow Time (s)	3.0	
All-Red Time (s)	1.0	
Lost Time Adjust (s) Total Lost Time (s)		
Lead/Lag		
Lead-Lag Optimize?		
Recall Mode	None	
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
Queue Length 50th (ft)		
Queue Length 95th (ft) Internal Link Dist (ft)		
Turn Bay Length (ft)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		
Intersection Summary		발표 비행

Actuated Cycle Length: 45.4

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

Splits and Phases:	4: Essex Street	& Burpee Road
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12 s	19 s	39 s	17 s
		<b>₹</b> Ø8	
		39 s	

## 2020 Existing Wkdy PM 4: Essex Street & Burpee Road

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	Þ			4			4			43	
Traffic Volume (vph)	50	603	0	0	695	107	0	0	0	56	0	41
Future Volume (vph)	50	603	0	0	695	107	0	0	0	56	0	41
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	11	11	12	12	16	12	12	12	12	12	12	12
Total Lost time (s)	4.0	4.0			4.0						4.0	
Lane Util. Factor	1.00	1.00			1.00						1.00	
Frt	1.00	1.00			0.98						0.94	
Flt Protected	0.95	1.00			1.00						0.97	
Satd. Flow (prot)	1711	1818			2115						1742	
Flt Permitted	0.19	1.00			1.00						0.97	
Satd. Flow (perm)	344	1818			2115	1.5 - 1.	10.4			6.4.27	1742	
Peak-hour factor, PHF	0.85	0.85	0.85	0,93	0.93	0.93	0.25	0.25	0.25	0.95	0.95	0.95
Adj. Flow (vph)	59	709	0	0	747	115	0	0	0	59	0	43
RTOR Reduction (vph)	0	0	0	0	4	0	0	0	0	0	92	0
Lane Group Flow (vph)	59	709	0	0	858	0	0	0	0	0	10	0
Heavy Vehicles (%)	2%	1%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Perm	NA	- 1		NA	STREE.	5 MIL V 1		1.1	Split	NA	
Protected Phases		4			8			2		6	6	
Permitted Phases	4			8			2					
Actuated Green, G (s)	30.4	30.4			30.4						4.8	
Effective Green, g (s)	30.4	30.4			30.4						4.8	
Actuated g/C Ratio	0.62	0.62			0.62						0.10	
Clearance Time (s)	4.0	4.0			4.0						4.0	
Vehicle Extension (s)	3.0	3.0			3.0						3.0	
Lane Grp Cap (vph)	211	1118		10	1301			S	A. 2 B	20.0	169	
v/s Ratio Prot		0.39			c0.41						c0.01	
v/s Ratio Perm	0.17											
v/c Ratio	0.28	0.63			0.66						0.06	
Uniform Delay, d1	4.4	6.0			6.1						20.2	
Progression Factor	1.00	1.00			1.00						1.00	
Incremental Delay, d2	0.7	1.2			1.2						0.1	
Delay (s)	5.1	7.2			7.4						20.4	
Level of Service	А	А			Α						С	
Approach Delay (s)		7.0			7.4			0.0			20.4	
Approach LOS		A			A			А			С	
Intersection Summary		1.62.54			Sec. Di		i fina (ha'					
HCM 2000 Control Delay			8.0	Н	CM 2000	Level of \$	Service		A			
HCM 2000 Volume to Capa	acity ratio		0.61									
Actuated Cycle Length (s)			49.4	S	um of losi	t time (s)			16.0			
Intersection Capacity Utilization	ation		55.4%	IC	CU Level of	of Service			В			
Analysis Period (min)			15									
c Critical Lane Group												

# 2028 No-Build Wkdy AM 4: Essex Street & Burpee Road

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	1⇒			\$			4			\$	
Traffic Volume (vph)	31	577	0	0	442	45	0	0	1	76	0	54
Future Volume (vph)	31	577	0	0	442	45	0	0	1	76	0	54
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	12	12	16	12	12	12	12	12	12	12
Storage Length (ft)	75		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25	1956	
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph)		30			30			30		S	30	
Link Distance (ft)		228			590			449			248	
Travel Time (s)		5.2			13.4			10.2			5.6	
Peak Hour Factor	0.85	0.85	0.85	0.87	0.87	0.87	0.25	0.25	0.25	0.73	0.73	0.73
Heavy Vehicles (%)	14%	3%	0%	0%	3%	3%	0%	0%	0%	0%	0%	3%
Shared Lane Traffic (%)	1170	0,0	0,0	0,10								900
Lane Group Flow (vph)	36	679	0	0	560	0	0	4	0	0	178	0
Turn Type	Perm	NA	0	Ū	NA		°.	NA		Split	NA	
Protected Phases	T CHI	4			8			2		6	6	
Permitted Phases	4	т.,		8	U		2	-		9/		19
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase	4	4		U	U		-	2		U.		
	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Initial (s)		22.5		22.5	22.5		12.0	12.0		19.0	19.0	
Minimum Split (s)	22.5	39.0		39.0	39.0		12.0	12.0		19.0	19.0	10.00
Total Split (s)	39.0				44.8%		13.8%	13.8%		21.8%	21.8%	Contra s
Total Split (%)	44.8%	44.8%		44.8%			3.0	3.0		3.0	3.0	
Yellow Time (s)	3.0	3.0		3.0	3.0						1.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0		
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	
Total Lost Time (s)	4.0	4.0			4.0			4.0			4.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		None	None	
v/c Ratio	0.13	0.68			0.48			0.01			0.45	
Control Delay	11.6	16.4			11.4			0.0			14.4	
Queue Delay	0.0	0.0			0.0			0.0			0.0	
Total Delay	11.6	16.4			11.4			0.0			14.4	
Queue Length 50th (ft)	3	72			49			0			13	
Queue Length 95th (ft)	34	#544			353			0			60	
Internal Link Dist (ft)		148			510			369			168	
Turn Bay Length (ft)	75											
Base Capacity (vph)	409	1429			1655			642			684	
Starvation Cap Reductn	0	0			0			0			0	
Spillback Cap Reductn	0	0			0			0			0	
Storage Cap Reductn	0	0			0			0			0	
Reduced v/c Ratio	0.09	0.48			0.34			0.01			0.26	
Intersection Summary		in Side			#01	' 같은 같은 이 가 있는 이 가 있다. 이 가 있 이 가 있다. 이 가 있 이 가 있 이	41.5	No.	Wind The		Hand South	
Агеа Туре:	Other											
Cycle Length: 87												

Cycle Length: 87

Lane Group	Ø9	dillos dillo	n en la	전에 15월 13일		Warner Stra		배일 모님
Lane Configurations								
Traffic Volume (vph)								
Future Volume (vph)								
Ideal Flow (vphpl)								
Lane Width (ft)								
Storage Length (ft)				1.14	A Contract of the	A DAME WE S		121
Storage Lanes							and the state	North Law
Taper Length (ft)								
Right Turn on Red						1.5.3112 3.3		
Link Speed (mph) Link Distance (ft)								
Travel Time (s)					16 10 20	and a marked		5 1 5
Peak Hour Factor								
Heavy Vehicles (%)								
Shared Lane Traffic (%)								
Lane Group Flow (vph)						A Children		
Turn Type								
Protected Phases	9							
Permitted Phases								
Detector Phase						Entry Anna Anna Anna Anna Anna Anna Anna Ann		
Switch Phase								
Minimum Initial (s)	5.0							1 LOTE
Minimum Split (s)	17.0							
Total Split (s)	17.0					11 11 11/ 201 <sup>9</sup>	oran manan	
Total Split (%)	20% 3.0							
Yellow Time (s)	3.0 1.0							
All-Red Time (s) Lost Time Adjust (s)	1.0							
Total Lost Time (s)								
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode	None							
v/c Ratio								
Control Delay								
Queue Delay								
Total Delay								
Queue Length 50th (ft)								
Queue Length 95th (ft)								
Internal Link Dist (ft)								
Turn Bay Length (ft)								
Base Capacity (vph) Starvation Cap Reductn								
Spillback Cap Reductn								
Storage Cap Reductn								
Reduced v/c Ratio								
					S	04.586.50		et. Runsall
Intersection Summary		and the second second		1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -		A STREET, MIN	TRACT A STREET	and the second second

### Actuated Cycle Length: 48.3

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

Splits and Phases:	4: Essex Street	& Burpee Road
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1ø2	₩ø6	<u>→</u> Ø4	.#\$ <sub>Ø9</sub>
12 5	19 s	39 s	17 s
		<b>↓</b> Ø8	
		39 s	

# 2028 No-Build Wkdy AM 4: Essex Street & Burpee Road

01/06/2021

	≯	-	$\mathbf{r}$	*	+	*	•	†	1	1	Ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	1>			\$			\$			\$	
Traffic Volume (vph)	31	577	0	0	442	45	0	0	1	76	0	54
Future Volume (vph)	31	577	0	0	442	45	0	0	1	76	0	54
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	11	11	12	12	16	12	12	12	12	12	12	12
Total Lost time (s)	4.0	4.0			4.0			4.0			4.0	
Lane Util. Factor	1.00	1.00			1.00			1.00			1.00	
Frt	1.00	1.00			0.99			0.86			0.94	
Flt Protected	0.95	1.00			1.00			1.00			0.97	
Satd. Flow (prot)	1531	1783			2064			1644			1721	
Flt Permitted	0.32	1.00			1.00			1.00			0.97	
Satd. Flow (perm)	510	1783	1 St. 2.	1	2064	Sublic in	100	1644	2	5.00 11/2	1721	
Peak-hour factor, PHF	0.85	0.85	0.85	0.87	0.87	0.87	0.25	0.25	0.25	0.73	0.73	0.73
Adj. Flow (vph)	36	679	0	0	508	52	0	0	4	104	0	74
RTOR Reduction (vph)	0	0	0	0	3	0	0	4	0	0	95	0
Lane Group Flow (vph)	36	679	0	0	557	0	0	0	0	0	83	0
Heavy Vehicles (%)	14%	3%	0%	0%	3%	3%	0%	0%	0%	0%	0%	3%
Turn Type	Perm	NA		12.1	NA			NA	121	Split	NA	
Protected Phases		4			8			2		6	6	
Permitted Phases	4			8			2					
Actuated Green, G (s)	27.2	27.2			27.2			0.6			8.6	
Effective Green, g (s)	27.2	27.2			27.2			0.6			8.6	
Actuated g/C Ratio	0.50	0.50			0.50			0.01			0.16	
Clearance Time (s)	4.0	4.0			4.0			4.0			4.0	
Vehicle Extension (s)	3.0	3.0			3.0			3.0			3.0	
Lane Grp Cap (vph)	255	893	5 1 81		1033			18			272	
v/s Ratio Prot		c0.38			0.27			c0.00			c0.05	
v/s Ratio Perm	0.07											
v/c Ratio	0.14	0.76			0.54			0.00			0.30	
Uniform Delay, d1	7.3	10.9			9.3			26.6			20.2	
Progression Factor	1.00	1.00			1.00			1.00			1.00	
Incremental Delay, d2	0.3	3.9			0.5			0.1			0.6	
Delay (s)	7.5	14.8			9.8			26.6			20.8	
Level of Service	А	В			А			С			С	
Approach Delay (s)		14.4			9.8			26.6			20.8	
Approach LOS		В			A			С			С	
Intersection Summary	antha tria	metalo							243		W. War	
HCM 2000 Control Delay			13.5	F	ICM 2000	Level of \$	Service		В			
HCM 2000 Volume to Capa	acity ratio		0.61									
Actuated Cycle Length (s)			54.3		Sum of los				16.0			
Intersection Capacity Utilization	ation		51.2%	10	CU Level	of Service			А			
Analysis Period (min)			15									

# 2028 No-Build Wkdy PM 4: Essex Street & Burpee Road

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	Þ			\$			\$			4	
Traffic Volume (vph)	59	670	0	0	771	120	0	0	0	64	0	48
Future Volume (vph)	59	670	0	0	771	120	0	0	0	64	0	48
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	12	12	16	12	12	12	12	12	12	12
Storage Length (ft)	75		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		0	0		0
Taper Length (ft)	25			25	ST ST.		25			25		- 10 - 1
Right Turn on Red	20		Yes			Yes			Yes			Yes
Link Speed (mph)		30	100		30	100		30			30	
Link Distance (ft)		228			590			449			248	
Travel Time (s)		5.2			13.4			10.2			5.6	
Peak Hour Factor	0.85	0.85	0.85	0.93	0.93	0.93	0.25	0.25	0.25	0.95	0.95	0.95
	2%	1%	0.05	1%	0.95	0%	0.20	0.20	0.20	0%	0%	0%
Heavy Vehicles (%)	270	170	0 /0	1 /0	0 /0	0 /0	0 70	0 /0	070	070	070	070
Shared Lane Traffic (%)	00	700	0	0	050	0	0	0	0	0	118	0
Lane Group Flow (vph)	69	788	0	0	958	0	0	0	0		NA	U
Turn Type	Perm	NA			NA			0		Split		
Protected Phases		4			8		0	2		6	6	
Permitted Phases	4	2		8			2	0		0	0	
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase		5 T 44										
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	22.5	22.5		22.5	22.5		12.0	12.0		19.0	19.0	
Total Split (s)	39.0	39.0		39.0	39.0		12.0	12.0		19.0	19.0	신방송에
Total Split (%)	44.8%	44.8%		44.8%	44.8%		13.8%	13.8%		21.8%	21.8%	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	
Total Lost Time (s)	4.0	4.0			4.0			4.0			4.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		None	None	
v/c Ratio	0.34	0.60			0.63						0.37	
Control Delay	15.5	11.5			11.4						10.7	
Queue Delay	0.0	0.0			0.0						0.0	
Total Delay	15.5	11.5			11.4						10.7	
Queue Length 50th (ft)	5	74			91						1	
Queue Length 95th (ft)	#71	#507			#671						46	
Internal Link Dist (ft)		148			510			369			168	
Turn Bay Length (ft)	75	110			0.00							
Base Capacity (vph)	204	1314			1532						603	
Starvation Cap Reductn	0	0			0						0	
Spillback Cap Reductn	0	0			0						0	
Storage Cap Reductn	0	0			0						0	
Reduced v/c Ratio	0.34	0,60			0.63						0.20	
Intersection Summary		- Silen		N+C+15						I. SL	en ser	
Area Type:	Other											
Cycle Length: 87	04101											

Proposed Residential Development VAI

Lane Group	Ø9			
Lane Configurations				
Traffic Volume (vph)				
Future Volume (vph)				
Ideal Flow (vphpl)				
Lane Width (ft)				
Storage Length (ft)	2 - 4 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1			
Storage Lanes				
Taper Length (ft)	San Santa	A THE SECTION		
Right Turn on Red				
Link Speed (mph)				
Link Distance (ft)				
Travel Time (s)	We want of the work			
Peak Hour Factor				
Heavy Vehicles (%)				
Shared Lane Traffic (%)				
Lane Group Flow (vph)	* 1.15 235 mil <sup>10</sup>	1 1993 1 1 1 1 1 1 1 1 1 1 1 1		
Turn Type	**			
Protected Phases	9			
Permitted Phases	56 A			
Detector Phase	181-5-705			
Switch Phase	Dearen Serberer			
Minimum Initial (s)	5.0	attraction of the state		
Minimum Split (s)	17.0			and a second
Total Split (s)	17.0			
Total Split (%)	20%			
Yellow Time (s)	3.0			
All-Red Time (s)	1.0			
Lost Time Adjust (s)				
Total Lost Time (s)				
Lead/Lag	Construction of the second			
Lead-Lag Optimize?				
Recall Mode	None			
v/c Ratio				
Control Delay				
Queue Delay				
Total Delay				
Queue Length 50th (ft)				
Queue Length 95th (ft)				
Internal Link Dist (ft)				
Turn Bay Length (ft)				
Base Capacity (vph)				
Starvation Cap Reductn				
Spillback Cap Reductn				
Storage Cap Reductn				
Reduced v/c Ratio				
Intersection Summary		· 新教教教室: 24	VICE AND AN ADDRESS	
intersection outlindry-a		2823651196 (A. M.) 1 (A.		

Actuated Cycle Length: 52.1

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases:	4: Essex Street	& Burpee Road
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12 s	19 5	39 5	17.5	
		Ø8		

### 2028 No-Build Wkdy PM 4: Essex Street & Burpee Road

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	f.			4			4			<b>(</b> )	
Traffic Volume (vph)	59	670	0	0	771	120	0	0	0	64	0	48
Future Volume (vph)	59	670	0	0	771	120	0	0	0	64	0	48
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	11	11	12	12	16	12	12	12	12	12	12	12
Total Lost time (s)	4.0	4.0			4.0						4.0	
Lane Util. Factor	1.00	1.00			1.00						1.00	
Frt	1.00	1.00			0.98						0.94	
Flt Protected	0.95	1.00			1.00						0.97	
Satd. Flow (prot)	1711	1818			2114						1740	
FIt Permitted	0.16	1.00			1.00						0.97	
Satd. Flow (perm)	282	1818	18 J. M.	1.00	2114		100	1 . A. A.	11.5		1740	
Peak-hour factor, PHF	0.85	0.85	0.85	0.93	0.93	0.93	0.25	0.25	0.25	0.95	0.95	0.95
Adj. Flow (vph)	69	788	0	0	829	129	0	0	0	67	0	51
RTOR Reduction (vph)	0	0	0	0	4	0	0	0	0	0	103	0
Lane Group Flow (vph)	.69	788	0	0	954	0	0	0	0	0	15	0
Heavy Vehicles (%)	2%	1%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Perm	NA	1.1	111	NA					Split	NA	
Protected Phases		4			8			2		6	6	
Permitted Phases	4			8			2					
Actuated Green, G (s)	36.6	36.6			36.6						5.1	
Effective Green, g (s)	36.6	36.6			36.6						5.1	
Actuated g/C Ratio	0.65	0.65			0.65						0.09	
Clearance Time (s)	4.0	4.0			4.0						4.0	-
Vehicle Extension (s)	3.0	3.0			3.0						3.0	
Lane Grp Cap (vph)	183	1183		1.1	1376						157	
v/s Ratio Prot		0.43			c0.45						c0.01	
v/s Ratio Perm	0.24											
v/c Ratio	0.38	0.67			0.69						0.10	
Uniform Delay, d1	4.5	6.0			6.2						23.4	
Progression Factor	1.00	1.00			1.00						1.00	
Incremental Delay, d2	1.3	1.4			1.5						0.3	
Delay (s)	5.8	7.5			7.8						23.7	
Level of Service	A	А			А						С	
Approach Delay (s)		7.3			7.8			0.0			23.7	
Approach LOS		Α			А			А			С	
Intersection Summary					Contraction of the	Whi la		1.6.11	Y soil		. Or Parket	
HCM 2000 Control Delay			8.5	Н	CM 2000	Level of S	Service		А			
HCM 2000 Volume to Capa	city ratio		0.64									
Actuated Cycle Length (s)			56.2	S	um of lost	time (s)			16.0			
Intersection Capacity Utiliza	ation		62.2%		CU Level o				В			
Analysis Period (min)			15									
a Critical Lana Crown												

c Critical Lane Group

# 2028 Build Wkdy AM 4: Essex Street & Burpee Road

01/07/2021

	٨	-	$\mathbf{F}$	4	-	*	-	†	1	1	Ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	ĥ			\$			4				
Traffic Volume (vph)	31	585	0	0	445	45	0	0	1	76	0	54
Future Volume (vph)	31	585	0	0	445	45	0	0	1	76	0	54
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	12	12	16	12	12	12	12	12	12	12
Storage Length (ft)	75		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		0	0		0
Taper Length (ft)	25		ALX.	25			25			25		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		228			590			449			248	
Travel Time (s)		5.2	105		13.4			10.2			5.6	
Peak Hour Factor	0.85	0.85	0.85	0.87	0.87	0.87	0.25	0.25	0.25	0.73	0.73	0.73
Heavy Vehicles (%)	14%	3%	0%	0%	3%	3%	0%	0%	0%	0%	0%	3%
	1470	570	070	070	070	070	0.10	070	070	070	070	070
Shared Lane Traffic (%)	36	688	0	0	563	0	0	4	0	0	178	0
Lane Group Flow (vph)			U	0	NA	U	U	NA	U	Split	NA	U
Turn Type	Perm	NA			8		10.00	2		6	6	
Protected Phases	A INCOMENT	4		0	0	1.00	0	2		0	0	
Permitted Phases	4			8	0		2	0		0	0	
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase								5.0				
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	22.5	22.5		22.5	22.5		12.0	12.0		19.0	19.0	
Total Split (s)	39.0	39.0		39.0	39.0		12.0	12.0		19.0	19.0	
Total Split (%)	44.8%	44.8%		44.8%	44.8%		13.8%	13.8%		21.8%	21.8%	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	
Total Lost Time (s)	4.0	4.0			4.0			4.0			4.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		None	None	
v/c Ratio	0.12	0.67			0.47			0.01			0.46	
Control Delay	11.5	16.3			11.3			0.0			14.7	
Queue Delay	0.0	0.0			0.0			0.0			0.0	
Total Delay	11.5	16.3			11.3			0.0			14.7	
Queue Length 50th (ft)	3	73			50			0			13	
Queue Length 95th (ft)	34	#555			356			0			60	
Internal Link Dist (ft)	UT	148			510			369			168	
Turn Bay Length (ft)	75	140			010			000			100	
Base Capacity (vph)	405	1407			1632			634			666	
					0			0			000	
Starvation Cap Reductn	0	0			0			0			0	
Spillback Cap Reductn	0	0			0			0			0	
Storage Cap Reductn	0	0									•	
Reduced v/c Ratio	0.09	0.49			0.34			0.01			0.27	
Intersection Summary	Other	010		(Chings)			TN 22 a	ALC: N	2019	1.000	and at	1000
Area Type:	Other											
Cycle Length: 87												

Proposed Residential Development VAI

Synchro 10 Report C:\Users\BGuen\Documents\8688\Analysis\bam.syn

Lane Group	Ø9	Strant's	and the subject			
Lane Configurations						
Traffic Volume (vph)						
Future Volume (vph)						
Ideal Flow (vphpl)						
Lane Width (ft)						
Storage Length (ft)						
Storage Lanes						
Taper Length (ft)						
Right Turn on Red						
Link Speed (mph)						
Link Distance (ft)						
Travel Time (s)			Magdel			
Peak Hour Factor						
Heavy Vehicles (%)						
Shared Lane Traffic (%)						
Lane Group Flow (vph)						
Turn Type						
Protected Phases	9					
Permitted Phases						
Detector Phase	1 2 0 0					
Switch Phase						
Minimum Initial (s)	5.0					
Minimum Split (s)	17.0					
Total Split (s)	17.0					
Total Split (%)	20%					
Yellow Time (s)	3.0		A TIME OF A			
All-Red Time (s)	1.0					
Lost Time Adjust (s)						
Total Lost Time (s)						
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None					
v/c Ratio						
Control Delay						
Queue Delay						
Total Delay						
Queue Length 50th (ft)						
Queue Length 95th (ft)						
Internal Link Dist (ft)						
Turn Bay Length (ft)						
Base Capacity (vph)						
Starvation Cap Reductn						
Spillback Cap Reductn						
Storage Cap Reductn						
Reduced v/c Ratio						
						10 m 20 m 4
Intersection Summary				with a state of the state of th	(10 m) - 1 m	a second from a s

#### 2028 Build Wkdy AM 4: Essex Street & Burpee Road

Actuated Cycle Length: 49.3

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

#### Splits and Phases: 4: Essex Street & Burpee Road

1 <sub>ø2</sub>	<b>↓</b> <sub>Ø6</sub>		£B <sub>Ø9</sub>
12 s	19 s	39.8	17 s
		<b>1</b> Ø8	
		39 s	

# 2028 Build Wkdy AM 4: Essex Street & Burpee Road

01/07/2021

	۶	->	7	*	-	*	1	1	1	1	÷.	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ኘ	¢ĵ			\$			4			4	
Traffic Volume (vph)	31	585	0	0	445	45	0	0	1	76	0	54
Future Volume (vph)	31	585	0	0	445	45	0	0	1	76	0	54
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	11	11	12	12	16	12	12	12	12	12	12	12
Total Lost time (s)	4.0	4.0			4.0			4.0			4.0	
Lane Util. Factor	1.00	1.00			1.00			1.00			1.00	
Frt	1.00	1.00			0.99			0.86			0.94	
Flt Protected	0.95	1.00			1.00			1.00			0.97	
Satd. Flow (prot)	1531	1783			2065			1644			1721	
Fit Permitted	0.32	1.00			1.00			1.00			0.97	
Satd. Flow (perm)	514	1783	a Think		2065		1.11	1644		37 - 1	1721	1.1
Peak-hour factor, PHF	0.85	0.85	0.85	0.87	0.87	0.87	0.25	0.25	0.25	0.73	0.73	0.73
Adj. Flow (vph)	36	688	0	0	511	52	0	0	4	104	0	74
RTOR Reduction (vph)	0	0	0	0	3	0	0	4	0	0	96	0
Lane Group Flow (vph)	36	688	0	0	560	0	0	0	0	0	82	0
Heavy Vehicles (%)	14%	3%	0%	0%	3%	3%	0%	0%	0%	0%	0%	3%
Turn Type	Perm	NA	12.1	- kanjur	NA			NA		Split	NA	
Protected Phases		4			8			2		6	6	
Permitted Phases	4			8			2					
Actuated Green, G (s)	28.2	28.2			28.2			0.6			8.5	
Effective Green, g (s)	28.2	28.2			28.2			0.6			8.5	
Actuated g/C Ratio	0.51	0.51			0.51			0.01			0.15	
Clearance Time (s)	4.0	4.0			4.0			4.0			4.0	
Vehicle Extension (s)	3.0	3.0			3.0			3.0			3.0	
Lane Grp Cap (vph)	262	909			1053			17			264	
v/s Ratio Prot		c0.39			0.27			c0.00			c0.05	
v/s Ratio Perm	0.07											
v/c Ratio	0.14	0.76			0.53			0.00			0.31	
Uniform Delay, d1	7.1	10.8			9.1			27.1			20.8	
Progression Factor	1.00	1.00			1.00			1.00			1.00	
Incremental Delay, d2	0.2	3.6			0.5			0.1			0.7	
Delay (s)	7.4	14.5			9.6			27.1			21.5	
Level of Service	А	В			A			С			С	
Approach Delay (s)		14.1			9.6			27.1			21.5	
Approach LOS		В			А			С			С	
Intersection Summary		1.500		silling.	50.3	Renativ	wê bi		15 24			
HCM 2000 Control Delay			13.3	Н	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capa	icity ratio		0.61									
Actuated Cycle Length (s)			55.3		um of lost				16.0			
Intersection Capacity Utiliza	ation		51.6%	IC	CU Level o	of Service			A			
Analysis Period (min)			15									
a Critical Lana Croup												

c Critical Lane Group

#### 2028 Build Wkdy PM 4: Essex Street & Burpee Road

01/07/2021

	≯	-	$\rightarrow$	4	+	×	-	<b>†</b>	1	1	Ļ	-
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ኘ	<b>↓</b>			\$			¢			\$	- X
Traffic Volume (vph)	59	676	0	0	779	120	0	0	0	64	0	48
Future Volume (vph)	59	676	0	0	779	120	0	0	0	64	0	48
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	12	12	16	12	12	12	12	12	12	12
Storage Length (ft)	75		0	0		0	0		0	0	THE WOL	0
Storage Lanes	1		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		228			590			449			248	
Travel Time (s)		5.2			13.4			10.2			5.6	
Peak Hour Factor	0.85	0.85	0.85	0.93	0.93	0.93	0.25	0.25	0.25	0.95	0.95	0.95
Heavy Vehicles (%)	2%	1%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)	270	170	070	170	070	070	070	070	070	070	070	070
	69	795	0	0	967	0	0	0	0	0	118	0
Lane Group Flow (vph)	Perm	NA	U	U	NA	0	U	0	U	Split	NA	U
Turn Type	Perm	NA 4			NA 8			2		6 Spin	6	del coniri
Protected Phases	4	4		0	0		2	2		0	U	
Permitted Phases	4			8	0		2	0		6	c	
Detector Phase	4	4		8	8		2	2		0	6	10 M 1
Switch Phase	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Initial (s)	5.0	5.0	100	5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	22.5	22.5		22.5	22.5		12.0	12.0		19.0	19.0	
Total Split (s)	39.0	39.0		39.0	39.0		12.0	12.0		19.0	19.0	
Total Split (%)	44.8%	44.8%		44.8%	44.8%		13.8%	13.8%		21.8%	21.8%	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	
Total Lost Time (s)	4.0	4.0			4.0			4.0			4.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		None	None	
v/c Ratio	0.35	0.61			0.63						0.37	
Control Delay	16.4	11.7			11.5						10.7	
Queue Delay	0.0	0.0			0.0						0.0	
Total Delay	16.4	11.7			11.5						10.7	
Queue Length 50th (ft)	5	75			93						1	
Queue Length 95th (ft)	#73	#513			#680						46	
Internal Link Dist (ft)		148			510			369			168	
Turn Bay Length (ft)	75											
Base Capacity (vph)	198	1314			1532						603	
Starvation Cap Reductn	0	0			0						0	
Spillback Cap Reductn	0	0			0						0	
Storage Cap Reductn	0	0			0						0	
Reduced v/c Ratio	0.35	0.61			0,63						0,20	
Intersection Summary	4454		18 57			1993 (A. 1995)	5.4.4			민지 모모		1.52.1
Area Type:	Other											
Cycle Length: 87												

Proposed Residential Development VAI

Synchro 10 Report C:\Users\BGuen\Documents\8688\Analysis\bpm.syn

Lane Group	Ø9	N T NAL OF THE	apply the part the	16.40		
Lane Configurations						
Traffic Volume (vph)						SPENN Y
Future Volume (vph)						
Ideal Flow (vphpl)						
Lane Width (ft)						
Storage Length (ft)						
Storage Lanes						
Taper Length (ft)						11 1 2
Right Turn on Red						
Link Speed (mph)						
Link Distance (ft)						
Travel Time (s)						
Peak Hour Factor						
Heavy Vehicles (%)						
Shared Lane Traffic (%)						
Lane Group Flow (vph)						
Turn Type						
Protected Phases	9					
Permitted Phases						
Detector Phase						- 1 water 1 1
Switch Phase						
Minimum Initial (s)	5.0					
Minimum Split (s)	17.0					
Total Split (s)	17.0					
Total Split (%)	20%					
Yellow Time (s)	3.0					
All-Red Time (s)	1.0					
Lost Time Adjust (s)						
Total Lost Time (s)						
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None					
v/c Ratio						
Control Delay						
Queue Delay						
Total Delay						
Queue Length 50th (ft)						
Queue Length 95th (ft)						
Internal Link Dist (ft)						
Turn Bay Length (ft)						
Base Capacity (vph)						
Starvation Cap Reductn						
Spillback Cap Reductn						
Storage Cap Reductn						
Reduced v/c Ratio						
Intersection Summary			NIN STREET	Witten-	- With 1 - 1 - 1	-3. "Yest"
Intersection Summary						E8 1708-19

#### 2028 Build Wkdy PM 4: Essex Street & Burpee Road

Actuated Cycle Length: 52.1 Natural Cycle: 90

Control Type: Actuated-Uncoordinated

95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 4: Essex Street & Burpee Road

1ø2	<b>₩</b> Ø6		£\$ø9
12 s	19 s	39 s	17 s
		<b>7</b> Ø8	
		39's	

# 2028 Build Wkdy PM 4: Essex Street & Burpee Road

01/07/2021

	٦		7	1	-	*	*	Ť	1	1	Ļ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	¢Î,			\$			\$			4	
Traffic Volume (vph)	59	676	0	0	779	120	0	0	0	64	0	48
Future Volume (vph)	59	676	0	0	779	120	0	0	0	64	0	48
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	11	11	12	12	16	12	12	12	12	12	12	12
Total Lost time (s)	4.0	4.0			4.0						4.0	
Lane Util. Factor	1.00	1.00			1.00						1.00	
Frt	1.00	1.00			0.98						0.94	
Flt Protected	0.95	1.00			1.00						0.97	
Satd. Flow (prot)	1711	1818			2115						1740	
Flt Permitted	0.15	1.00			1.00						0.97	
Satd. Flow (perm)	273	1818		100	2115	1.1		n st	11.00	W 8.4	1740	
Peak-hour factor, PHF	0.85	0.85	0.85	0.93	0.93	0.93	0.25	0.25	0.25	0.95	0.95	0.95
Adj. Flow (vph)	69	795	0	0	838	129	0	0	0	67	0	51
RTOR Reduction (vph)	0	0	0	0	4	0	0	0	0	0	103	0
Lane Group Flow (vph)	69	795	0	0	963	0	0	0	0	0	15	0
Heavy Vehicles (%)	2%	1%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Perm	NA			NA					Split	NA	
Protected Phases		4			8			2		6	6	
Permitted Phases	4			8			2				1.5.6	
Actuated Green, G (s)	36.6	36.6			36.6						5.1	
Effective Green, g (s)	36.6	36.6			36.6						5.1	
Actuated g/C Ratio	0.65	0.65			0.65						0.09	
Clearance Time (s)	4.0	4.0			4.0						4.0	2.0.5
Vehicle Extension (s)	3.0	3.0			3.0						3.0	
Lane Grp Cap (vph)	177	1183			1377						157	
v/s Ratio Prot		0.44			c0.46						c0.01	
v/s Ratio Perm	0.25										1 3 7	
v/c Ratio	0.39	0.67			0.70						0.10	
Uniform Delay, d1	4.6	6.1			6.3						23.4	
Progression Factor	1.00	1.00			1.00						1.00	
Incremental Delay, d2	1.4	1.5			1.6						0.3	
Delay (s)	6.0	7.6			7.9						23.7	
Level of Service	A	A			А						С	
Approach Delay (s)		7.5			7.9			0.0			23.7	
Approach LOS		A			A			A			С	
Intersection Summary			12158	57.3	tir she			u - 0 - 1	n lleð Fus	1.5		1.1
HCM 2000 Control Delay			8.6	Н	CM 2000	Level of	Service		A			
HCM 2000 Volume to Capa	acity ratio		0.65									
Actuated Cycle Length (s)			56.2		um of los				16.0			
Intersection Capacity Utilization	ation		62.2%	IC	CU Level	of Service			В			
Analysis Period (min)			15									
c Critical Lane Group												

c Critical Lane Group

Essex Street at Pitman Road

		$\mathbf{r}$	1	-	1	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	Þ			÷Î	W		
Traffic Volume (vph)	541	1	0	445	1	1	
Future Volume (vph)	541	1	0	445	1	1	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	15	12	12	14	12	12	
Link Speed (mph)	30			30	30		
Link Distance (ft)	300			106	236		
Travel Time (s)	6.8			2.4	5.4		
Peak Hour Factor	0.86	0.86	0.85	0.85	0.25	0.25	
Heavy Vehicles (%)	3%	0%	0%	3%	0%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	630	0	0	524	8	0	
Sign Control	Free			Free	Stop		
Intersection Summary					al desi	la islino	10 - 20 (N - 11 - 11 - 10 - 10 - 10 - 10 - 10 - 1
Area Type:	Other						

Control Type: Unsignalized

# 2020 Existing Wkdy AM 2: Pitman Road & Essex Street

01/06/2021
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	-	$\mathbf{r}$	*	-	1	1
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1+			đ.	Υ <b>r</b>	and the second
Traffic Volume (veh/h)	541	1	0	445	1	1
Future Volume (Veh/h)	541	1	0	445	1	1
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.86	0.86	0.85	0.85	0.25	0.25
Hourly flow rate (vph)	629	1	0	524	4	4
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)	1 tonto					
Upstream signal (ft)	300			334		
pX, platoon unblocked	000		0.78		0.86	0.78
vC, conflicting volume			630		1154	630
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			389		689	389
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		99	99
cM capacity (veh/h)			924		357	520
	CD 4		1.000	The new place	A.163	Sec. 171
Direction, Lane # Volume Total	EB 1 630	WB 1 524	NB 1 8	A PARAMANAN		
Volume Left	0.00	0	4			
	1	0	4			
Volume Right cSH	1700	924	424			
Volume to Capacity	0.37	0.00	0.02			
	0.37	0.00	0.02			
Queue Length 95th (ft)	0.0	0.0	13.7			
Control Delay (s)	0.0	0.0				
Lane LOS	0.0	0.0	В 13.7			
Approach Delay (s) Approach LOS	0.0	0.0	13.7 B			
			D		the second second second	
Intersection Summary	1997 S. A. Gara			「王と」に	NG S IS	ent'epti
Average Delay			0.1			
Intersection Capacity Utiliz	zation		38.5%	IC	CU Level o	of Service
Analysis Period (min)			15			

# 2020 Existing Wkdy PM 2: Pitman Road & Essex Street

	->	$\mathbf{r}$	-	-	-	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ĵ.			٩ ۴	W.		
Traffic Volume (vph)	661	2	1	740	藏 14	0	
Future Volume (vph)	661	2	1	740	1	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	15	12	12	14	12	12	
Link Speed (mph)	30	N		30	30		
Link Distance (ft)	300			106	236		
Travel Time (s)	6.8	1.5		2.4	5.4		
Peak Hour Factor	0.84	0.84	0.93	0.93	0.25	0.25	
Heavy Vehicles (%)	1%	0%	0%	1%	0%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	789	0	0	797	4	0	
Sign Control	Free			Free	Stop		
Intersection Summary	The start and	12323				10 A	
Area Type:	Other						
Control Type: Unsignalized	Sheet						그는 일이는 것도가 같은 것이라. 것이 같아요.

01/06/2021

# 2020 Existing Wkdy PM 2: Pitman Road & Essex Street

01	/06/2021	
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	-	$\mathbf{i}$	*	-	1	1
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	12	m max		ج ا	Y	1000
Traffic Volume (veh/h)	661	2	1	740	1	0
Future Volume (Veh/h)	661	2	1	740	1	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.84	0.84	0.93	0.93	0.25	0.25
Hourly flow rate (vph)	787	2	1	796	4	0
Pedestrians	E Sara Di	Product				
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)	1. W. C.					
Median type	None			None		
Median storage veh)						
Upstream signal (ft)	300			334		
pX, platoon unblocked			0.76		0.83	0.76
vC, conflicting volume			789		1586	788
vC1, stage 1 conf vol	4					
vC2, stage 2 conf vol						
vCu, unblocked vol	t solott	The second	563		949	562
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		98	100
cM capacity (veh/h)			773		241	403
Direction, Lane #	EB 1	WB 1	NB 1	t sier s	TUNE LA	1-0000
Volume Total	789	797	4			
Volume Left	0	1	4			
Volume Right	2	0	0			
cSH	1700	773	241			
Volume to Capacity	0.46	0.00	0.02			
Queue Length 95th (ft)	0	0	1			
Control Delay (s)	0.0	0.0	20.2			
Lane LOS		Α	С			
Approach Delay (s)	0.0	0.0	20.2			
Approach LOS			С			
Intersection Summary		e sper	1			15 2 7 16
Average Delay			0.1			
Intersection Capacity Utilization	on		49.7%	IC	U Level o	of Service
Analysis Period (min)			15			

		$\rightarrow$	1	-	1	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	€Î			र्भ	34		
Traffic Volume (vph)	601	1	0	498	1	1	
Future Volume (vph)	601	1	0	498	1	1	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	15	12	12	14	12	12	
Link Speed (mph)	30			30	30		
Link Distance (ft)	300			106	236		
Travel Time (s)	6.8			2.4	5.4		
Peak Hour Factor	0.86	0.86	0.85	0.85	0.25	0.25	
Heavy Vehicles (%)	3%	0%	0%	3%	0%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	700	0	0	586	8	0	
Sign Control	Free			Free	Stop		
Intersection Summary		S. Car	iami IV	A States	ANG IN	. <u>X. 1</u> 787	Mr. Aler S.
Area Type: Of	her						
Control Tuno: Unsignalized							

Control Type: Unsignalized

01/06/2021

# 2028 No-Build Wkdy AM 2: Pitman Road & Essex Street

01/06/2021
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	-	$\mathbf{r}$	*	-	-	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ĥ			र्स	¥۴.		
Traffic Volume (veh/h)	601	1	0	498	1	1	
Future Volume (Veh/h)	601	1	0	498	1	1	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.86	0.86	0.85	0.85	0.25	0.25	
Hourly flow rate (vph)	699	1	0	586	4	4	
Pedestrians Lane Width (ft)							
Walking Speed (ft/s)							A STATE OF A
Percent Blockage	<b>N</b>						
Right turn flare (veh)							
Median type	None			None			
Median storage veh)	NONE			None			
Upstream signal (ft)	300			334			
	- 500		0.75	554	0.84	0.75	· · · · · · · · · · · · · · · · · · ·
pX, platoon unblocked			700		1286	700	
vC, conflicting volume			700		1200	700	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol			400		761	437	
vCu, unblocked vol			438		6.4	6.2	
tC, single (s)			4.1		0.4	0.2	
tC, 2 stage (s)			0.0		3.5	3.3	
tF (s)			2.2		99	3.3 99	
p0 queue free %			100				
cM capacity (veh/h)			853		318	470	
Direction, Lane #	EB 1	WB 1	NB 1			SA U 41 - 7	
Volume Total	700	586	8				
Volume Left	0	0	4				
Volume Right	1	0	4				
cSH	1700	853	379				
Volume to Capacity	0.41	0.00	0.02				
Queue Length 95th (ft)	0	0	2				
Control Delay (s)	0.0	0.0	14.7				
Lane LOS			В				
Approach Delay (s)	0.0	0.0	14.7				
Approach LOS			В				
Intersection Summary			5, m >-(	a vite bi			
Average Delay			0.1				
Intersection Capacity Utiliza	ation		41.7%	IC	U Level o	of Service	A
Analysis Period (min)			15				

# 2028 No-Build Wkdy PM 2: Pitman Road & Essex Street

		~	1	-	*	
		•			)	(
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	Ţ⇒			र्स	¥	
Traffic Volume (vph)	738	2	1	823	1	0
Future Volume (vph)	738	2	1	823	1	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	15	12	12	14	12	12
Link Speed (mph)	30		2.20	30	30	
Link Distance (ft)	300			106	236	
Travel Time (s)	6.8			2.4	5.4	
Peak Hour Factor	0.84	0.84	0.93	0.93	0.25	0.25
Heavy Vehicles (%)	1%	0%	0%	1%	0%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	881	0	0	886	4	0
Sign Control	Free			Free	Stop	
Intersection Summary		A. 114	-marker		5 ar 4	
Area Type:	Other					

Control Type: Unsignalized

# 2028 No-Build Wkdy PM 2: Pitman Road & Essex Street

	-	$\rightarrow$	-	+	1	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	1>		and the second second	ę.	14		
Traffic Volume (veh/h)	738	2	1	823	1	0	
Future Volume (Veh/h)	738	2	1	823	1	0	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.84	0.84	0.93	0.93	0.25	0.25	
Hourly flow rate (vph)	879	2	1	885	4	0	
Pedestrians	0.011	gu Ada	100	199			
Lane Width (ft)							
Walking Speed (ft/s)			1003				
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)							
Upstream signal (ft)	300			334			
pX, platoon unblocked			0.73		0.81	0.73	
vC, conflicting volume			881		1767	880	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol			652	1231 103	1079	650	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %	1.20		100		98	100	
cM capacity (veh/h)			689		197	345	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	881	886	4				
Volume Left	0	. 1	4				
Volume Right	2	0	0				
cSH	1700	689	197				
Volume to Capacity	0.52	0.00	0.02				
Queue Length 95th (ft)	0	0	2				
Control Delay (s)	0.0	0.0	23.6				
Lane LOS		Α	С				
Approach Delay (s)	0.0	0.0	23.6				
Approach LOS			С				
Intersection Summary	-Franking	如。咦!	in the		nt satisficati	構作して	
Average Delay	1. 21	- C	0.1				
Intersection Capacity Utilizati	on		54.1%	IC	U Level c	of Service	
Analysis Period (min)			15				

### 2028 Build Wkdy AM 2: Pitman Road & Essex Street

	-	7	*	-	1	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	2014年1月1日2月1日日本市中国主教会内
Lane Configurations	Î≯			et.	N/		
Traffic Volume (vph)	608	1	0	516	1	1	
Future Volume (vph)	608	1	0	516	1	1	
deal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	15	12	12	14	12	12	
Link Speed (mph)	30			30	30		
Link Distance (ft)	300			106	236		
Travel Time (s)	6.8			2.4	5.4		
Peak Hour Factor	0.86	0.86	0.85	0.85	0.25	0.25	
Heavy Vehicles (%)	3%	0%	0%	3%	0%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	708	0	0	607	8	0	
Sign Control	Free			Free	Stop		
Intersection Summary	21.1200		113,7E	ers's	1. 57		
Area Type: Control Type: Unsignalized	Other			-		1.5.5	

Control Type: Unsignalized

# 2028 Build Wkdy AM 2: Pitman Road & Essex Street

	-	$\rightarrow$	4	-	-	1
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	Þ			र्स	- W	
Traffic Volume (veh/h)	608	1	0	516	1	1
Future Volume (Veh/h)	608	1	0	516	1	1
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.86	0.86	0.85	0.85	0.25	0.25
Hourly flow rate (vph)	707	1	0	607	4	4
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)	14					
Median type	None			None		
Median storage veh)				00/		
Upstream signal (ft)	300		0	334	0.04	0.75
pX, platoon unblocked			0.75		0.84	0.75
vC, conflicting volume			708		1314	708
vC1, stage 1 conf vol						
vC2, stage 2 conf vol			445		800	445
vCu, unblocked vol			<b>445</b> 4.1		800 6.4	445 6.2
tC, single (s)			4.1		0.4	0.2
tC, 2 stage (s)			2.2		3.5	3.3
tF (s)			100		3.5	3.3 99
p0 queue free % cM capacity (veh/h)			845		300	464
		14.10%			000	
Direction, Lane #	EB 1	WB 1	NB 1	2 3 c	12413	1.10
Volume Total	708	607	8			
Volume Left	0	0	4			
Volume Right	1 1700	845	364			
cSH	0.42	0.00	0.02			
Volume to Capacity	0.42	0.00	0.02			
Queue Length 95th (ft)	0.0	0.0	15.1			
Control Delay (s) Lane LOS	0.0	0.0	- C			
Approach Delay (s)	0.0	0.0	15.1			
Approach LOS	0.0	0.0	C			
			Ŭ			
Intersection Summary	2, 217, 21	-	0.4		S 11 10 3	
Average Delay			0.1	10	- امندم الل	of Service
Intersection Capacity Utilizat	ion		42.1%	IC	U Level o	or Service
Analysis Period (min)			15			

# 2028 Build Wkdy PM 2: Pitman Road & Essex Street

	-	$\mathbf{r}$	1	+	*	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ĵ≯			¢	4		
Traffic Volume (vph)	757	2	1	836	1	0	
Future Volume (vph)	757	2	1	836	1	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	15	12	12	14	12	12	
Link Speed (mph)	30			30	30		
Link Distance (ft)	300			106	236		
Travel Time (s)	6.8			2.4	5.4		
Peak Hour Factor	0.84	0.84	0.93	0.93	0.25	0.25	
Heavy Vehicles (%)	1%	0%	0%	1%	0%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	903	0	0	900	4	0	
Sign Control	Free			Free	Stop		
Intersection Summary	$\mathbb{H}^{1} \cong \mathbb{H}^{1}$			P. J. R.	19.477	Line St	
Area Type:	Other						

Control Type: Unsignalized

# 2028 Build Wkdy PM 2: Pitman Road & Essex Street

		$\mathbf{r}$	-	-	-	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	<b>∱</b> →		11072014-2413	Ą	¥		
Traffic Volume (veh/h)	757	2	1	836	1	0	
Future Volume (Veh/h)	757	2	1	836	1	0	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.84	0.84	0.93	0.93	0.25	0.25	
Hourly flow rate (vph)	901	2	1	899	4	0	
Pedestrians					er en Sa		
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)				7.0			
Upstream signal (ft)	300			334			
oX, platoon unblocked			0.72		0.81	0.72	
/C, conflicting volume			903		1803	902	
vC1, stage 1 conf vol						2.1	
vC2, stage 2 conf vol							
vCu, unblocked vol			675		1109	674	
tC, single (s)			4.1		6.4	6.2	
C, 2 stage (s)							
F (s)			2.2		3.5	3.3	
p0 queue free %			100		98	100	
cM capacity (veh/h)			670		190	332	
Direction, Lane #	EB 1	WB1	NB 1		97 T 12		a de la casa de las deservos de las de
/olume Total	903	900	4				
/olume Left	0	-1	4				
/olume Right	2	0	0				
SH	1700	670	190				
Volume to Capacity	0.53	0.00	0.02				
Queue Length 95th (ft)	0	0	2				
Control Delay (s)	0.0	0.0	24.3				
Lane LOS		Α	С				
Approach Delay (s)	0.0	0.0	24.3				
Approach LOS			С				
Intersection Summary	1.19					in Parcellina	2. 전에 가지 않는 것 같은 것 같이 있다. 이 것 같이 있는 것 같이 있다. 이 것 같이 있는 것 같이 있다. 같이 있는 것 같은 것 같은 것 같은 것 같은 것 같은 것 같이 있는 것
Average Delay			0.1				
Intersection Capacity Utilization	on		54.8%	IC	U Level o	of Service	e A
Analysis Period (min)			15				

Essex Street at Elm Place (South)

#### 2020 Existing Wkdy AM 3: Elm Place & Essex Street/Essex Street

		>	1	-	-	-	
		•			ì	· · · ·	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	''''''''''''''''''''''''''''''''''''''
Lane Configurations	<b>1</b> >			र्भ	¥,4		
Traffic Volume (vph)	534	8	4	435	10	11	
Future Volume (vph)	534	8	4	435	10	11	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	15	12	12	13	10	12	
Link Speed (mph)	30			30	30		
Link Distance (ft)	106			228	383		
Travel Time (s)	2.4			5.2	8.7		
Peak Hour Factor	0.88	0.88	0.92	0.92	0.41	0.41	
Heavy Vehicles (%)	3%	0%	0%	3%	0%	11%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	616	0	0	477	51	0	
Sign Control	Free			Free	Stop		
Intersection Summary	Sau - Nev				Turnites		0ml
Area Type:	Other						
Control Type: Unsignaliza	d						

Control Type: Unsignalized

# 2020 Existing Wkdy AM 3: Elm Place & Essex Street/Essex Street

	-	$\rightarrow$	-	-	•	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	Ţ.			÷.	N/		
Traffic Volume (veh/h)	534	8	4	435	10	11	
Future Volume (Veh/h)	534	8	4	435	10	11	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.88	0.88	0.92	0.92	0.41	0.41	
Hourly flow rate (vph)	607	9	4	473	24	27	
Pedestrians				2.000		L. Decret	
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)				2112			
Upstream signal (ft)	406			228			
pX, platoon unblocked			0.79	100	0.87	0.79	
vC, conflicting volume			616		1092	612	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol			377		611	371	
tC, single (s)			4.1		6.4	6.3	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.4	
p0 queue free %			100		94	95	
cM capacity (veh/h)			939		398	516	
Direction, Lane #	EB 1	WB 1	NB 1		THE MENT	e in 1	
Volume Total	616	477	51				
Volume Left	0	4	24				
Volume Right	9	0	27				
cSH	1700	939	453				
Volume to Capacity	0.36	0.00	0.11				
Queue Length 95th (ft)	0	0	9				
Control Delay (s)	0.0	0.1	14.0				
Lane LOS		Α	В				
Approach Delay (s)	0.0	0.1	14.0				
Approach LOS			В				
Intersection Summary		HI SHI Y		<b>用</b> 。[[1]	- 7	ent, ay	の存在に、「「「「「「「「「」」」」という。
Average Delay			0.7	1921			
Intersection Capacity Utiliza	ation		38.6%	IC	U Level o	f Service	А
Analysis Period (min)			15				

#### 2020 Existing Wkdy PM 3: Elm Place & Essex Street/Essex Street

		~	<	-	•	-	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	<b>þ</b>			<del>ا</del>	¥*		
Traffic Volume (vph)	635	26	11	730	11	14	
Future Volume (vph)	635	26	11	730	11	14	
Ideal Flow (vphpl)		1900	1900	1900	1900	1900	
Lane Width (ft)	15	12	12	13	10	12	
Link Speed (mph)	.30			30	30		AND A REAL PROPERTY OF A REAL PR
Link Distance (ft)	106			228	383		
Travel Time (s)				5.2	8.7		
Peak Hour Factor	0.84	0.84	0.94	0.94	0.40	0.40	
Heavy Vehicles (%)	1%	0%	0%	1%	0%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	787	0	0	789	63	0	
Sign Control	Free			Free	Stop		
Intersection Summary	A Kanada			110 - 11 112 - 201-		in the state	
Area Type:	Other						

Control Type: Unsignalized

01/06/2021

# 2020 Existing Wkdy PM 3: Elm Place & Essex Street/Essex Street

01/00/2021	01	/06/2021	
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	-	$\rightarrow$	*	+	-	1
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.			ef.	¥	1000-10001
Traffic Volume (veh/h)	635	26	11	730	11	14
Future Volume (Veh/h)	635	26	11	730	11	14
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.84	0.84	0.94	0.94	0.40	0.40
Hourly flow rate (vph)	756	31	12	777	28	35
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)	406			228		
pX, platoon unblocked			0.76		0.83	0.76
vC, conflicting volume			787		1572	772
vC1, stage 1 conf vol			13 17			
vC2, stage 2 conf vol						
vCu, unblocked vol			559		931	539
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)			1-12			N 11 1 1 1 1
tF (s)			2.2		3.5	3.3
p0 queue free %			98		89	92
cM capacity (veh/h)			774		244	414
5) (3 % 6)	ED 4		NB 1	d tim the	1.1.1.1. 1. 1.1.1.	TO STATE OF
Direction, Lane #	EB 1 787	WB 1 789	63	The full state of the state	CLOY D RHD	
Volume Left	0	12	28			
	31	0	35			
Volume Right cSH	1700	774	316			
	0.46	0.02	0.20			
Volume to Capacity		0.02	18			
Queue Length 95th (ft)	0	0.4	19.2			
Control Delay (s)	0.0					
Lane LOS	0.0	A	C			
Approach Delay (s)	0.0	0.4	19.2			
Approach LOS			С			
Intersection Summary	단행가보고				91 5 <sup>mg</sup> /	E. BYCHIE
Average Delay			0.9			
Intersection Capacity Utilizati	on		57.2%	IC	CU Level o	f Service
Analysis Period (min)			15			

# 2028 No-Build Wkdy AM 3: Elm Place & Essex Street/Essex Street

		~	1	-	*	*	
		•			)	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	Ĵ⇒			र्स	14		
Traffic Volume (vph)	593	9	4	487	11	12	
Future Volume (vph)	593	9	4	487	11	12	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	15	12	12	13	10	12	
Link Speed (mph)	30			30	30		
Link Distance (ft)	106			228	383		
Travel Time (s)	2.4			5.2	8.7		
Peak Hour Factor	0.88	0.88	0.92	0.92	0.41	0.41	
Heavy Vehicles (%)	3%	0%	0%	3%	0%	11%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	684	0	0	533	56	0	
Sign Control	Free			Free	Stop		
Intersection Summary						With the	House and
Area Type:	Other						

Control Type: Unsignalized

01/06/2021

### 2028 No-Build Wkdy AM 3: Elm Place & Essex Street/Essex Street

01/	/06/2021
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	-	$\mathbf{r}$	-	-	1	1		
Movement	EBT	EBR	WBL	WBT	NBL	NBR		1419333-14
Lane Configurations	¢.			<del>با</del>	١			
Traffic Volume (veh/h)	593	9	4	487	11	12		
Future Volume (Veh/h)	593	9	4	487	11	12		
Sign Control	Free	11-12		Free	Stop			
Grade	0%			0%	0%			
Peak Hour Factor	0.88	0.88	0.92	0.92	0.41	0.41		
Hourly flow rate (vph)	674	10	4	529	27	29		
Pedestrians	071	10	112	020	0.0			
Lane Width (ft)								C 7/2
Walking Speed (ft/s)	CALE NO.							SHALL MELLING
Percent Blockage								
Right turn flare (veh)								a shini Mat
Median type	None			None				
Median storage veh)	NONE			NONG				d shelet land
	406			228				
Upstream signal (ft)	400		0.76	220	0.85	0.76		
pX, platoon unblocked			684		1216	679		
vC, conflicting volume			004		1210	079		
vC1, stage 1 conf vol								
vC2, stage 2 conf vol			404		676	415		or states where
vCu, unblocked vol			421		676			
tC, single (s)			4.1		6.4	6.3		
tC, 2 stage (s)			0.0		0.5	0.4		
tF (s)			2.2		3.5	3.4		
p0 queue free %			100		92	94		
cM capacity (veh/h)			869		357	468		
Direction, Lane #	EB 1	WB 1	NB 1	- 1977 - S				Second Belleville
Volume Total	684	533	56					
Volume Left	0	4	27					
Volume Right	10	0	29					
cSH	1700	869	407					1
Volume to Capacity	0.40	0.00	0.14					
Queue Length 95th (ft)	0	0	12					
Control Delay (s)	0.0	0.1	15.3					
Lane LOS		Α	С					
Approach Delay (s)	0.0	0.1	15.3					
Approach LOS			С					
Intersection Summary		Stear 3						
Average Delay			0.7				1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
Intersection Capacity Utiliza	ation		41.8%	IC	U Level c	of Service	А	
Analysis Period (min)	-		15					

		$\mathbf{x}$	1	+	•	-	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	₿			÷,	¥		
Traffic Volume (vph)	709	29	12	812	12	15	
Future Volume (vph)	709	29	12	812	12	15	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	15	12	12	13	10	12	
Link Speed (mph)	30	to EU		30	30		
_ink Distance (ft)	106			228	383		
Travel Time (s)	2.4		32-33	5.2	8.7		
Peak Hour Factor	0.84	0.84	0.94	0.94	0.40	0.40	
Heavy Vehicles (%)	1%	0%	0%	1%	0%	0%	
Shared Lane Traffic (%)							
ane Group Flow (vph)	879	0	0	877	68	0	
Sign Control	Free			Free	Stop		
Intersection Summary				I ANA DI			
Area Type:	Other						
O I T I T I I I I I I I I I I I I I I I							

Control Type: Unsignalized

01/06/2021

01/06/2021	
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	-	$\rightarrow$	4	-	1	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ţ,			<del>ب</del> اً	۲f		
Traffic Volume (veh/h)	709	29	12	812	12	15	
Future Volume (Veh/h)	709	29	12	812	12	15	
Sign Control	Free	**		Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.84	0.84	0.94	0.94	0.40	0.40	
Hourly flow rate (vph)	844	35	13	864	30	38	
Pedestrians Lane Width (ft)						u citily	
Walking Speed (ft/s)		di unital					
Percent Blockage							
Right turn flare (veh)			A 1937 CT				
Median type	None			None			
Median storage veh)	None	S 167		None			
Upstream signal (ft)	406			228			
pX, platoon unblocked	400		0.73	220	0.81	0.73	
vC, conflicting volume	0.1.2000		879		1752	862	
			015	1.00	11.52	002	
vC1, stage 1 conf vol vC2, stage 2 conf vol	In Energy	1.1.2		0.001170			
vCu, unblocked vol		21.22	647		1057	623	
			4.1		6.4	6.2	
tC, single (s)			4.1		0.4	0.2	
tC, 2 stage (s)			2.2		3.5	3.3	
tF (s)			98		85	89	
p0 queue free %				ner obsid	201	357	
cM capacity (veh/h)			690		201	307	
Direction, Lane #	EB 1	WB 1	NB 1	na de la companya de La companya de la comp	日川西部門田	en southing	
Volume Total	879	877	68				
Volume Left	0	13	30				
Volume Right	35	0	38				
cSH	1700	690	266				
Volume to Capacity	0.52	0.02	0.26				
Queue Length 95th (ft)	0	1	25				
Control Delay (s)	0.0	0.5	23.1				
Lane LOS	W	А	С				
Approach Delay (s)	0.0	0.5	23.1				
Approach LOS			С				
Intersection Summary	NI CONTRACTOR		0 days		時種都得		
Average Delay	1810	35.3	1.1	Sec. 1	h 1		
Intersection Capacity Utiliza	ation		62.3%	IC	U Level o	f Service	В
Analysis Period (min)			15				

#### 2028 Build Wkdy AM 3: Elm Place & Essex Street/Essex Street

1 1 ¥ WBL WBT EBR NBL NBR EBT Lane Group 29 **1**≁ 593 Lane Configurations 487 20 7 Traffic Volume (vph) 16 Future Volume (vph) 593 16 7 487 29 20 Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 12 Lane Width (ft) 15 12 12 13 10 30 Link Speed (mph) 30 30 Link Distance (ft) 106 228 265 2.4 5.2 6.0 Travel Time (s) 0.41 Peak Hour Factor 0.88 0.88 0.92 0.92 0.41 Heavy Vehicles (%) 3% 0% 0% 3% 0% 11% Shared Lane Traffic (%) 537 120 Lane Group Flow (vph) 692 0 0 0 Sign Control Free Free Stop Intersection Summary Area Type: Other

Control Type: Unsignalized

#### 2028 Build Wkdy AM 3: Elm Place & Essex Street/Essex Street

	-	$\mathbf{r}$	*	-	1	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ţ.			ર્સ	¥¥.		
Traffic Volume (veh/h)	593	16	7	487	29	20	
Future Volume (Veh/h)	593	16	7	487	29	20	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.88	0.88	0.92	0.92	0.41	0.41	
Hourly flow rate (vph)	674	18	8	529	71	49	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)							
Upstream signal (ft)	406			228			
pX, platoon unblocked			0.75		0.85	0.75	
vC, conflicting volume			692		1228	683	
vC1, stage 1 conf vol					1.00		
vC2, stage 2 conf vol							
vCu, unblocked vol			428		691	416	
tC, single (s)			4.1		6.4	6.3	
tC, 2 stage (s)						UNDER .	
tF (s)			2.2		3.5	3.4	
p0 queue free %			99		79	89	and the second second second
cM capacity (veh/h)			861		346	466	
Direction, Lane #	EB 1	WB 1	NB 1	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	S. S. S. J.	2. 4. 5	
Volume Total	692	537	120			and the second second	
Volume Left	002	8	71				
Volume Right	18	0	49				
cSH	1700	861	387				
Volume to Capacity	0.41	0.01	0.31				
Queue Length 95th (ft)	0.41	1	32				
Control Delay (s)	0.0	0.3	18.4				
Lane LOS	0.0	A	C				
Approach Delay (s)	0.0	0.3	18.4				
Approach LOS	0.0	0.0	C				
Intersection Summary	10.0	1 100	38 T.	12.30	SAR.		이야 있는 동생은 아내는 것 ㅠ~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~
Average Delay			1.7	1	11.7.2	100	
Intersection Capacity Utiliza	ation		42.2%	IC	U Level o	of Service	А
Analysis Period (min)			15				

#### 2028 Build Wkdy PM 3: Elm Place & Essex Street/Essex Street

		~	*	-	-	r	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	Î÷			÷	W		
Traffic Volume (vph)	709	48	20	812	25	21	
Future Volume (vph)	709	48	20	812	25	21	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	15	12	12	13	10	12	
Link Speed (mph)	30			30	30		
Link Distance (ft)	106			228	296		
Travel Time (s)	2.4			5.2	6.7		
Peak Hour Factor	0.84	0.84	0.94	0.94	0.40	0.40	
Heavy Vehicles (%)	1%	0%	0%	1%	0%	0%	
Shared Lane Traffic (%)	1000						
Lane Group Flow (vph)	901	0	0	885	116	0	
Sign Control	Free			Free	Stop		
Intersection Summary	1. State 1.	4,1,231	UN ANA	2025.0			
Area Type:	Other						

Control Type: Unsignalized

# 2028 Build Wkdy PM 3: Elm Place & Essex Street/Essex Street

	-	$\rightarrow$	-	-	1	1		
Movement	EBT	EBR	WBL	WBT	NBL	NBR		121-3
Lane Configurations	¢Î,			<del>با</del>	¥¥.			
Traffic Volume (veh/h)	709	48	20	812	25	21		
Future Volume (Veh/h)	709	48	20	812	25	21		
Sign Control	Free			Free	Stop			
Grade	0%			0%	0%			
Peak Hour Factor	0.84	0.84	0.94	0.94	0.40	0.40		
Hourly flow rate (vph)	844	57	21	864	63	53		
Pedestrians			MAX					
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type	None			None				
Median storage veh)								
Upstream signal (ft)	406			228				
pX, platoon unblocked			0.72		0.81	0.72		
vC, conflicting volume			901		1778	872		
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol			670		1071	630		
tC, single (s)			4.1		6,4	6.2		
tC, 2 stage (s)								
tF (s)			2.2		3.5	3.3		
p0 queue free %			97		67	85		
cM capacity (veh/h)			671		194	350		
Direction, Lane #	EB 1	WB 1	NB 1		0.040		the exection of the sweet	1000
/olume Total	901	885	116		1	A Service		-
Volume Left	0	21	63					
Volume Right	57	0	53					
cSH	1700	671	243					
Volume to Capacity	0.53	0.03	0.48					
Queue Length 95th (ft)	0.00	2	59					
Control Delay (s)	0.0	0.9	32.6					
Lane LOS	0.0	A	02.0 D					
Approach Delay (s)	0.0	0.9	32.6					
Approach LOS	0.0	0.0	52.0 D					
Intersection Summary		SUPUL	1 101 22/07			No the state		2182.5
Average Delay			2.4					
Intersection Capacity Utilizatio	n		68.8%	IC	U Level o	of Service	С	
Analysis Period (min)	1.00		15					

Elm Place at the Project Site Driveway

#### 2028 Build Wkdy AM 5: Elm Place & Site Driveway

1 t ļ 4 ٨  $\mathbf{i}$ NBL NBT SBT EBL EBR SBR Lane Group T+ Lane Configurations 1 4 Traffic Volume (vph) 23 13 10 26 0 0 10 26 0 23 13 Future Volume (vph) 0 1900 1900 1900 1900 1900 1900 Ideal Flow (vphpl) Link Speed (mph) 30 30 30 100 265 Link Distance (ft) 106 6.0 Travel Time (s) 2.4 2.3 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 Shared Lane Traffic (%) 28 0 25 25 0 0 Lane Group Flow (vph) Free Sign Control Stop Free Intersection Summary Area Type: Other

Control Type: Unsignalized

### 2028 Build Wkdy AM 5: Elm Place & Site Driveway

	≯	$\mathbf{r}$	-	1	Ļ	1				
Movement	EBL	EBR	NBL	NBT	SBT	SBR			<b>S</b>	R.
Lane Configurations	Y			<del>ب</del> ا	₽					
Traffic Volume (veh/h)	26	0	0	23	13	10				
Future Volume (Veh/h)	26	0	0	23	13	10				
Sign Control	Stop			Free	Free					
Grade	0%			0%	0%					
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92				
Hourly flow rate (vph)	28	0	0	25	14	11				
Pedestrians										
Lane Width (ft)										
Walking Speed (ft/s)										
Percent Blockage										
Right turn flare (veh)			Number Co.							
Median type		÷ .		None	None					
Median storage veh)			N. 35.	, tono						the state of the later.
Upstream signal (ft)										
pX, platoon unblocked										
vC, conflicting volume	44	20	25							
vC1, stage 1 conf vol		20	20							
vC2, stage 2 conf vol										
vCu, unblocked vol	44	20	25							1 1 1 2 1 2 1 2 1
tC, single (s)	6.4	6.2	4.1							
tC, 2 stage (s)	0.4	0.2								
tF (s)	3.5	3.3	2.2							
p0 queue free %	.97	100	100							
cM capacity (veh/h)	966	1058	1589							
Direction, Lane #	EB 1	NB 1	SB 1 25	1918		CULIN			20-17	a u nan na mula jules
Volume Total	28	25								
Volume Left	28	0	0							
Volume Right	0	0	11							
cSH	966	1589	1700							
Volume to Capacity	0.03	0.00	0.01							
Queue Length 95th (ft)	2	0	0							
Control Delay (s)	8.8	0.0	0.0							
Lane LOS	Α	-1. V-1								
Approach Delay (s)	8.8	0.0	0.0							
Approach LOS	A									
Intersection Summary	14 Å T	15.	New Y			12414.0	NIL NE REP	12.53	1. 1.	AND A PARTY AND A PARTY
Average Delay			3.2							
Intersection Capacity Utilization			13.3%	IC	CU Level o	of Service			А	
Analysis Period (min)			15							

#### 2028 Build Wkdy PM 5: Elm Place & Site Driveway

۶ • ţ 4 t  $\mathbf{i}$ EBL EBR NBL NBT SBT SBR Lane Group **Å** 41 **4** 27 Lane Configurations 1 Traffic Volume (vph) 19 0 0 27 27 Future Volume (vph) 0 27 41 19 0 1900 1900 1900 1900 1900 Ideal Flow (vphpl) 1900 Link Speed (mph) 30 30 30 Link Distance (ft) 97 98 296 Travel Time (s) 2.2 2.2 6.7 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 Shared Lane Traffic (%) 21 29 74 0 0 Lane Group Flow (vph) 0 Sign Control Stop Free Free Intersection Summary Area Type: Other Control Type: Unsignalized

#### 2028 Build Wkdy PM 5: Elm Place & Site Driveway

	۶	7	1	Ť	4	1	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Υ <b>γ</b>			<del>با</del>	1⇒		
Traffic Volume (veh/h)	19	0	0	27	41	27	
Future Volume (Veh/h)	19	0	0	27	41	27	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	21	0	0	29	45	29	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	88	60	74				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	88	60	74				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	98	100	100				
cM capacity (veh/h)	912	1006	1526				
Direction, Lane #	EB 1	NB 1	SB 1		144		
Volume Total	21	29	74				
Volume Left	21	0	0				
Volume Right	0	0	29				
cSH	912	1526	1700				
Volume to Capacity	0.02	0.00	0.04				
Queue Length 95th (ft)	2	0	0				
Control Delay (s)	9.0	0.0	0.0				
ane LOS	Α						
Approach Delay (s)	9.0	0.0	0.0				
Approach LOS	A						
ntersection Summary	712	an in	149°54,				
Average Delay			1.5			10	
Intersection Capacity Utilization	l		13.8%	IC	CU Level o	of Service	A
Analysis Period (min)			15				